



MiTek Canada, Inc.

100 Industrial Road
Bradford, ON, Canada L3Z 3G7
Phone (905) 952-2900
Toll Free (800) 268-3434
Fax (905) 952-2901

August 25,2015

Re: G1507147
W/O# 090

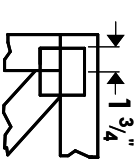
The truss drawing(s) referenced below have been prepared by or for MiTek Canada, Inc. based on the data provided by Nelson Lumber-Grande Prairie,AB (Can).

Pages or sheets enclosed: P4324079 thru P4324125

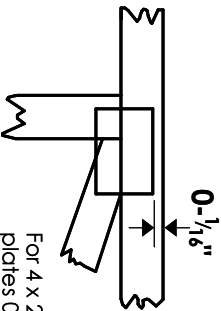
The seal on these drawings indicates acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer. Engineering Services provided by MiTek Canada, Inc

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{8}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

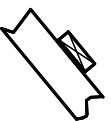
* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

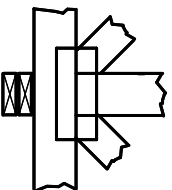
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

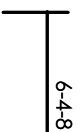


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min. size shown is for crushing only.

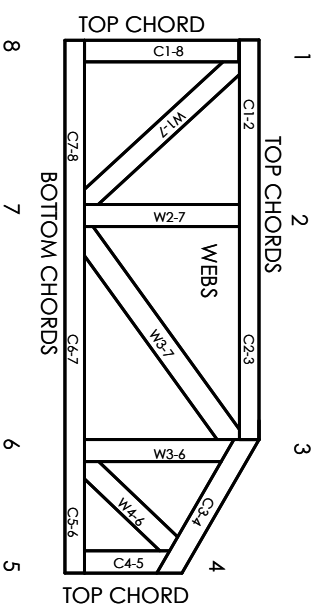
Industry Standards:

TPC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
 DSB-89: Design Standard for Bracing.
 BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm
 (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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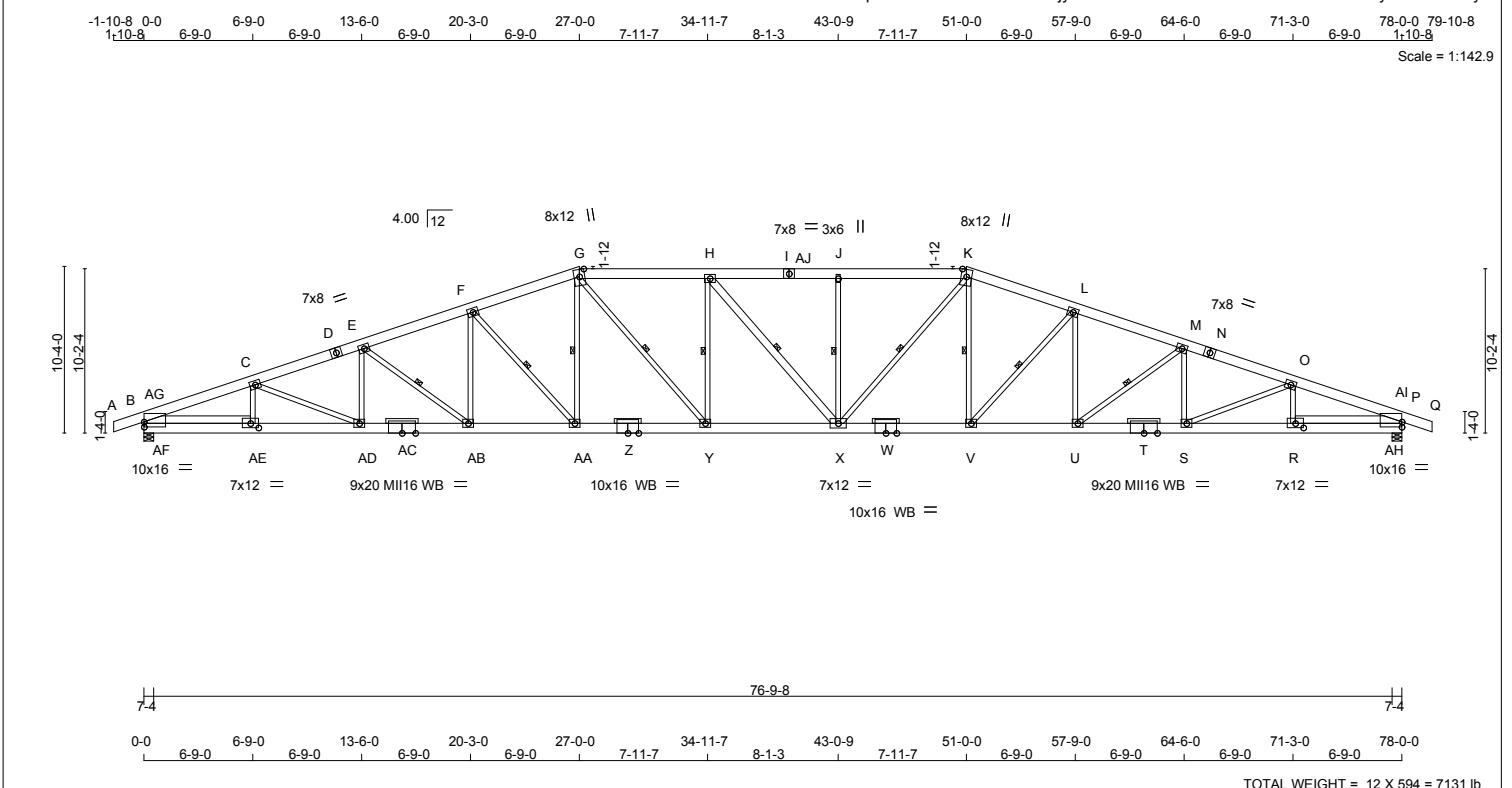
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps of joint locations are regulated by TPC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPC Quality Criteria.



MiTek Engineering Reference Sheet: MII-7473C rev. 09/13/2012



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - G	2x8	DRY 1950F 1.7E	SPF
G - I	2x8	DRY 1950F 1.7E	SPF
I - K	2x8	DRY 1950F 1.7E	SPF
K - N	2x8	DRY 1950F 1.7E	SPF
N - Q	2x8	DRY 1950F 1.7E	SPF
B - AC	2x8	DRY 1950F 1.7E	SPF
AC - Z	2x8	DRY 1950F 1.7E	SPF
Z - W	2x8	DRY 1950F 1.7E	SPF
W - T	2x8	DRY 1950F 1.7E	SPF
T - P	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	LUMBER	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS EXCEPT H - X

MEMBER	SIZE	LUMBER	DESCR.
H - X	2x6	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	5611	0	780 / 0	0 / 0
B	5611	0	780 / 0	0 / 0
P	5611	0	780 / 0	0 / 0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 985 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT P FOR 985 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 190 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	4440	2584 / 0	780 / 0	0 / 0	0 / -1395	1076 / 0	0 / 0
P	4440	2584 / 0	780 / 0	0 / 0	0 / -1395	1076 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	136 / -136	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
DL = 5.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 4.00/12 AND -4.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-4 AND 0-4 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
CALCULATED VERT. DEFL.(LL) = L/999 (0.79")
ALLOWABLE DEFL.(TL)= L/240 (3.90")
CALCULATED VERT. DEFL.(TL) = L/828 (1.13")

CSI: TC=0.34 (M-O-1), BC=0.56 (R-S-1),
WB=0.82 (F-A-A-2), SSI=0.33 (G-H-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
M116	473	276	2341

MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (N) (INPUT = 0.90)
JSI METAL= 0.95 (B) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, F, H, L, M, O						
C	TMWW-t	MT20	6.0	8.0		
D, I, N						
D	TS-t	MT20	7.0	8.0		
G	TTWW+m	MT20	8.0	12.0	Edge	
J	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	8.0	12.0	Edge	
P	TMBMW1-I	MT20	10.0	16.0	3.75	
R	BMWW-t	MT20	7.0	12.0	3.25	6.00
S, U, V, Y, AA, AB, AD						
S	BMWW-t	MT20	6.0	8.0		
T	BS-t	M116	9.0	20.0		
W	BS-t	MT20	10.0	16.0		
X	BMWW-t	MT20	7.0	12.0		
Z	BS-t	MT20	10.0	16.0		
AC	BS-t	M116	9.0	20.0		
AE	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	C H O R D S		W E B S	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED UNBRAC. MEMB. FORCE (LBS)
FR-TO				
A-B	-3 / 0	-107.1 -107.1 0.05 (2)	10.00	AE-C -956 / 260
B-AG	-7850 / 1286	-107.1 -107.1 0.21 (1)	4.21	C-AD -42 / 662
AG-C	-11729 / 1989	-107.1 -107.1 0.32 (1)	3.48	AD-E -101 / 233
C-D	-12297 / 2060	-107.1 -107.1 0.34 (1)	3.40	E-AB -1140 / 321
D-E	-12297 / 2060	-107.1 -107.1 0.34 (1)	3.40	AB-F -109 / 842
E-F	-11582 / 1923	-107.1 -107.1 0.27 (1)	3.55	F-AA -1717 / 474
F-G	-10590 / 1725	-107.1 -107.1 0.26 (1)	3.69	AA-G -268 / 1523
G-H	-11148 / 1730	-119.6 -119.6 0.33 (1)	3.55	G-Y -305 / 2017
H-AJ	-11136 / 1725	-119.6 -119.6 0.33 (1)	3.55	Y-H -1337 / 339
AJ-I	-11136 / 1725	-119.6 -119.6 0.33 (1)	3.55	H-X -1413 / 1392
I-J	-11136 / 1725	-119.6 -119.6 0.33 (1)	3.55	X-J -1195 / 303
J-K	-11137 / 1727	-119.6 -119.6 0.33 (1)	3.55	X-K -301 / 2007
K-L	-10591 / 1726	-107.1 -107.1 0.26 (1)	3.69	V-K -270 / 1529
L-M	-11581 / 1923	-107.1 -107.1 0.27 (1)	3.55	V-L -1715 / 474
M-N	-12297 / 2060	-107.1 -107.1 0.34 (1)	3.40	U-L -108 / 840
N-O	-12297 / 2060	-107.1 -107.1 0.34 (1)	3.40	U-M -1141 / 322
O-AI	-11729 / 1990	-107.1 -107.1 0.32 (1)	3.48	S-M -100 / 234
AI-P	-7850 / 1304	-107.1 -107.1 0.21 (1)	4.21	S-O -42 / 663
P-Q	-3 / 0	-107.1 -107.1 0.05 (3)	10.00	R-O -957 / 259
				AF-AG 0 / 605
B-AF	-1030 / 5380	-27.5 -27.5 0.30 (1)	6.25	AG-AE -905 / 5790
AF-AE	-1030 / 5380	-27.5 -27.5 0.31 (1)	6.25	R-AI -889 / 5790
AE-AD	-1922 / 11091	-27.5 -27.5 0.56 (1)	6.25	AH-AI 0 / 606
AD-AC	-1886 / 11684	-27.5 -27.5 0.53 (1)	6.25	
AC-AB	-1886 / 11684	-27.5 -27.5 0.53 (1)	6.25	
AB-AA	-1639 / 10991	-27.5 -27.5 0.49 (1)	6.25	
AA-Z	-1326 / 10040	-27.5 -27.5 0.45 (1)	6.25	
Z-Y	-1326 / 10040	-27.5 -27.5 0.45 (1)	6.25	
Y-X	-1372 / 11147	-27.5 -27.5 0.51 (1)	6.25	
X-W	-1187 / 10041	-27.5 -27.5 0.46 (1)	6.25	
W-V	-1187 / 10041	-27.5 -27.5 0.46 (1)	6.25	
V-U	-1500 / 10990	-27.5 -27.5 0.50 (1)	6.25	
U-T	-1748 / 11685	-27.5 -27.5 0.53 (1)	6.25	
T-S	-1748 / 11685	-27.5 -27.5 0.53 (1)	6.25	
S-R	-1784 / 11091	-27.5 -27.5 0.56 (1)	6.25	
R-AH	-906 / 5380	-27.5 -27.5 0.31 (1)	6.25	
AH-P	-906 / 5380	-27.5 -27.5 0.30 (1)	6.25	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBC 4.1.6.2.(8)

CONTINUED ON PAGE 2

APEGA Permit No. P3837

August 25, 2015

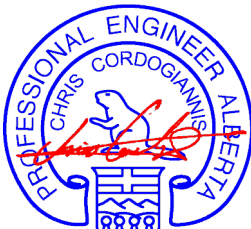
JOB NAME G1507147	TRUSS NAME A	QUANTITY 12	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324079
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:23 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-iamzBecl8740XXU9h7CKEKbf4AKDyV0seY7h1?ykfIA

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

APEGA
Permit No. P3837

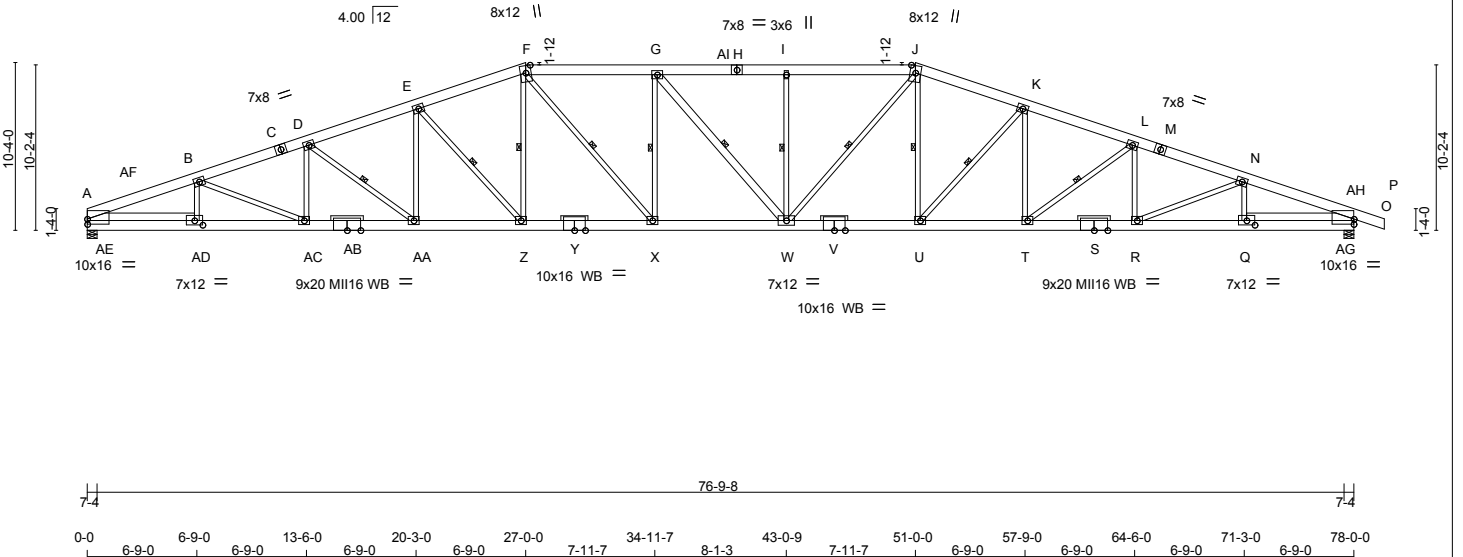
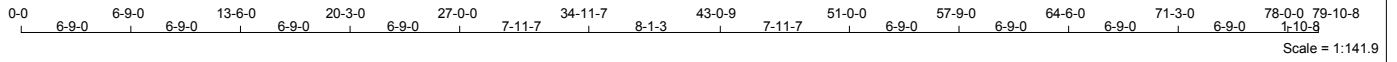


August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.



Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:25 2015 Page 1
 ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-fzujcKe?gkKkneXpYEOJlg?a_?hQPW85scn5tykfl8



TOTAL WEIGHT = 20 X 590 = 11792 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - J	2x8	DRY 1950F 1.7E	SPF
J - M	2x8	DRY 1950F 1.7E	SPF
M - P	2x8	DRY 1950F 1.7E	SPF
A - AB	2x8	DRY 1950F 1.7E	SPF
AB - Y	2x8	DRY 1950F 1.7E	SPF
Y - V	2x8	DRY 1950F 1.7E	SPF
V - S	2x8	DRY 1950F 1.7E	SPF
S - O	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	DRY	No.2	SPF
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT G - W 2x4 DRY No.2 SPF

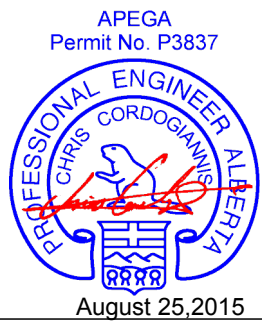
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75
B, D, E, G, K, L, N	TMWW-t	MT20	6.0	8.0	
C, H, M	TS-t	MT20	7.0	8.0	
F	TTWW+m	MT20	8.0	12.0	Edge
I	TMW+w	MT20	3.0	6.0	
J	TTWW+m	MT20	8.0	12.0	Edge
Q	TMBMW1-I	MT20	10.0	16.0	3.75
O	BMWW-t	MT20	7.0	12.0	3.25 6.00
R, T, U, X, Z, AA, AC	BMWW-t	MT20	6.0	8.0	
S	BS-t	MI16	9.0	20.0	
V	BS-t	MT20	10.0	16.0	
W	BMWWW-t	MT20	7.0	12.0	
Y	BS-t	MT20	10.0	16.0	
AB	BS-t	MI16	9.0	20.0	
AD	BMWW-t	MT20	7.0	12.0	3.25 6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
 WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	5399	0	5399	193
O	5611	0	5611	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 924 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT O FOR 985 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 193 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	MAX. PERM. LIVE	WIND	DEAD	SOIL
A	4296	2460 / 0	780 / 0	0 / 0	0 / -1339	1056 / 0	0 / 0
O	4440	2584 / 0	780 / 0	0 / 0	0 / -1395	1076 / 0	0 / 0

HORIZONTAL REACTIONS

JT	1	2	3	4	5	6
A	0 / 0	0 / 0	0 / 0	138 / -135	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, O

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.40FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-AA, E-Z, F-Z, F-X, G-X, G-W, I-W, J-W, J-U, K-U, L-T.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB. UNBRAC	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO		FROM	TO	LENGTH	FR-TO	
A-AF	-7850 / 1286	-107.1	-107.1	0.21 (1)	4.21	AD-B -956 / 260
AF-B	-11729 / 1989	-107.1	-107.1	0.32 (1)	3.48	B-AC -42 / 662
B-C	-12297 / 2060	-107.1	-107.1	0.34 (1)	3.40	AC-D -101 / 233
C-D	-12297 / 2060	-107.1	-107.1	0.34 (1)	3.40	DA-AA -1140 / 321
D-E	-11582 / 1923	-107.1	-107.1	0.27 (1)	3.55	AA-E -109 / 842
E-F	-10590 / 1725	-107.1	-107.1	0.26 (1)	3.69	E-Z -1717 / 474
F-G	-11148 / 1730	-119.6	-119.6	0.33 (1)	3.55	Z-F -268 / 1523
G-AI	-11136 / 1725	-119.6	-119.6	0.33 (1)	3.55	F-X -305 / 2017
AI-H	-11136 / 1725	-119.6	-119.6	0.33 (1)	3.55	X-G -1337 / 339
H-I	-11136 / 1725	-119.6	-119.6	0.33 (1)	3.55	G-W -1413 / 1392
I-J	-11137 / 1727	-119.6	-119.6	0.33 (1)	3.55	W-I -1195 / 303
J-K	-10591 / 1726	-107.1	-107.1	0.26 (1)	3.69	W-J -301 / 2007
K-L	-11581 / 1923	-107.1	-107.1	0.27 (1)	3.55	U-J -270 / 1529
L-M	-12297 / 2060	-107.1	-107.1	0.34 (1)	3.40	U-K -1715 / 474
M-N	-12297 / 2060	-107.1	-107.1	0.34 (1)	3.40	T-K -108 / 840
N-AH	-11729 / 1990	-107.1	-107.1	0.32 (1)	3.48	T-L -1141 / 322
AH-O	-7850 / 1304	-107.1	-107.1	0.21 (1)	4.21	R-L -100 / 234
O-P	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	R-N -42 / 663
						Q-N -957 / 259
A-AE	-1030 / 5380	-27.5	-27.5	0.30 (1)	6.25	AE-AF 0 / 605
AE-AD	-1030 / 5380	-27.5	-27.5	0.31 (1)	6.25	AF-AD -905 / 5790
AD-AC	-1922 / 11091	-27.5	-27.5	0.56 (1)	6.25	Q-AH -889 / 5790
AC-AB	-1886 / 11684	-27.5	-27.5	0.53 (1)	6.25	AG-AH 0 / 606
AB-AA	-1886 / 11684	-27.5	-27.5	0.53 (1)	6.25	
AA-Z	-1639 / 10991	-27.5	-27.5	0.49 (1)	6.25	
Z-Y	-1326 / 10040	-27.5	-27.5	0.45 (1)	6.25	
Y-X	-1326 / 10040	-27.5	-27.5	0.45 (1)	6.25	
X-W	-1372 / 11147	-27.5	-27.5	0.51 (1)	6.25	
W-V	-1187 / 10041	-27.5	-27.5	0.46 (1)	6.25	
V-U	-1187 / 10041	-27.5	-27.5	0.46 (1)	6.25	
U-T	-1500 / 10990	-27.5	-27.5	0.50 (1)	6.25	
T-S	-1748 / 11685	-27.5	-27.5	0.53 (1)	6.25	
S-R	-1748 / 11685	-27.5	-27.5	0.53 (1)	6.25	
R-Q	-1784 / 11091	-27.5	-27.5	0.56 (1)	6.25	
Q-AG	-906 / 5380	-27.5	-27.5	0.31 (1)	6.25	
AG-O	-906 / 5380	-27.5	-27.5	0.30 (1)	6.25	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 4.00/12 AND -4.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-4 AND 0-4 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 -SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 -SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.79")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/828 (1.13")

CSI: TC=0.34 (L-N:1), BC=0.56 (Q-R:1),
 WB=0.82 (E-Z:2), SSI=0.33 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
MI16	473	276	1245

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (M) (INPUT = 0.90)
 JSI METAL= 0.95 (O) (INPUT = 1.00)

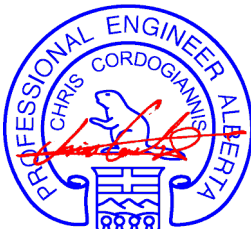
JOB NAME G1507147	TRUSS NAME A1	QUANTITY 20	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324080
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:25 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-fzujcKe?gkKknreXpYFoJlg?a_?hQPW85scn5tykf8

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

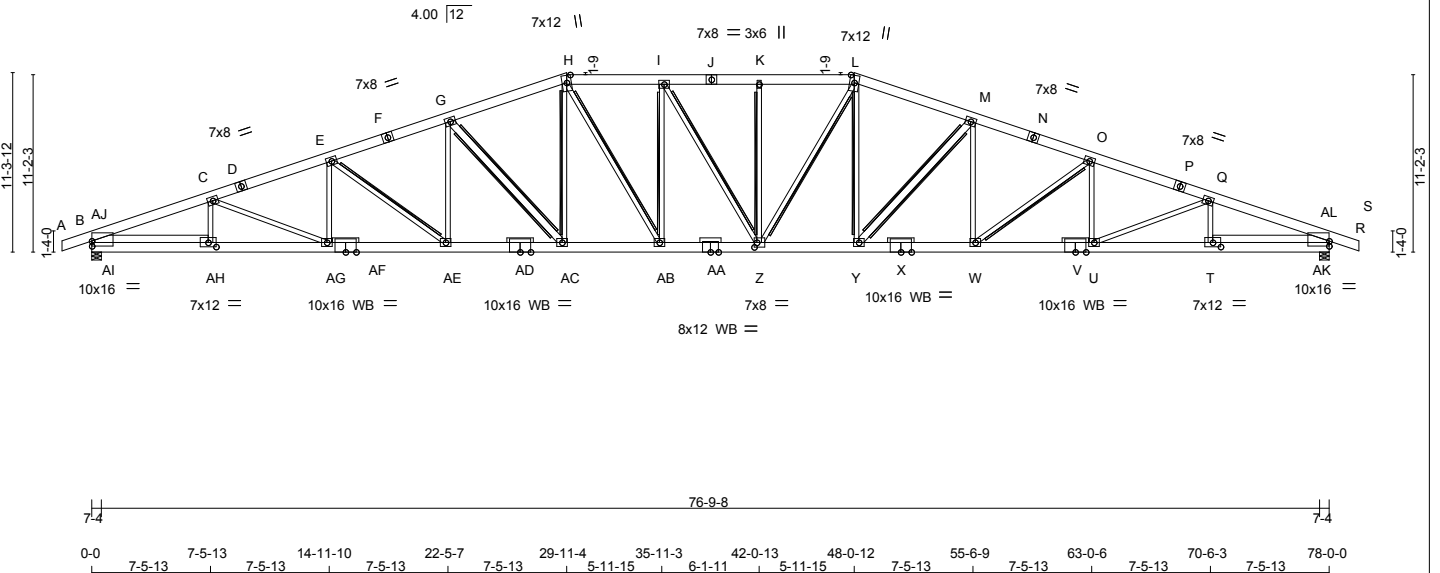
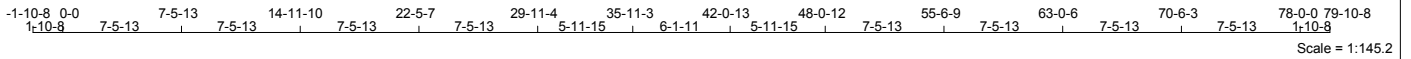
APEGA
Permit No. P3837



August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 617 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - J	2x8	DRY 1950F 1.7E	SPF
J - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
P - S	2x8	DRY 1950F 1.7E	SPF
B - AF	2x8	DRY 1950F 1.7E	SPF
AF - AD	2x8	DRY 1950F 1.7E	SPF
AD - AA	2x8	DRY 1950F 1.7E	SPF
AA - X	2x8	DRY 1950F 1.7E	SPF
X - V	2x8	DRY 1950F 1.7E	SPF
V - R	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

EXCEPT

G - AC	2x6	DRY	No.2	SPF
Y - M	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

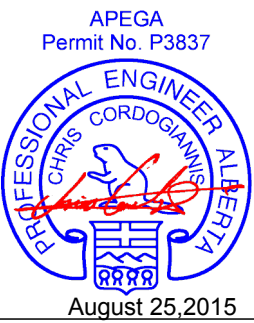
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-t	MT20	10.0	16.0	3.75
C, E, G, I, M, O, Q	C	TMWW-t	MT20	6.0	8.0
D, F, J, N, P	D	TS-t	MT20	7.0	8.0
H	TTWW+m	MT20	7.0	12.0	Edge
K	TMW+w	MT20	3.0	6.0	
L	TTWW+m	MT20	7.0	12.0	Edge
R	TMBMW1-t	MT20	10.0	16.0	3.75
T	BMWW-t	MT20	7.0	12.0	3.25 6.00
U, W, Y, AB, AC, AE, AG	U	BMWW-t	MT20	6.0	8.0
V, X, AD, AF	V	BS-t	MT20	10.0	16.0
Z	BMWWW-t	MT20	7.0	8.0	3.50 2.00
AA	BS-t	MT20	8.0	12.0	
AH	BMWW-t	MT20	7.0	12.0	3.25 6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	5461	0	5461	211	-1150	7-4	5-15
R	5461	0	5461	0	-1115	7-4	5-15

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT R FOR 1115 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 211 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
B	4320	2584 / 0	780 / 0	0 / 0	0 / -1436	956 / 0	0 / 0
R	4320	2584 / 0	780 / 0	0 / 0	0 / -1411	956 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	151 / -151	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, R

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.47FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT E-AE, H-AC, K-Z, L-Y, O-W
 2x4 DRY SPF No.2 I-BRACE AT G-AC, M-Y
 2x4 DRY SPF 2100F 1.8E T-BRACE AT H-AB, I-AB, I-Z, L-Z

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	CHORDS MAX. FACTORED			WEBS MAX. FACTORED		
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB. FORCE (LBS)	UNBRAC LENGTH	FR-TO
FR-TO		FROM	TO			
A-B	-3 / 0	-107.1	-107.1	0.05 (2)	10.00	AH-C -795 / 287
B-AJ	-7750 / 1540	-107.1	-107.1	0.23 (2)	4.22	C-AG -91 / 689
AJ-C	-11582 / 2416	-107.1	-107.1	0.35 (1)	3.49	AG-E -104 / 347
C-D	-11799 / 2490	-107.1	-107.1	0.34 (1)	3.47	E-AE -1354 / 353
D-E	-11799 / 2490	-107.1	-107.1	0.34 (1)	3.47	AE-G -116 / 1012
E-F	-10854 / 2336	-107.1	-107.1	0.30 (2)	3.64	G-AC -2025 / 525
F-G	-10854 / 2336	-107.1	-107.1	0.30 (2)	3.64	AC-H -309 / 1707
G-H	-9628 / 2113	-107.1	-107.1	0.26 (1)	3.85	H-AB -406 / 1900
H-I	-9652 / 2133	-107.1	-107.1	0.19 (1)	3.89	AB-I -1493 / 427
I-J	-9639 / 2071	-107.1	-107.1	0.18 (1)	3.90	I-Z -791 / 739
J-K	-9639 / 2071	-107.1	-107.1	0.18 (1)	3.90	Z-K -862 / 295
K-L	-9640 / 2072	-107.1	-107.1	0.19 (1)	3.88	Z-L -395 / 1872
L-M	-9629 / 2011	-107.1	-107.1	0.26 (1)	3.85	Y-L -320 / 1722
M-N	-10854 / 2236	-107.1	-107.1	0.30 (3)	3.64	Y-M -2022 / 537
N-O	-10854 / 2236	-107.1	-107.1	0.30 (3)	3.64	W-M -124 / 1010
O-P	-11799 / 2395	-107.1	-107.1	0.34 (1)	3.47	W-O -1355 / 367
P-Q	-11799 / 2395	-107.1	-107.1	0.34 (1)	3.47	U-O -108 / 347
Q-AI	-11582 / 2333	-107.1	-107.1	0.35 (1)	3.49	U-Q -90 / 690
AL-R	-7750 / 1508	-107.1	-107.1	0.23 (3)	4.22	T-Q -796 / 278
R-S	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	AI-AJ 0 / 640
						AJ-AH -1114 / 5663
						T-AL -1052 / 5663
						AK-AL 0 / 640
B-AI	-1249 / 5356	-27.5	-27.5	0.30 (1)	6.25	
AI-AH	-1249 / 5356	-27.5	-27.5	0.30 (1)	6.25	
AH-AG	-2337 / 10958	-27.5	-27.5	0.55 (1)	6.25	
AG-AF	-2291 / 11210	-27.5	-27.5	0.51 (1)	6.25	
AF-AE	-2291 / 11210	-27.5	-27.5	0.51 (1)	6.25	
AE-AD	-2014 / 10302	-27.5	-27.5	0.46 (1)	6.25	
AD-AC	-2014 / 10302	-27.5	-27.5	0.46 (1)	6.25	
AC-AB	-1663 / 9114	-27.5	-27.5	0.40 (1)	6.25	
AB-AA	-1739 / 9652	-27.5	-27.5	0.43 (1)	6.25	
AA-Z	-1739 / 9652	-27.5	-27.5	0.43 (1)	6.25	
Z-Y	-1503 / 9115	-27.5	-27.5	0.40 (1)	6.25	
Y-X	-1765 / 10302	-27.5	-27.5	0.46 (1)	6.25	
X-W	-1765 / 10302	-27.5	-27.5	0.46 (1)	6.25	
W-V	-2046 / 11210	-27.5	-27.5	0.51 (1)	6.25	
V-U	-2046 / 11210	-27.5	-27.5	0.51 (1)	6.25	
U-T	-2103 / 10958	-27.5	-27.5	0.55 (1)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBC2010 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74% OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.74")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/932 (1.00")

CSI: TC=0.35 (C-AJ-1), BC=0.55 (AG-AH-1), WB=0.98 (I-AB-3), SSI=0.30 (B-AJ-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (AB) (INPUT = 0.90)

JSI METAL= 1.00 (AF) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A2	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324081
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:53 2015 Page 2
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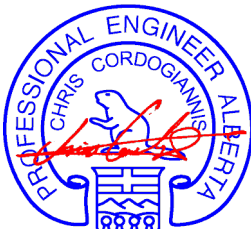
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
T-AK	-1063 / 5356	-27.5 -27.5	0.30 (1)	6.25			
AK-R	-1063 / 5356	-27.5 -27.5	0.30 (1)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

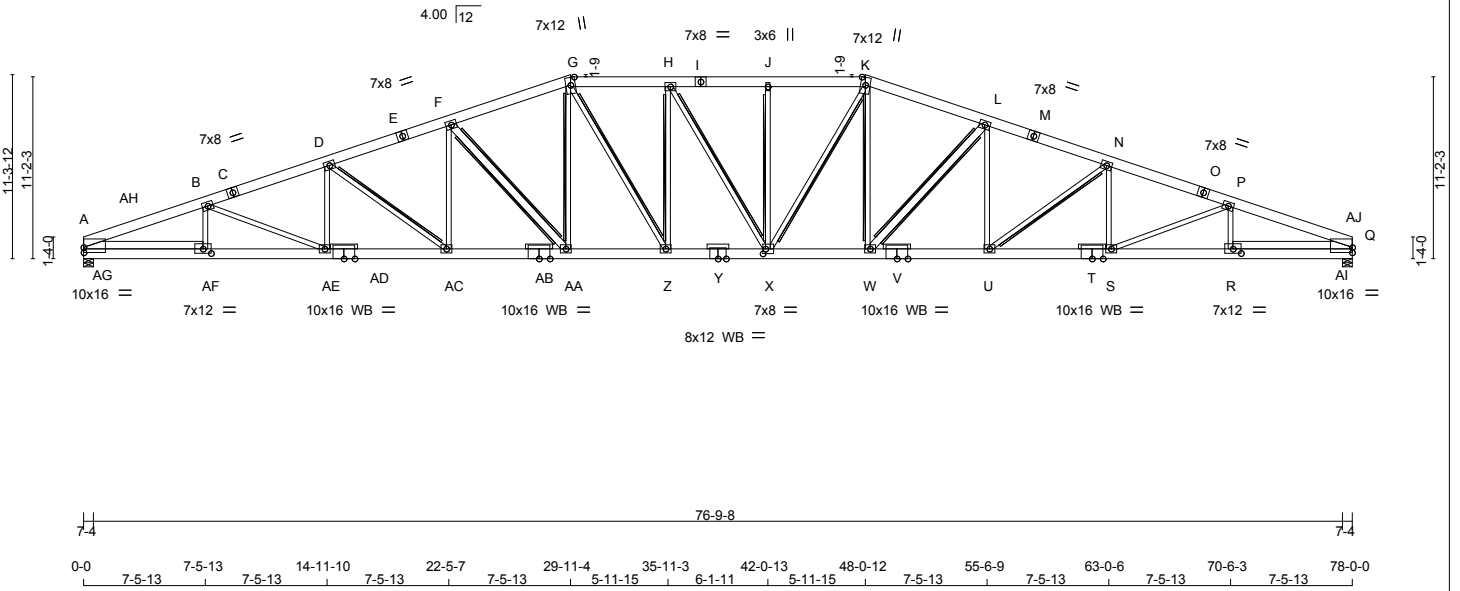
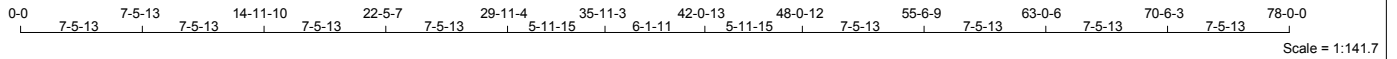
APEGA
Permit No. P3837



August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 608 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - G	2x8	DRY 1950F 1.7E	SPF
G - I	2x8	DRY 1950F 1.7E	SPF
I - K	2x8	DRY 1950F 1.7E	SPF
K - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
O - Q	2x8	DRY 1950F 1.7E	SPF
A - AD	2x8	DRY 1950F 1.7E	SPF
AD - AB	2x8	DRY 1950F 1.7E	SPF
AB - Y	2x8	DRY 1950F 1.7E	SPF
Y - V	2x8	DRY 1950F 1.7E	SPF
V - T	2x8	DRY 1950F 1.7E	SPF
T - Q	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT

F - AA	2x6	DRY	No.2	SPF
W - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

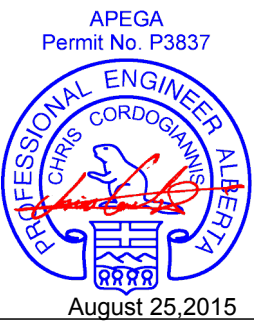
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-t	MT20	10.0	16.0	3.75
B, D, F, H, L, N, P	TMWW-t	MT20	6.0	8.0	
C, E, I, M, O	TS-t	MT20	7.0	8.0	
G	TTWW+m	MT20	7.0	12.0	Edge
J	TMW+w	MT20	3.0	6.0	
K	TTWW+m	MT20	7.0	12.0	Edge
Q	TMBMW1-t	MT20	10.0	16.0	3.75
R	BMWW-t	MT20	7.0	12.0	3.25 6.00
S, U, W, Z, AA, AC, AE	BMWW-t	MT20	6.0	8.0	
T, V, AB, AD	BS-t	MT20	10.0	16.0	
X	BMWWW-t	MT20	7.0	8.0	3.50 2.00
Y	BS-t	MT20	8.0	12.0	
AF	BMWW-t	MT20	7.0	12.0	3.25 6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
A	5249	0	5249	212
Q	5249	0	5249	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1089 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1054 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 212 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX SNOW	MIN LIVE	COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
A	4176	2460 / 0	780 / 0	0 / 0	0 / -1379	936 / 0	0 / 0
Q	4176	2460 / 0	780 / 0	0 / 0	0 / -1355	936 / 0	0 / 0

HORIZONTAL REACTIONS

JT	FACTORED	UNBRACED	WIND	DEAD	SOIL
A	0 / 0	0 / 0	152 / -152	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, Q

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.47FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT D-AC, G-AA, J-X, K-W, N-U
 2x4 DRY SPF No.2 I-BRACE AT F-AA, L-W
 2x4 DRY SPF 2100F 1.8E T-BRACE AT G-Z, H-Z, H-X, K-X

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

CHORDS		FACTORED		WEBS		FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-AH	-7750 / 1540	-107.1	-107.1	0.23 (2)	4.22	AF-B	-795 / 287	0.11 (1)
AH-B	-11582 / 2416	-107.1	-107.1	0.35 (1)	3.49	B-AE	-91 / 689	0.11 (3)
B-C	-11799 / 2490	-107.1	-107.1	0.34 (1)	3.47	AE-D	-104 / 347	0.06 (5)
C-D	-11799 / 2490	-107.1	-107.1	0.34 (1)	3.47	D-AC	-1354 / 353	0.73 (2)
D-E	-10854 / 2336	-107.1	-107.1	0.30 (2)	3.64	AC-F	-116 / 1012	0.16 (2)
E-F	-10854 / 2336	-107.1	-107.1	0.30 (2)	3.64	F-AA	-2025 / 525	0.95 (2)
F-G	-9628 / 2113	-107.1	-107.1	0.26 (1)	3.85	AA-G	-309 / 1707	0.27 (2)
G-H	-9652 / 2133	-107.1	-107.1	0.19 (1)	3.89	G-Z	-406 / 1900	0.39 (14)
H-I	-9639 / 2071	-107.1	-107.1	0.18 (1)	3.90	Z-H	-1493 / 427	0.98 (3)
I-J	-9639 / 2071	-107.1	-107.1	0.18 (1)	3.90	H-X	-791 / 739	0.76 (9)
J-K	-9640 / 2072	-107.1	-107.1	0.19 (1)	3.88	X-J	-862 / 295	0.63 (3)
K-L	-9629 / 2011	-107.1	-107.1	0.26 (1)	3.85	X-K	-395 / 1872	0.38 (13)
L-M	-10854 / 2236	-107.1	-107.1	0.30 (3)	3.64	W-K	-320 / 1722	0.28 (3)
M-N	-10854 / 2236	-107.1	-107.1	0.30 (3)	3.64	W-L	-2022 / 537	0.95 (3)
N-O	-11799 / 2395	-107.1	-107.1	0.34 (1)	3.47	U-L	-124 / 1010	0.16 (3)
O-P	-11799 / 2395	-107.1	-107.1	0.34 (1)	3.47	U-N	-1355 / 367	0.73 (3)
P-AJ	-11582 / 2333	-107.1	-107.1	0.35 (1)	3.49	S-N	-108 / 347	0.06 (6)
AJ-Q	-7750 / 1508	-107.1	-107.1	0.23 (3)	4.22	S-P	-90 / 690	0.11 (2)
						R-P	-796 / 278	0.11 (1)
A-AG	-1248 / 5356	-27.5	-27.5	0.30 (1)	6.25	AG-AH	0 / 640	0.00 (1)
AG-AF	-1248 / 5356	-27.5	-27.5	0.30 (1)	6.25	AH-AF	-1114 / 5663	0.67 (1)
AF-AE	-2335 / 10958	-27.5	-27.5	0.55 (1)	6.25	R-AJ	-1052 / 5663	0.67 (1)
AE-AD	-2290 / 11210	-27.5	-27.5	0.51 (1)	6.25	AI-AJ	0 / 640	0.00 (1)
AD-AC	-2290 / 11210	-27.5	-27.5	0.51 (1)	6.25			
AC-AB	-2013 / 10302	-27.5	-27.5	0.46 (1)	6.25			
AB-AA	-2013 / 10302	-27.5	-27.5	0.46 (1)	6.25			
AA-Z	-1661 / 9114	-27.5	-27.5	0.40 (1)	6.25			
Z-Y	-1737 / 9652	-27.5	-27.5	0.43 (1)	6.25			
Y-X	-1737 / 9652	-27.5	-27.5	0.43 (1)	6.25			
X-W	-1501 / 9115	-27.5	-27.5	0.40 (1)	6.25			
W-V	-1763 / 10302	-27.5	-27.5	0.46 (1)	6.25			
V-U	-1763 / 10302	-27.5	-27.5	0.46 (1)	6.25			
U-T	-2044 / 11210	-27.5	-27.5	0.51 (1)	6.25			
T-S	-2044 / 11210	-27.5	-27.5	0.51 (1)	6.25			
S-R	-2101 / 10958	-27.5	-27.5	0.55 (1)	6.25			
R-AI	-1061 / 5356	-27.5	-27.5	0.30 (1)	6.25			
AI-Q	-1061 / 5356	-27.5	-27.5	0.30 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCS 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCS 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.74")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/932 (1.00")

CSI: TC=0.35 (B-AH:1), BC=0.55 (AE-AF:1),
 WB=0.98 (H-Z:3), SSI=0.30 (A-AH:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Z) (INPUT = 0.90)
 JSI METAL= 1.00 (AD) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A2A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324082
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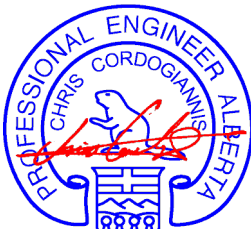
Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:56 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-FxGS6x0RMGEK?cVU7Zl43HSleTa4BNeiDAdwgaykfkf

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM),INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX
AWAY FROM EAVE.

APEGA
Permit No. P3837

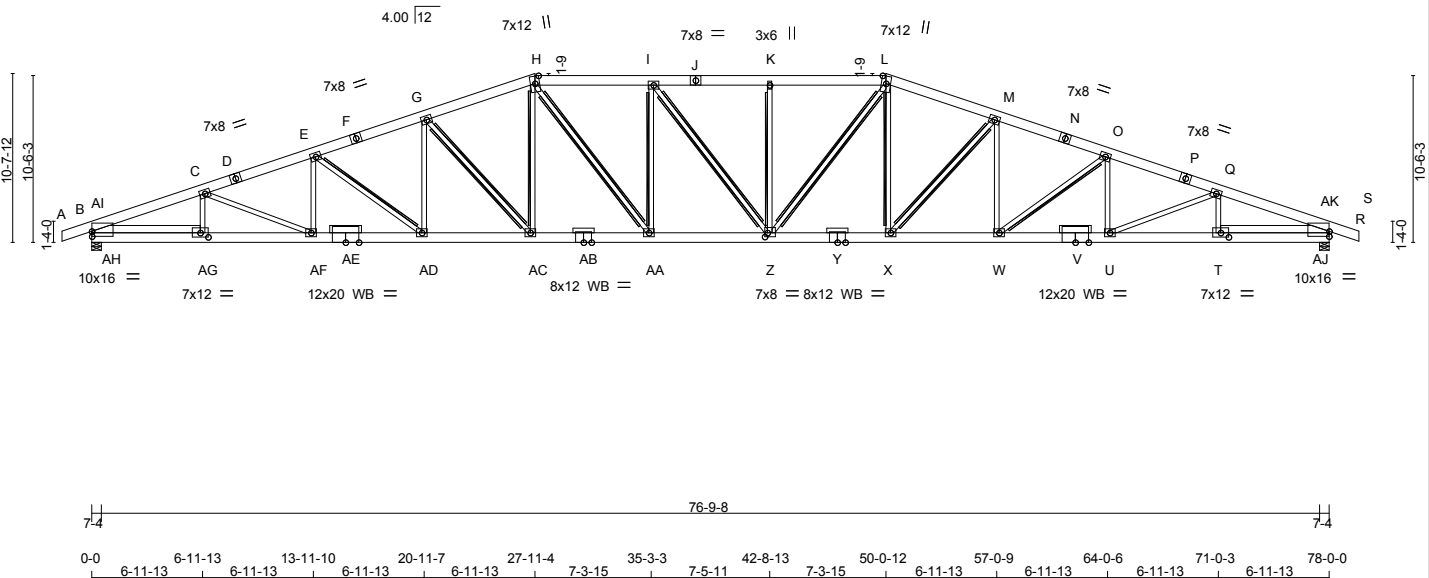
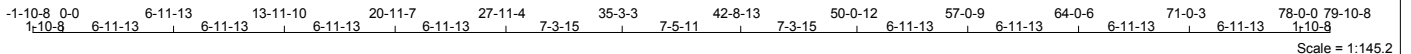


August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.



Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:59 2015 Page 1
 ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-fWxaly2fBcvs3E3ohrnw4p6hcsOkS9v8rbHvykfk



TOTAL WEIGHT = 593 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - J	2x8	DRY 1950F 1.7E	SPF
J - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
P - S	2x8	DRY 1950F 1.7E	SPF
B - AE	2x8	DRY 1950F 1.7E	SPF
AE - AB	2x8	DRY 1950F 1.7E	SPF
AB - Y	2x8	DRY 1950F 1.7E	SPF
Y - V	2x8	DRY 1950F 1.7E	SPF
V - R	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

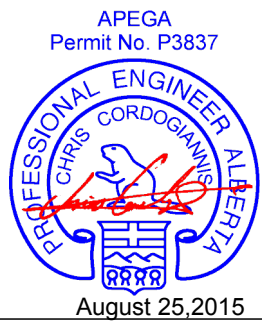
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, G, I, M, O, Q						
C	TMWW-t	MT20	6.0	8.0		
D, F, J, N, P						
D	TS-t	MT20	7.0	8.0		
H	TTWW+m	MT20	7.0	12.0	Edge	
K	TMW+w	MT20	3.0	6.0		
L	TTWW+m	MT20	7.0	12.0	Edge	
R	TMBMW1-I	MT20	10.0	16.0	3.75	
T	BMWW-t	MT20	7.0	12.0	3.25	6.00
U, W, X, AA, AC, AD, AF						
U	BMWW-t	MT20	6.0	8.0		
V	BS-t	MT20	12.0	20.0		
Y	BS-t	MT20	8.0	12.0		
Z	BMWWW-t	MT20	7.0	8.0	3.00	2.00
AB	BS-t	MT20	8.0	12.0		
AE	BS-t	MT20	12.0	20.0		
AG	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	5461	0	5517	198	-1162	7-4	6-0
R	5461	0	5517	0	-1132	7-4	6-0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1162 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT R FOR 1132 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 198 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4320	2622 / 0	780 / 0	0 / 0	0 / -1444	956 / 0	0 / 0
R	4320	2622 / 0	780 / 0	0 / 0	0 / -1423	956 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	141 / -141	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, R

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.47FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT E-AD, H-AC, I-AA, K-Z, L-X, O-W
 2x4 DRY SPF No.2 I-BRACE AT G-AC, H-AA, I-Z, L-Z, M-X

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIVE (LC1)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	-3 / 0	-107.1	-107.1	0.05 (2)	10.00	AG-C	-879 / 298	0.11 (1)
B-AI	-7696 / 1550	-107.1	-107.1	0.21 (2)	4.24	C-AF	-124 / 833	0.13 (3)
AI-C	-11448 / 2418	-107.1	-107.1	0.32 (2)	3.52	AF-E	-160 / 280	0.05 (10)
C-D	-11866 / 2536	-107.1	-107.1	0.33 (2)	3.47	E-AD	-1171 / 310	0.53 (2)
D-E	-11866 / 2536	-107.1	-107.1	0.33 (2)	3.47	AD-G	-95 / 867	0.14 (2)
E-F	-11060 / 2409	-107.1	-107.1	0.27 (2)	3.63	G-AC	-1780 / 471	0.94 (2)
F-G	-11060 / 2409	-107.1	-107.1	0.27 (2)	3.63	AC-H	-270 / 1564	0.25 (2)
G-H	-9982 / 2215	-107.1	-107.1	0.25 (1)	3.80	H-AA	-469 / 2219	0.40 (14)
H-I	-10290 / 2287	-107.1	-107.1	0.26 (1)	3.74	AA-I	-1589 / 473	0.98 (3)
I-J	-10279 / 2219	-107.1	-107.1	0.25 (1)	3.74	I-Z	-736 / 699	0.63 (9)
J-K	-10279 / 2219	-107.1	-107.1	0.25 (1)	3.74	Z-K	-1040 / 359	0.64 (3)
K-L	-10280 / 2220	-107.1	-107.1	0.26 (1)	3.73	L-L	-461 / 2199	0.39 (13)
L-M	-9984 / 2126	-107.1	-107.1	0.25 (1)	3.80	X-L	-284 / 1575	0.25 (3)
M-N	-11060 / 2323	-107.1	-107.1	0.27 (3)	3.63	X-M	-1777 / 487	0.94 (3)
N-O	-11060 / 2323	-107.1	-107.1	0.27 (3)	3.63	W-M	-108 / 864	0.14 (3)
O-P	-11866 / 2454	-107.1	-107.1	0.33 (3)	3.47	W-O	-1172 / 331	0.53 (3)
P-Q	-11866 / 2454	-107.1	-107.1	0.33 (3)	3.47	U-O	-166 / 287	0.05 (9)
Q-AK	-11447 / 2347	-107.1	-107.1	0.32 (3)	3.52	U-Q	-124 / 834	0.13 (2)
AK-R	-7696 / 1524	-107.1	-107.1	0.21 (3)	4.24	T-Q	-879 / 290	0.11 (1)
R-S	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	AH-AI	-9 / 607	0.00 (1)
B-AH	-1236 / 5303	-27.5	-27.5	0.29 (1)	6.25	AI-AG	-1127 / 5626	0.66 (1)
AH-AG	-1236 / 5303	-27.5	-27.5	0.31 (2)	6.25	T-AK	-1072 / 5626	0.66 (1)
AG-AF	-2332 / 10827	-27.5	-27.5	0.54 (1)	6.25	AJ-AK	-5 / 607	0.00 (1)
AF-AE	-2338 / 11274	-27.5	-27.5	0.51 (1)	6.25			
AE-AD	-2338 / 11274	-27.5	-27.5	0.51 (1)	6.25			
AD-AC	-2095 / 10496	-27.5	-27.5	0.47 (1)	6.25			
AC-AB	-1782 / 9456	-27.5	-27.5	0.43 (1)	6.25			
AB-AA	-1782 / 9456	-27.5	-27.5	0.43 (1)	6.25			
AA-Z	-1918 / 10289	-27.5	-27.5	0.46 (1)	6.25			
Z-Y	-1600 / 9457	-27.5	-27.5	0.43 (1)	6.25			
Y-X	-1600 / 9457	-27.5	-27.5	0.43 (1)	6.25			
X-W	-1868 / 10495	-27.5	-27.5	0.47 (1)	6.25			
W-V	-2115 / 11274	-27.5	-27.5	0.51 (1)	6.25			
V-U	-2115 / 11274	-27.5	-27.5	0.51 (1)	6.25			
U-T	-2119 / 10827	-27.5	-27.5	0.54 (1)	6.25			
T-AJ	-1061 / 5303	-27.5	-27.5	0.31 (3)	6.25			
AJ-R	-1061 / 5303	-27.5	-27.5	0.29 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.77")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/893 (1.05")

CSI: TC=0.33 (C-E); BC=0.54 (AF-AG); WB=0.98 (I-AA); SSI=0.28 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Z) (INPUT = 0.90)
 JSI METAL= 0.99 (V) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A3	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324083
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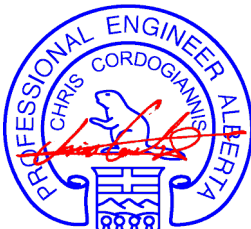
Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:59 2015 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM),INTERNAL
WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON
(OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX
AWAY FROM EAVE.

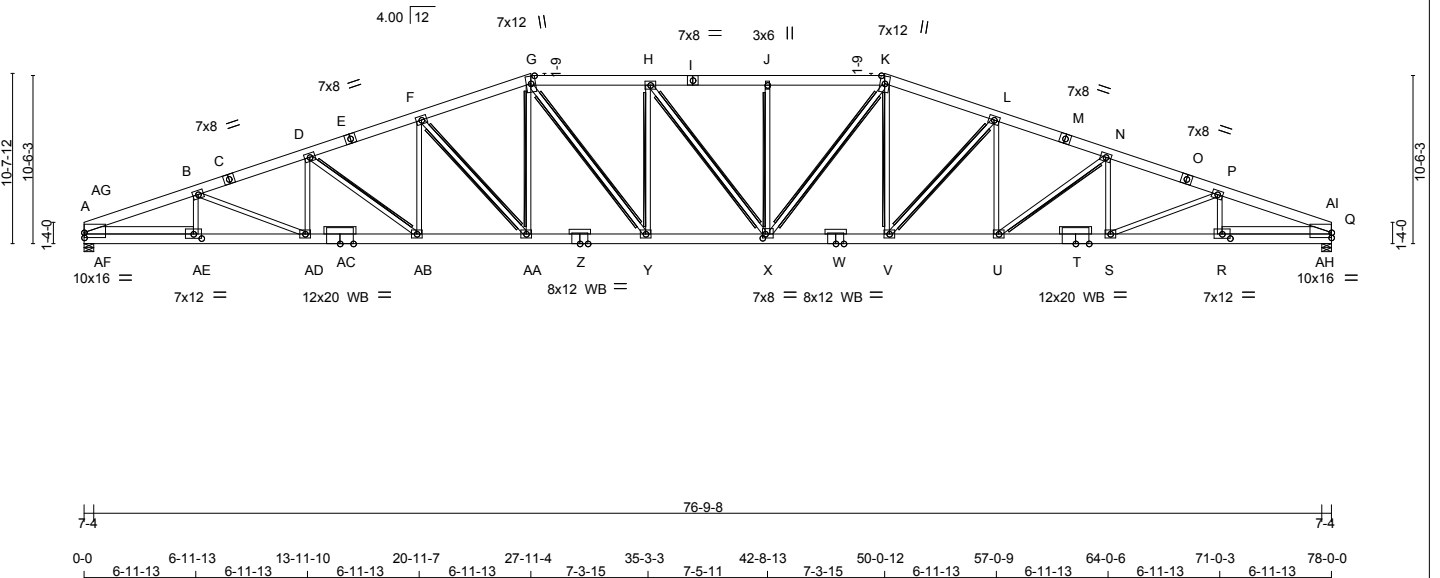
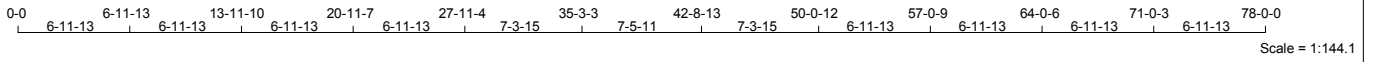
APEGA
Permit No. P3837



August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 584 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - G	2x8	DRY 1950F 1.7E	SPF
G - I	2x8	DRY 1950F 1.7E	SPF
I - K	2x8	DRY 1950F 1.7E	SPF
K - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
O - Q	2x8	DRY 1950F 1.7E	SPF
A - AC	2x8	DRY 1950F 1.7E	SPF
AC - Z	2x8	DRY 1950F 1.7E	SPF
Z - W	2x8	DRY 1950F 1.7E	SPF
W - T	2x8	DRY 1950F 1.7E	SPF
T - Q	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

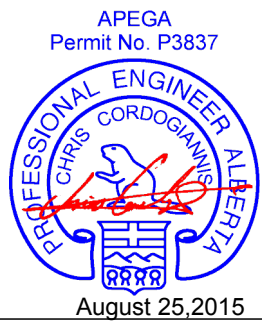
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, F, H, L, N, P						
B	TMWW-t	MT20	6.0	8.0		
C, E, I, M, O						
C	TS-t		7.0	8.0		
G	TTWW+m	MT20	7.0	12.0	Edge	
J	TMW+w		3.0	6.0		
K	TTWW+m	MT20	7.0	12.0	Edge	
Q	TMBMW1-I	MT20	10.0	16.0	3.75	
R	BMWW-t	MT20	7.0	12.0	3.25	6.00
S, U, V, Y, AA, AB, AD						
S	BMWW-t	MT20	6.0	8.0		
T	BS-t	MT20	12.0	20.0		
W	BS-t	MT20	8.0	12.0		
X	BMWW-t	MT20	7.0	8.0	3.00	2.00
Z	BS-t	MT20	8.0	12.0		
AC	BS-t	MT20	12.0	20.0		
AE	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	5249	0	5277	199
Q	5249	0	5277	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1101 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1071 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 199 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS PERM. LIVE	WIND	DEAD	SOIL
A	4176	2478 / 0	780 / 0	0 / 0	0 / -1388	936 / 0	0 / 0
Q	4176	2478 / 0	780 / 0	0 / 0	0 / -1366	936 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	142 / -142	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, Q

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.47FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT D-AB, G-AA, H-Y, J-X, K-V, N-U
 2x4 DRY SPF No.2 I-BRACE AT F-AA, G-Y, H-X, K-X, L-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS		FACTORED		UNBRAC LENGTH	WEBS	
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD (LC1)	MAX (LC)		MEMB. FORCE (LBS)	MAX (LC)
FR-TO			FROM	TO		FR-TO	
A-AG	-7696 / 1550	-107.1	-107.1	0.21 (2)	4.24	AE-B -879 / 298	0.11 (1)
AG-B	-11448 / 2418	-107.1	-107.1	0.32 (2)	3.52	BA-D -124 / 833	0.13 (3)
B-C	-11866 / 2536	-107.1	-107.1	0.33 (2)	3.47	AD-D -160 / 280	0.05 (10)
C-D	-11866 / 2536	-107.1	-107.1	0.33 (2)	3.47	DA-B -1171 / 310	0.53 (2)
D-E	-11060 / 2409	-107.1	-107.1	0.27 (2)	3.63	AB-F -95 / 867	0.14 (2)
E-F	-11060 / 2409	-107.1	-107.1	0.27 (2)	3.63	FA-AA -1780 / 471	0.94 (2)
F-G	-9982 / 2215	-107.1	-107.1	0.25 (1)	3.80	AA-G -270 / 1564	0.25 (2)
G-H	-10290 / 2287	-107.1	-107.1	0.26 (1)	3.74	G-Y -469 / 2219	0.40 (14)
H-I	-10279 / 2219	-107.1	-107.1	0.25 (1)	3.74	Y-H -1589 / 473	0.98 (3)
I-J	-10279 / 2219	-107.1	-107.1	0.25 (1)	3.74	H-X -736 / 699	0.63 (9)
J-K	-10280 / 2220	-107.1	-107.1	0.26 (1)	3.73	X-J -1040 / 359	0.64 (3)
K-L	-9984 / 2126	-107.1	-107.1	0.25 (1)	3.80	X-K -461 / 2199	0.39 (13)
L-M	-11060 / 2323	-107.1	-107.1	0.27 (3)	3.63	V-K -284 / 1575	0.25 (3)
M-N	-11060 / 2323	-107.1	-107.1	0.27 (3)	3.63	V-L -1777 / 487	0.94 (3)
N-O	-11866 / 2454	-107.1	-107.1	0.33 (3)	3.47	U-L -108 / 864	0.14 (3)
O-P	-11866 / 2454	-107.1	-107.1	0.33 (3)	3.47	U-N -1172 / 331	0.53 (3)
P-AI	-11447 / 2347	-107.1	-107.1	0.32 (3)	3.52	S-N -166 / 287	0.05 (9)
AI-Q	-7696 / 1524	-107.1	-107.1	0.21 (3)	4.24	S-P -124 / 834	0.13 (2)
						R-P -879 / 290	0.11 (1)
A-AF	-1235 / 5303	-27.5	-27.5	0.29 (1)	6.25	AF-AG -9 / 607	0.00 (1)
AF-AE	-1235 / 5303	-27.5	-27.5	0.31 (2)	6.25	AG-AE -1127 / 5626	0.66 (1)
AE-AD	-2330 / 10827	-27.5	-27.5	0.54 (1)	6.25	R-AI -1072 / 5626	0.66 (1)
AD-AC	-2337 / 11274	-27.5	-27.5	0.51 (1)	6.25	AH-AI -5 / 607	0.00 (1)
AC-AB	-2337 / 11274	-27.5	-27.5	0.51 (1)	6.25		
AB-AA	-2094 / 10496	-27.5	-27.5	0.47 (1)	6.25		
AA-Z	-1780 / 9456	-27.5	-27.5	0.43 (1)	6.25		
Z-Y	-1780 / 9456	-27.5	-27.5	0.43 (1)	6.25		
Y-X	-1916 / 10289	-27.5	-27.5	0.46 (1)	6.25		
X-W	-1598 / 9457	-27.5	-27.5	0.43 (1)	6.25		
W-V	-1598 / 9457	-27.5	-27.5	0.43 (1)	6.25		
V-U	-1866 / 10495	-27.5	-27.5	0.47 (1)	6.25		
U-T	-2113 / 11274	-27.5	-27.5	0.51 (1)	6.25		
T-S	-2113 / 11274	-27.5	-27.5	0.51 (1)	6.25		
S-R	-2117 / 10827	-27.5	-27.5	0.54 (1)	6.25		
R-AH	-1059 / 5303	-27.5	-27.5	0.31 (3)	6.25		
AH-Q	-1059 / 5303	-27.5	-27.5	0.29 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.77")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/893 (1.05")

CSI: TC=0.33 (B-D:2), BC=0.54 (AD-AE:1),
 WB=0.98 (H-Y:3), SSI=0.28 (G-H:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)
MT20	618	354	1667	822
			2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)

JSI METAL= 0.99 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A3A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324084
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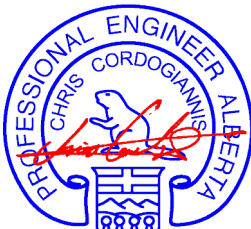
Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:02 2015 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX
AWAY FROM EAVE.

APEGA
Permit No. P3837



August 25, 2015

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JOB NAME G1507147	TRUSS NAME A4	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324085
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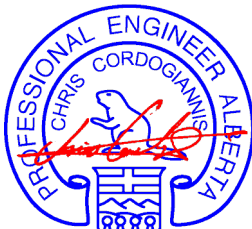
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Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:05 2015 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX
AWAY FROM EAVE.

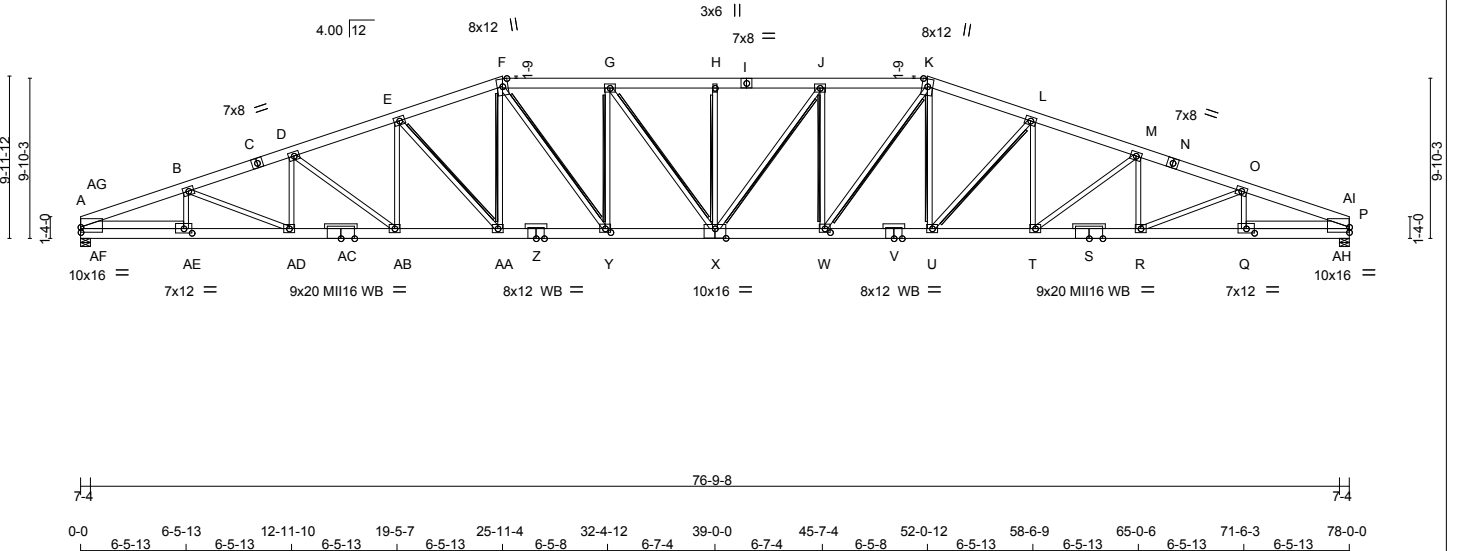
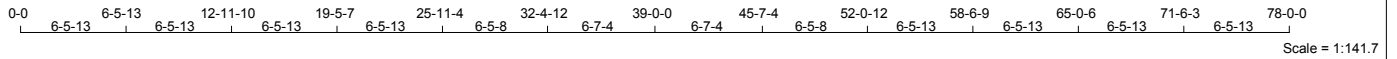
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TOTAL WEIGHT = 588 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - F	2x8	DRY 1950F 1.7E	SPF
F - I	2x8	DRY 1950F 1.7E	SPF
I - K	2x8	DRY 1950F 1.7E	SPF
K - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
A - AC	2x8	DRY 1950F 1.7E	SPF
AC - Z	2x8	DRY 1950F 1.7E	SPF
Z - X	2x8	DRY 1950F 1.7E	SPF
X - V	2x8	DRY 1950F 1.7E	SPF
V - S	2x8	DRY 1950F 1.7E	SPF
S - P	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	TYPE	QTY	DESCR.
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

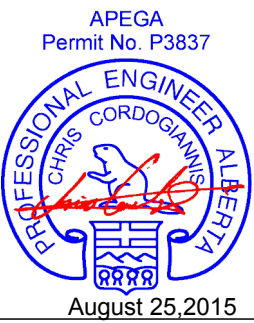
ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, E, G, J, L, M, O						
B	TMWW-t	MT20	6.0	8.0		
C, I, N						
C	TS-t	MT20	7.0	8.0		
F	TTWW+m	MT20	8.0	12.0	Edge	
H	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	8.0	12.0	Edge	
P	TMBMW1-I	MT20	10.0	16.0	3.75	
Q	BMWW-t	MT20	7.0	12.0	3.25	6.00
R, T, U, AA, AB, AD						
R	BMWW-t	MT20	6.0	8.0		
S	BS-t	MI16	9.0	20.0		
V	BS-t	MT20	8.0	12.0		
W	BMWW-t	MT20	6.0	8.0	2.75	4.00
X	BSWWW-I	MT20	10.0	16.0	7.00	8.00
Y	BMWW-t	MT20	6.0	8.0	2.75	4.00
Z	BS-t	MT20	8.0	12.0		
AC	BS-t	MI16	9.0	20.0		
AE	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
 WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
A	5249	0	5353	186
P	5249	0	5353	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1112 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1086 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 186 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	4176	2528 / 0	780 / 0	0 / 0	0 / -1396	936 / 0
P	4176	2528 / 0	780 / 0	0 / 0	0 / -1377	936 / 0

HORIZONTAL REACTIONS

JT	VERT	HORZ	UPLIFT	IN-SX	IN-SY
A	0	0	0	133	-133
P	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, P

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.45FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT E-AA, F-AA, F-Y, G-Y, G-X, H-X, J-X, J-W, K-W, K-U, L-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LIVE (LC1)	MAX. UNBRAC (LC)	MEMB. FORCE (LBS)	MAX. FACTORED (LC)
FR-TO		FROM	TO	LENGTH	FR-TO	
A-AG	-7719 / 1557	-107.1	-107.1	0.20 (2)	4.24	AE-B -976 / 311
AG-B	-11432 / 2411	-107.1	-107.1	0.30 (2)	3.53	B-AD -160 / 988
B-C	-11941 / 2574	-107.1	-107.1	0.32 (2)	3.45	AD-D -223 / 217
C-D	-11941 / 2574	-107.1	-107.1	0.32 (2)	3.45	D-AB -967 / 264
D-E	-11258 / 2478	-107.1	-107.1	0.26 (2)	3.61	AB-E -73 / 729
E-F	-10320 / 2312	-107.1	-107.1	0.25 (2)	3.75	E-AA -1550 / 415
F-G	-10800 / 2429	-107.1	-107.1	0.25 (1)	3.67	AA-F -240 / 1383
G-H	-11091 / 2438	-107.1	-107.1	0.24 (1)	3.64	F-Y -531 / 2527
H-I	-11091 / 2438	-107.1	-107.1	0.24 (1)	3.64	Y-G -1889 / 517
I-J	-11091 / 2438	-107.1	-107.1	0.24 (1)	3.64	G-X -225 / 1139
J-K	-10800 / 2338	-107.1	-107.1	0.25 (1)	3.67	X-H -765 / 266
K-L	-10320 / 2236	-107.1	-107.1	0.25 (3)	3.75	X-J -226 / 1139
L-M	-11258 / 2404	-107.1	-107.1	0.26 (3)	3.61	W-J -1889 / 517
M-N	-11941 / 2504	-107.1	-107.1	0.32 (3)	3.45	W-K -531 / 2527
N-O	-11941 / 2504	-107.1	-107.1	0.32 (3)	3.45	U-K -255 / 1383
O-AI	-11432 / 2351	-107.1	-107.1	0.30 (3)	3.53	U-L -1550 / 439
AI-P	-7719 / 1536	-107.1	-107.1	0.20 (3)	4.24	T-L -91 / 729
						T-M -967 / 291
A-AF	-1219 / 5286	-27.5	-27.5	0.29 (2)	6.25	R-M -232 / 225
AF-AE	-1219 / 5286	-27.5	-27.5	0.31 (2)	6.25	R-O -160 / 988
AE-AD	-2318 / 10809	-27.5	-27.5	0.54 (2)	6.25	Q-O -976 / 303
AD-AC	-2376 / 11347	-27.5	-27.5	0.52 (2)	6.25	AF-AG -17 / 566
AC-AB	-2376 / 11347	-27.5	-27.5	0.52 (2)	6.25	AG-AE -1134 / 5606
AB-AA	-2171 / 10683	-27.5	-27.5	0.47 (1)	6.25	Q-AI -1087 / 5606
AA-Z	-1895 / 9783	-27.5	-27.5	0.44 (1)	6.25	AH-AI -14 / 566
Z-Y	-1895 / 9783	-27.5	-27.5	0.44 (1)	6.25	
Y-X	-2083 / 10799	-27.5	-27.5	0.49 (1)	6.25	
X-W	-1981 / 10799	-27.5	-27.5	0.49 (1)	6.25	
W-V	-1696 / 9783	-27.5	-27.5	0.44 (1)	6.25	
V-U	-1696 / 9783	-27.5	-27.5	0.44 (1)	6.25	
U-T	-1965 / 10683	-27.5	-27.5	0.47 (1)	6.25	
T-S	-2174 / 11347	-27.5	-27.5	0.52 (3)	6.25	
S-R	-2174 / 11347	-27.5	-27.5	0.52 (3)	6.25	
R-Q	-2125 / 10809	-27.5	-27.5	0.54 (3)	6.25	
Q-AH	-1055 / 5286	-27.5	-27.5	0.31 (3)	6.25	
AH-P	-1055 / 5286	-27.5	-27.5	0.29 (3)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBC2010 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.82")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/840 (1.11")

CSI: TC=0.32 (M-O-3), BC=0.54 (Q-R-3),
 WB=0.97 (G-Y-3), SSI=0.25 (A-AG-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)	SECTION (PSI)	(PLI)
MT20	618	354	1667	822	2284	1656
MI16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Y) (INPUT = 0.90)
 JSI METAL= 0.99 (X) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A4A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324086
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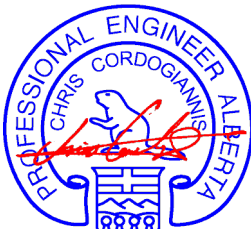
Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:08 2015 Page 2
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TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX
AWAY FROM EAVE.

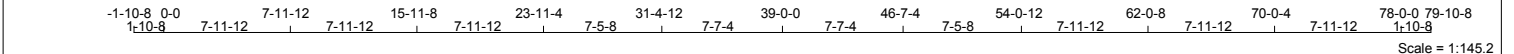
APEGA
Permit No. P3837



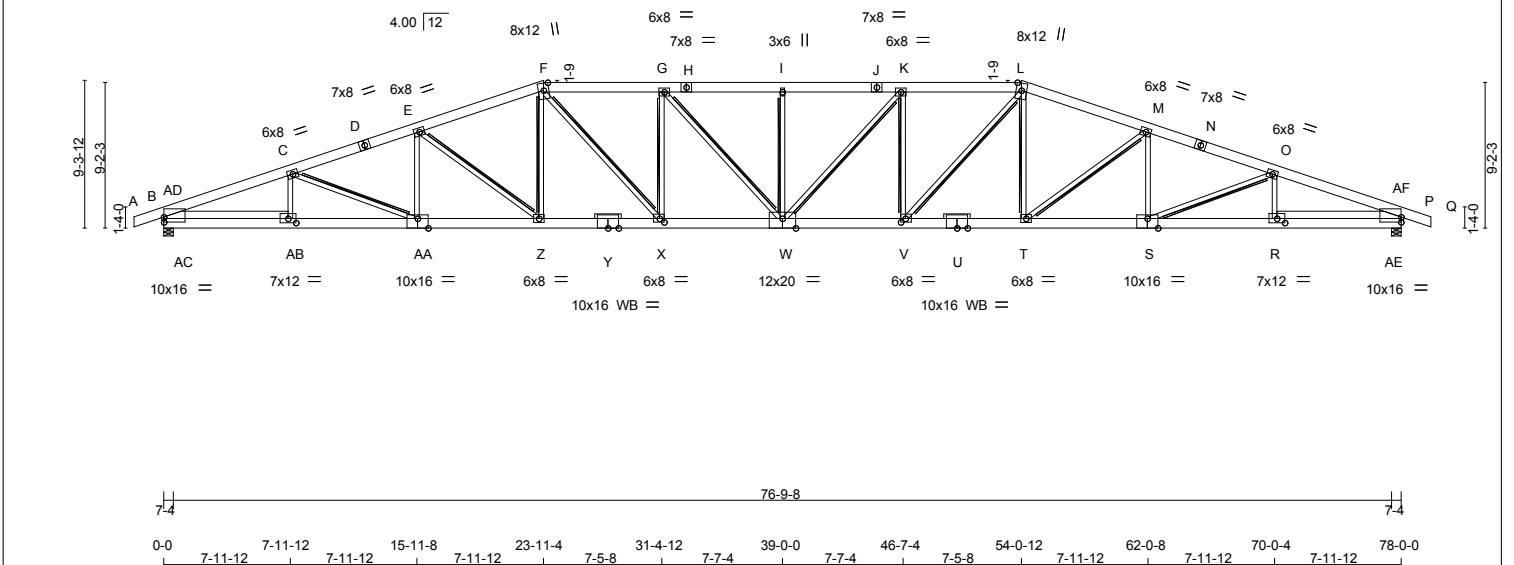
August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:145.2



TOTAL WEIGHT = 571 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - J	2x8	DRY 1950F 1.7E	SPF
J - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - Q	2x8	DRY 1950F 1.7E	SPF
B - AA	2x8	DRY 1950F 1.7E	SPF
AA - Y	2x8	DRY 1950F 1.7E	SPF
Y - W	2x8	DRY 1950F 1.7E	SPF
W - U	2x8	DRY 1950F 1.7E	SPF
U - S	2x8	DRY 1950F 1.7E	SPF
S - P	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

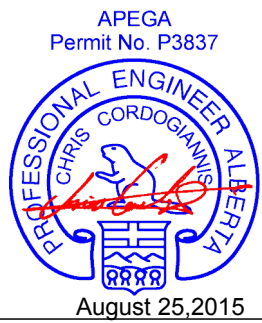
ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, G, K, M, O						
C	TMWW-t	MT20	6.0	8.0		
D, H, J, N						
D	TS-t	MT20	7.0	8.0		
F	TTWW+m	MT20	8.0	12.0	Edge	
I	TMW+w	MT20	3.0	6.0		
L	TTWW+m	MT20	8.0	12.0	Edge	
P	TMBMW1-I	MT20	10.0	16.0	3.75	
R	BMWW-t	MT20	7.0	12.0	3.25	6.00
S	BSWW-I	MT20	10.0	16.0	Edge	8.00
T	BMWW-t	MT20	6.0	8.0		
U	BS-t	MT20	10.0	16.0		
V	BMWW-t	MT20	6.0	8.0	2.75	3.50
W	BSWW-I	MT20	12.0	20.0	Edge	10.00
X	BMWW-t	MT20	6.0	8.0	2.75	3.50
Y	BS-t	MT20	10.0	16.0		
Z	BMWW-t	MT20	6.0	8.0		
AA	BSWW-I	MT20	10.0	16.0	Edge	8.00
AB	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	5461	0	5663	171	-1184	7-4	6-2
P	5461	0	5663	0	-1161	7-4	6-2

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1184 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1161 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 171 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	4320	2719 / 0	780 / 0	0 / 0	0 / -1460	956 / 0	0 / 0
P	4320	2719 / 0	780 / 0	0 / 0	0 / -1444	956 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	122 / -122	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, P

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.39FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x3 DRY SPF No.2 T-BRACE AT C-AA, F-Z, I-W, L-T, O-S
2x4 DRY SPF No.2 T-BRACE AT E-Z, F-X, G-X, G-W, K-W, K-V, L-V, M-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	C H O R D S		W E B S	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX VERT. LOAD (PLF)	MAX UNBRAC LENGTH
FR-TO				
A-B	-3 / 0	-107.1	-107.1	0.05 (2)
B-AD	-8066 / 1601	-107.1	-107.1	0.25 (2)
AD-C	-11985 / 2518	-107.1	-107.1	0.39 (2)
C-D	-11902 / 2577	-107.1	-107.1	0.38 (2)
D-E	-11902 / 2577	-107.1	-107.1	0.38 (2)
E-F	-10668 / 2388	-107.1	-107.1	0.34 (2)
F-G	-11545 / 2608	-107.1	-107.1	0.32 (2)
G-H	-11970 / 2642	-107.1	-107.1	0.33 (2)
H-I	-11970 / 2642	-107.1	-107.1	0.33 (2)
I-J	-11970 / 2642	-107.1	-107.1	0.33 (3)
J-K	-11970 / 2642	-107.1	-107.1	0.33 (3)
K-L	-11545 / 2526	-107.1	-107.1	0.32 (3)
L-M	-10668 / 2323	-107.1	-107.1	0.34 (3)
M-N	-11902 / 2516	-107.1	-107.1	0.38 (3)
N-O	-11902 / 2516	-107.1	-107.1	0.38 (3)
O-AF	-11985 / 2464	-107.1	-107.1	0.39 (3)
AF-P	-8066 / 1584	-107.1	-107.1	0.25 (3)
P-Q	-3 / 0	-107.1	-107.1	0.05 (3)
B-AE	-1284 / 5633	-27.5	-27.5	0.32 (2)
AC-AB	-1284 / 5633	-27.5	-27.5	0.32 (2)
AB-AA	-2399 / 11340	-27.5	-27.5	0.57 (2)
AA-Z	-2335 / 11318	-27.5	-27.5	0.50 (2)
Z-Y	-2003 / 10102	-27.5	-27.5	0.45 (1)
Y-X	-2003 / 10102	-27.5	-27.5	0.45 (1)
X-W	-2288 / 11544	-27.5	-27.5	0.52 (1)
W-V	-2180 / 11544	-27.5	-27.5	0.52 (1)
V-U	-1816 / 10102	-27.5	-27.5	0.45 (1)
U-T	-1816 / 10102	-27.5	-27.5	0.45 (1)
T-S	-2151 / 11318	-27.5	-27.5	0.50 (3)
S-R	-2222 / 11340	-27.5	-27.5	0.57 (3)
R-AE	-1133 / 5633	-27.5	-27.5	0.32 (3)
AE-P	-1133 / 5633	-27.5	-27.5	0.32 (3)
AB-C	-1284 / 5633	-27.5	-27.5	0.32 (2)
AC-AA	-2399 / 11340	-27.5	-27.5	0.57 (2)
AA-Z	-2335 / 11318	-27.5	-27.5	0.50 (2)
Z-Y	-2003 / 10102	-27.5	-27.5	0.45 (1)
Y-X	-2003 / 10102	-27.5	-27.5	0.45 (1)
X-W	-2288 / 11544	-27.5	-27.5	0.52 (1)
W-V	-2180 / 11544	-27.5	-27.5	0.52 (1)
V-U	-1816 / 10102	-27.5	-27.5	0.45 (1)
U-T	-1816 / 10102	-27.5	-27.5	0.45 (1)
T-S	-2151 / 11318	-27.5	-27.5	0.50 (3)
S-R	-2222 / 11340	-27.5	-27.5	0.57 (3)
R-AE	-1133 / 5633	-27.5	-27.5	0.32 (3)
AE-P	-1133 / 5633	-27.5	-27.5	0.32 (3)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
DL = 5.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
CALCULATED VERT. DEFL.(LL) = L/999 (0.89")
ALLOWABLE DEFL.(TL)= L/240 (3.90")
CALCULATED VERT. DEFL.(TL) = L/777 (1.21")

CSI: TC=0.39 (C-AD:2), BC=0.57 (AA-AB:2),
WB=0.94 (M-T:3), SSI=0.34 (B-AD:2)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)
MT20	618	354	1667	822
	2284	1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)
JSI METAL= 1.00 (AA) (INPUT = 1.00)

CONTINUED ON PAGE 2

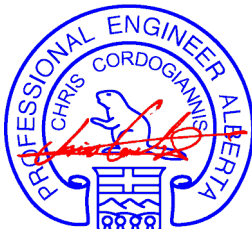
JOB NAME G1507147	TRUSS NAME A5	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324087
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:11 2015 Page 2
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WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

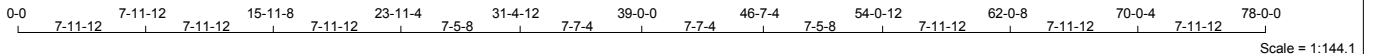
APEGA
Permit No. P3837



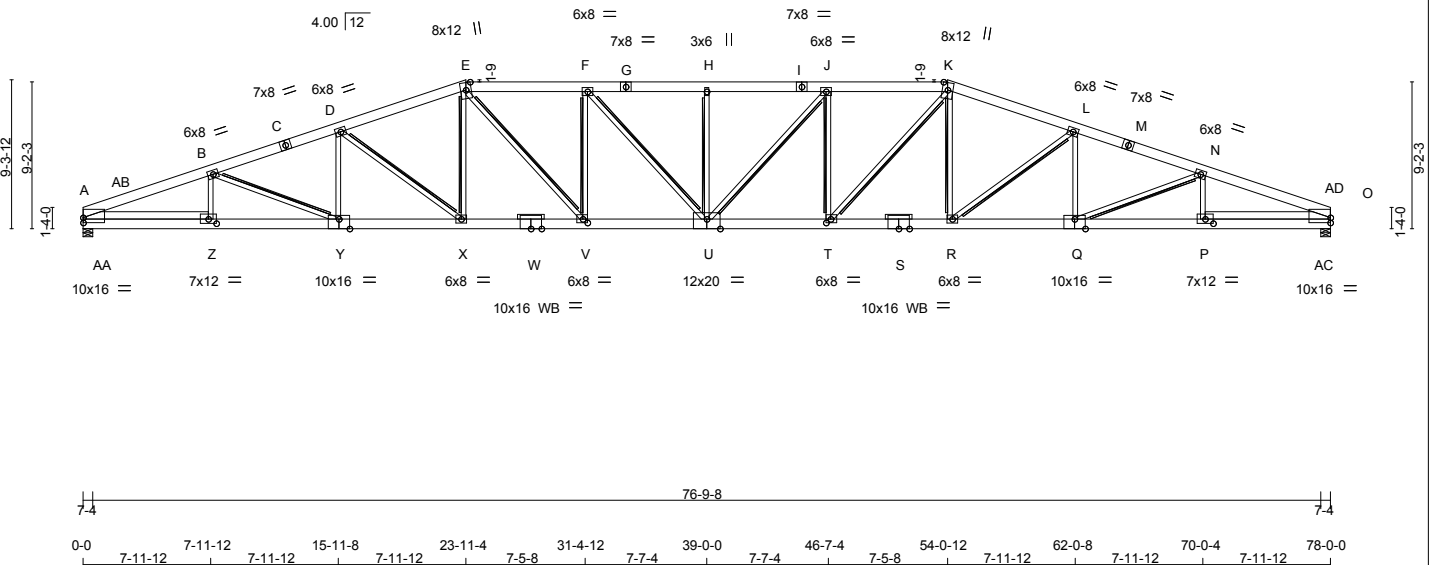
August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:144.1



TOTAL WEIGHT = 561 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - G	2x8	DRY 1950F 1.7E	SPF
G - I	2x8	DRY 1950F 1.7E	SPF
I - K	2x8	DRY 1950F 1.7E	SPF
K - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
A - Y	2x8	DRY 1950F 1.7E	SPF
Y - W	2x8	DRY 1950F 1.7E	SPF
W - U	2x8	DRY 1950F 1.7E	SPF
U - S	2x8	DRY 1950F 1.7E	SPF
S - Q	2x8	DRY 1950F 1.7E	SPF
Q - O	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

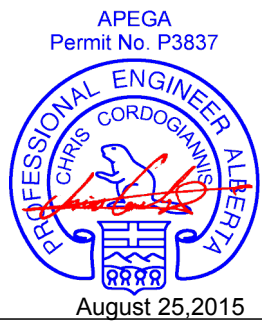
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, F, J, L, N						
B	TMWW-t	MT20	6.0	8.0		
C, G, I, M						
C	TS-t	MT20	7.0	8.0		
E	TTWW+m	MT20	8.0	12.0	Edge	
H	TMW+w	MT20	3.0	6.0		
K	TTWW+m	MT20	8.0	12.0	Edge	
O	TMBMW1-I	MT20	10.0	16.0	3.75	
P	BMWW-t	MT20	7.0	12.0	3.25	6.00
Q	BSWW-I	MT20	10.0	16.0	Edge	8.00
R	BMWW-t	MT20	6.0	8.0		
S	BS-t	MT20	10.0	16.0		
T	BMWW-t	MT20	6.0	8.0	2.75	3.50
U	BSWW-I	MT20	12.0	20.0	Edge	10.00
V	BMWW-t	MT20	6.0	8.0	2.75	3.50
W	BS-t	MT20	10.0	16.0		
X	BMWW-t	MT20	6.0	8.0		
Y	BSWW-I	MT20	10.0	16.0	Edge	8.00
Z	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
A	5249	0	5423	172
O	5249	0	5423	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1123 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT O FOR 1100 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 172 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	4176	2575 / 0	780 / 0	0 / 0	0 / -1404	936 / 0	0 / 0
O	4176	2575 / 0	780 / 0	0 / 0	0 / -1388	936 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	123 / -123	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, O

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.39FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x3 DRY SPF No.2 T-BRACE AT B-Y, E-X, H-U, K-R, N-Q
 2x4 DRY SPF No.2 T-BRACE AT D-X, E-V, F-V, F-U, J-U, J-T, K-T, L-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-AB	-8066 / 1601	-107.1	-107.1	0.25 (2)	4.11	Z-B	-723 / 283
AB-B	-11985 / 2518	-107.1	-107.1	0.39 (2)	3.39	B-Y	-103 / 757
B-C	-11902 / 2577	-107.1	-107.1	0.38 (2)	3.42	Y-D	-142 / 393
C-D	-11902 / 2577	-107.1	-107.1	0.38 (2)	3.42	D-X	-1522 / 419
D-E	-10668 / 2388	-107.1	-107.1	0.34 (2)	3.61	X-E	-166 / 1167
E-F	-11545 / 2608	-107.1	-107.1	0.32 (2)	3.52	E-V	-623 / 2960
F-G	-11970 / 2642	-107.1	-107.1	0.33 (2)	3.46	V-F	-2035 / 567
G-H	-11970 / 2642	-107.1	-107.1	0.33 (2)	3.46	F-U	-247 / 1248
H-I	-11970 / 2642	-107.1	-107.1	0.33 (3)	3.46	U-H	-878 / 307
I-J	-11970 / 2642	-107.1	-107.1	0.33 (3)	3.46	J-U	-247 / 1248
J-K	-11545 / 2526	-107.1	-107.1	0.32 (3)	3.52	T-J	-2035 / 567
K-L	-10668 / 2323	-107.1	-107.1	0.34 (3)	3.61	T-K	-623 / 2960
L-M	-11902 / 2516	-107.1	-107.1	0.38 (3)	3.42	R-K	-184 / 1167
M-N	-11902 / 2516	-107.1	-107.1	0.38 (3)	3.42	R-L	-1522 / 453
N-AD	-11985 / 2464	-107.1	-107.1	0.39 (3)	3.39	Q-L	-150 / 393
AD-O	-8066 / 1584	-107.1	-107.1	0.25 (3)	4.11	Q-N	-102 / 779
						P-N	-723 / 278
A-AA	-1283 / 5633	-27.5	-27.5	0.32 (2)	6.25	AA-AB	0 / 667
AA-Z	-1283 / 5633	-27.5	-27.5	0.32 (2)	6.25	AB-Z	-1143 / 5762
Z-Y	-2397 / 11340	-27.5	-27.5	0.57 (2)	6.25	P-AD	-1099 / 5762
Y-X	-2333 / 11318	-27.5	-27.5	0.50 (2)	6.25	AC-AD	0 / 667
X-W	-2001 / 10102	-27.5	-27.5	0.45 (1)	6.25		
W-V	-2001 / 10102	-27.5	-27.5	0.45 (1)	6.25		
V-U	-2287 / 11544	-27.5	-27.5	0.52 (1)	6.25		
U-T	-2179 / 11544	-27.5	-27.5	0.52 (1)	6.25		
T-S	-1814 / 10102	-27.5	-27.5	0.45 (1)	6.25		
S-R	-1814 / 10102	-27.5	-27.5	0.45 (1)	6.25		
R-Q	-2149 / 11318	-27.5	-27.5	0.50 (3)	6.25		
Q-P	-2220 / 11340	-27.5	-27.5	0.57 (3)	6.25		
P-AC	-1131 / 5633	-27.5	-27.5	0.32 (3)	6.25		
AC-O	-1131 / 5633	-27.5	-27.5	0.32 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.89")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/777 (1.21")

CSI: TC=0.39 (B-AB:2), BC=0.57 (P-Q:3),
 WB=0.94 (L-R:3), SSI=0.34 (A-AB:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
 JSI METAL= 1.00 (Y) (INPUT = 1.00)

CONTINUED ON PAGE 2

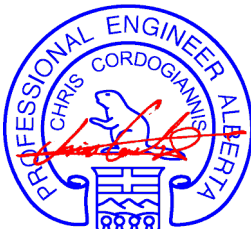
JOB NAME G1507147	TRUSS NAME A5A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324088
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:14 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYiyi3SS-jOLFu5Ek7oVn9NtyAKdo4CMSkk?PXnMM__tJXykfkN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

APEGA
Permit No. P3837

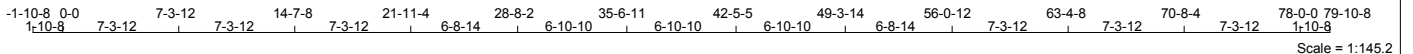


August 25, 2015

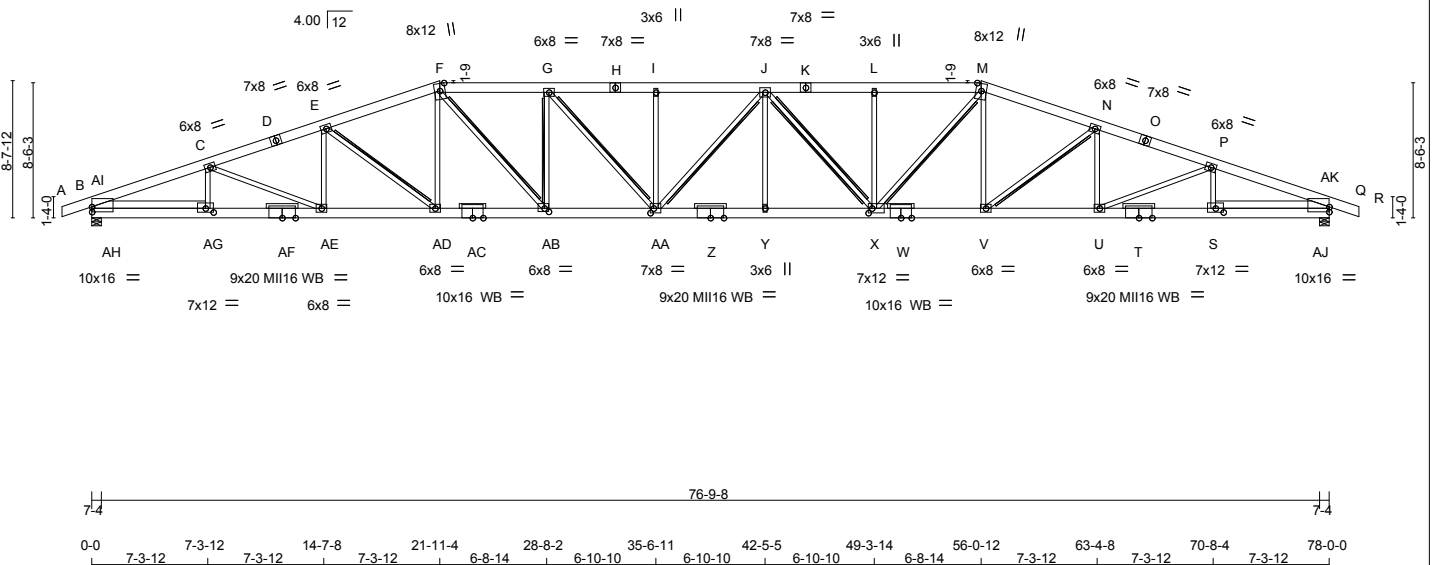
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.



Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:16 2015 Page 1
 ID:Uqb72Eaw6FPE10XPJ82uYjy13SS-fnT?JmF_fPIUOg1KlIfmtVHjUXOEtLfqHT_NQyKfkl



Scale = 1:145.2



TOTAL WEIGHT = 581 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - K	2x8	DRY 1950F 1.7E	SPF
K - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
O - R	2x8	DRY 1950F 1.7E	SPF
B - AF	2x8	DRY 1950F 1.7E	SPF
AF - AC	2x8	DRY 1950F 1.7E	SPF
AC - Z	2x8	DRY 1950F 1.7E	SPF
Z - W	2x8	DRY 1950F 1.7E	SPF
W - T	2x8	DRY 1950F 1.7E	SPF
T - Q	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, G, N, P						
C	TMWW-t	MT20	6.0	8.0		
D, H, K, O						
D	TS-t	MT20	7.0	8.0		
F	TTWW+m	MT20	8.0	12.0	Edge	
I	TMW+w	MT20	3.0	6.0		
J	TMWW-t	MT20	7.0	8.0		
L	TMW+w	MT20	3.0	6.0		
M	TTWW+m	MT20	8.0	12.0	Edge	
Q	TMBMW1-I	MT20	10.0	16.0	3.75	
S	BMWW-t	MT20	7.0	12.0	3.00	6.00
T, Z, AF						
T	BS-t	MII16	9.0	20.0		
U, V, AD, AE						
U	BMWW-t	MT20	6.0	8.0		
W	BS-t	MT20	10.0	16.0		
X	BMWW-t	MT20	7.0	12.0	3.50	2.75
Y	BMW+w	MT20	3.0	6.0		
AA	BMWW-t	MT20	7.0	8.0	3.50	2.50
AB	BMWW-t	MT20	6.0	8.0	2.50	3.25
AC	BS-t	MT20	10.0	16.0		
AG	BMWW-t	MT20	7.0	12.0	3.00	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT				
B	5461 0	5727 158	-1193 7-4	6-3
Q	5461 0	5727 0	-1174 7-4	6-3

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1193 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1174 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 158 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
B	4320	2762 / 0	780 / 0	0 / 0	0 / -1467	956 / 0	0 / 0
Q	4320	2762 / 0	780 / 0	0 / 0	0 / -1453	956 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	113 / -113	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.36FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT E-AD, F-AB, G-AB, G-AA, J-AA, M-X, N-V
 2x4 DRY SPF No.2 I-BRACE AT J-X

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX VERT. LOAD (PLF)	MAX UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX FACTORED (LBS)
FR-TO						
A-B	-3 / 0	-107.1	-107.1	0.05 (2)	10.00	AG-C -845 / 295
B-AI	-8057 / 1606	-107.1	-107.1	0.23 (2)	4.13	CA-E -144 / 931
AI-C	-11989 / 2509	-107.1	-107.1	0.35 (2)	3.41	AE-E -220 / 302
C-D	-12203 / 2621	-107.1	-107.1	0.34 (2)	3.41	E-AD -1233 / 352
D-E	-12203 / 2621	-107.1	-107.1	0.34 (2)	3.41	AD-F -137 / 990
E-F	-11202 / 2475	-107.1	-107.1	0.31 (2)	3.56	F-AB -691 / 3303
F-G	-12183 / 2748	-107.1	-107.1	0.33 (2)	3.43	AB-G -2342 / 614
G-H	-12865 / 2873	-107.1	-107.1	0.33 (2)	3.36	GA-A -354 / 1774
H-I	-12865 / 2873	-107.1	-107.1	0.33 (2)	3.36	AA-I -809 / 282
I-J	-12865 / 2873	-107.1	-107.1	0.32 (2)	3.36	AA-J -493 / 439
J-K	-12172 / 2673	-107.1	-107.1	0.30 (3)	3.45	Y-J 0 / 327
K-L	-12172 / 2673	-107.1	-107.1	0.30 (3)	3.45	J-X -1818 / 375
L-M	-12173 / 2675	-107.1	-107.1	0.33 (3)	3.42	X-L -987 / 337
M-N	-11203 / 2422	-107.1	-107.1	0.31 (3)	3.56	X-M -682 / 3284
N-O	-12202 / 2570	-107.1	-107.1	0.34 (3)	3.41	V-M -163 / 997
O-P	-12202 / 2570	-107.1	-107.1	0.34 (3)	3.41	V-N -1231 / 394
P-AK	-11989 / 2464	-107.1	-107.1	0.35 (3)	3.41	U-N -233 / 312
AK-Q	-8056 / 1593	-107.1	-107.1	0.23 (3)	4.13	U-P -143 / 930
Q-R	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	S-P -844 / 290
B-AH	-1259 / 5578	-27.5	-27.5	0.31 (2)	6.25	AH-AI -8 / 624
AH-AG	-1259 / 5578	-27.5	-27.5	0.32 (2)	6.25	AI-AG -1158 / 5828
AG-AF	-2385 / 11339	-27.5	-27.5	0.57 (2)	6.25	S-AK -1121 / 5828
AF-AE	-2385 / 11339	-27.5	-27.5	0.57 (2)	6.25	AJ-AK 0 / 623
AE-AD	-2384 / 11601	-27.5	-27.5	0.51 (2)	6.25	
AD-AC	-2107 / 10611	-27.5	-27.5	0.47 (2)	6.25	
AC-AB	-2107 / 10611	-27.5	-27.5	0.47 (2)	6.25	
AB-AA	-2453 / 12182	-27.5	-27.5	0.55 (2)	6.25	
AA-Z	-2545 / 12881	-27.5	-27.5	0.58 (1)	6.25	
Z-Y	-2545 / 12881	-27.5	-27.5	0.58 (1)	6.25	
Y-X	-2545 / 12881	-27.5	-27.5	0.58 (1)	6.25	
X-W	-1940 / 10612	-27.5	-27.5	0.47 (3)	6.25	
W-V	-1940 / 10612	-27.5	-27.5	0.47 (3)	6.25	
V-U	-2220 / 11601	-27.5	-27.5	0.51 (3)	6.25	
U-T	-2227 / 11340	-27.5	-27.5	0.57 (3)	6.25	
T-S	-2227 / 11340	-27.5	-27.5	0.57 (3)	6.25	
S-AJ	-1119 / 5578	-27.5	-27.5	0.32 (3)	6.25	
AJ-Q	-1119 / 5578	-27.5	-27.5	0.31 (3)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/968 (0.97")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/712 (1.32")

CSI: TC=0.35 (P-AK:3), BC=0.58 (Y-AA:1),
 WB=1.00 (J-X:2), SSI=0.31 (Q-AK:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

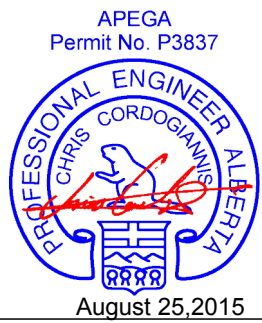
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PLI)
	MAX	MIN	MAX
MT20	618	354	1667 822 2284 1656
MII16	473	276	2341 1245 4454 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)
 JSI METAL= 0.97 (Q) (INPUT = 1.00)



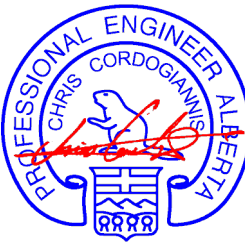
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JOB NAME G1507147	TRUSS NAME A6	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324089
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:16 2015 Page 2
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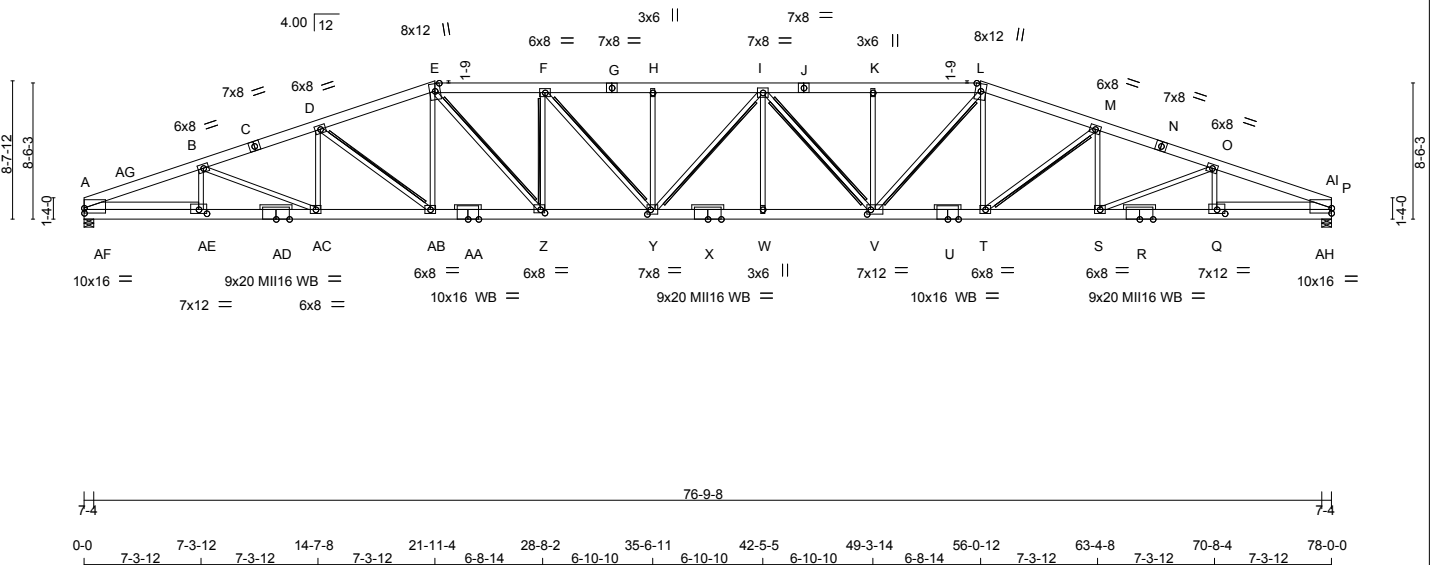
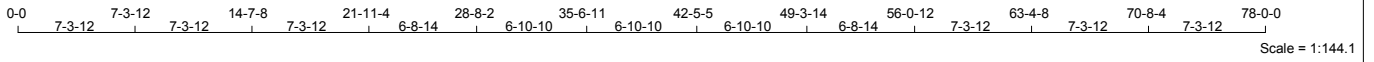
<p>WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.</p>	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM);INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>	
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APEGA
Permit No. P3837



August 25, 2015

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	 100 Industrial Road Bradford, ON L3Z 3G7
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TOTAL WEIGHT = 571 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY	1950F 1.7E	SPF
C - E	2x8	DRY	1950F 1.7E	SPF
E - G	2x8	DRY	1950F 1.7E	SPF
G - J	2x8	DRY	1950F 1.7E	SPF
J - L	2x8	DRY	1950F 1.7E	SPF
L - N	2x8	DRY	1950F 1.7E	SPF
N - P	2x8	DRY	1950F 1.7E	SPF
A - AD	2x8	DRY	1950F 1.7E	SPF
AD - AA	2x8	DRY	1950F 1.7E	SPF
AA - X	2x8	DRY	1950F 1.7E	SPF
X - U	2x8	DRY	1950F 1.7E	SPF
U - R	2x8	DRY	1950F 1.7E	SPF
R - P	2x8	DRY	1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, F, M, O						
B	TMWW-t	MT20	6.0	8.0		
C, G, J, N						
C	TS-t	MT20	7.0	8.0		
E	TTWW+m	MT20	8.0	12.0	Edge	
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	7.0	8.0		
K	TMW+w	MT20	3.0	6.0		
L	TTWW+m	MT20	8.0	12.0	Edge	
P	TMBMW1-I	MT20	10.0	16.0	3.75	
Q	BMWW-t	MT20	7.0	12.0	3.00	6.00
R, X, AD						
R	BS-t	MII16	9.0	20.0		
S, T, AB, AC						
S	BMWW-t	MT20	6.0	8.0		
U	BS-t	MT20	10.0	16.0		
V	BMWW-t	MT20	7.0	12.0	3.50	2.75
W	BMW+w	MT20	3.0	6.0		
Y	BMWW-t	MT20	7.0	8.0	3.50	2.50
Z	BMWW-t	MT20	6.0	8.0	2.50	3.25
AA	BS-t	MT20	10.0	16.0		
AE	BMWW-t	MT20	7.0	12.0	3.00	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
 WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
A	5249	0	5487	159
P	5249	0	5487	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1132 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1113 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 159 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	4176	2618 / 0	780 / 0	0 / 0	0 / -1410	936 / 0	0 / 0
P	4176	2618 / 0	780 / 0	0 / 0	0 / -1397	936 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	114 / -114	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, P

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 3.36FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT D-AB, E-Z, F-Z, F-Y, I-Y, L-V, M-T
 2x4 DRY SPF No.2 I-BRACE AT I-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S			W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIVE (PLF)	MAX UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO		
A-AG	-8057 / 1606	-107.1	-107.1	0.23 (2)	4.13	AE-B -845 / 295	0.11 (2)
AG-B	-11989 / 2509	-107.1	-107.1	0.35 (2)	3.41	B-AC -144 / 931	0.15 (3)
B-C	-12203 / 2621	-107.1	-107.1	0.34 (2)	3.41	AC-D -220 / 302	0.07 (10)
C-D	-12203 / 2621	-107.1	-107.1	0.34 (2)	3.41	D-AB -1233 / 352	0.61 (2)
D-E	-11202 / 2475	-107.1	-107.1	0.31 (2)	3.56	AB-E -137 / 990	0.16 (2)
E-F	-12183 / 2748	-107.1	-107.1	0.33 (2)	3.43	E-Z -691 / 3303	0.53 (3)
F-G	-12865 / 2873	-107.1	-107.1	0.33 (2)	3.36	Z-F -2342 / 614	0.82 (3)
G-H	-12865 / 2873	-107.1	-107.1	0.33 (2)	3.36	F-Y -354 / 1774	0.29 (3)
H-I	-12865 / 2873	-107.1	-107.1	0.32 (2)	3.36	Y-H -809 / 282	0.68 (3)
I-J	-12172 / 2673	-107.1	-107.1	0.30 (3)	3.45	Y-I -493 / 439	0.34 (10)
J-K	-12172 / 2673	-107.1	-107.1	0.30 (3)	3.45	W-I 0 / 327	0.05 (17)
K-L	-12173 / 2675	-107.1	-107.1	0.33 (3)	3.42	I-V -1818 / 375	1.00 (2)
L-M	-11203 / 2422	-107.1	-107.1	0.31 (3)	3.56	V-K -987 / 337	0.83 (3)
M-N	-12202 / 2570	-107.1	-107.1	0.34 (3)	3.41	V-L -682 / 3284	0.53 (2)
N-O	-12202 / 2570	-107.1	-107.1	0.34 (3)	3.41	T-L -163 / 997	0.16 (3)
O-AI	-11989 / 2464	-107.1	-107.1	0.35 (3)	3.41	T-M -1231 / 394	0.61 (3)
AI-P	-8056 / 1593	-107.1	-107.1	0.23 (3)	4.13	S-M -233 / 312	0.08 (9)
						S-O -143 / 930	0.15 (2)
						Q-O -844 / 290	0.11 (3)
A-AF	-1258 / 5578	-27.5	-27.5	0.31 (2)	6.25	AF-AG -8 / 624	0.00 (1)
AF-AE	-1258 / 5578	-27.5	-27.5	0.32 (2)	6.25	AG-AE -1158 / 5828	0.69 (2)
AE-AD	-2384 / 11339	-27.5	-27.5	0.57 (2)	6.25	Q-AI -1120 / 5828	0.69 (3)
AD-AC	-2384 / 11339	-27.5	-27.5	0.57 (2)	6.25	AH-AI 0 / 623	0.00 (1)
AC-AB	-2383 / 11601	-27.5	-27.5	0.51 (2)	6.25		
AB-AA	-2106 / 10611	-27.5	-27.5	0.47 (2)	6.25		
AA-Z	-2106 / 10611	-27.5	-27.5	0.47 (2)	6.25		
Z-Y	-2452 / 12182	-27.5	-27.5	0.55 (2)	6.25		
Y-X	-2543 / 12881	-27.5	-27.5	0.58 (1)	6.25		
X-W	-2543 / 12881	-27.5	-27.5	0.58 (1)	6.25		
W-V	-2543 / 12881	-27.5	-27.5	0.58 (1)	6.25		
V-U	-1938 / 10612	-27.5	-27.5	0.47 (3)	6.25		
U-T	-1938 / 10612	-27.5	-27.5	0.47 (3)	6.25		
T-S	-2217 / 11601	-27.5	-27.5	0.51 (3)	6.25		
S-R	-2225 / 11340	-27.5	-27.5	0.57 (3)	6.25		
R-Q	-2225 / 11340	-27.5	-27.5	0.57 (3)	6.25		
Q-AH	-1117 / 5578	-27.5	-27.5	0.32 (3)	6.25		
AH-P	-1117 / 5578	-27.5	-27.5	0.31 (3)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/968 (0.97")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/712 (1.32")

CSI: TC=0.35 (O-AI-3), BC=0.58 (W-Y-1),
 WB=1.00 (I-V-2), SSI=0.31 (P-AI-3)

DOL LUMBER=1.00 NAIL=1.00 LENS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

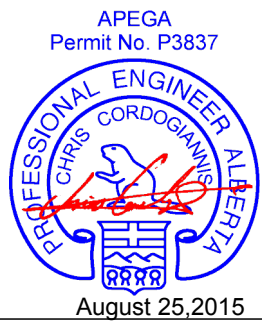
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	618	354	1667
MII16	473	276	2341

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
 JSI METAL= 0.94 (U) (INPUT = 1.00)



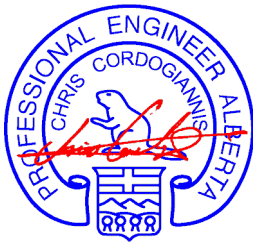
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JOB NAME G1507147	TRUSS NAME A6A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324090
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:19 2015 Page 2
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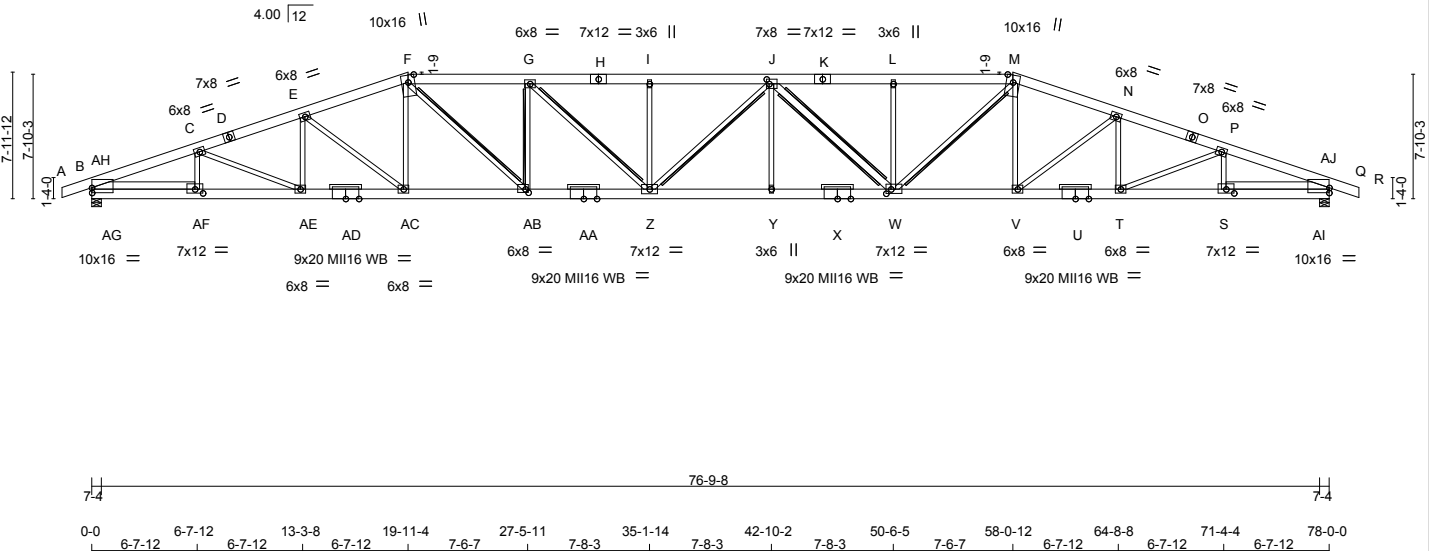
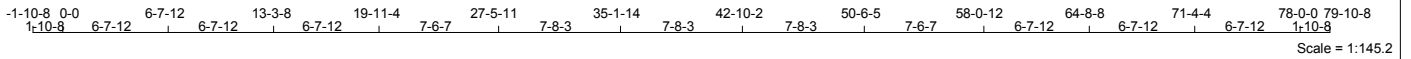
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APEGA
Permit No. P3837



August 25, 2015

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TOTAL WEIGHT = 574 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - K	2x8	DRY 1950F 1.7E	SPF
K - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
O - R	2x8	DRY 1950F 1.7E	SPF
B - AD	2x8	DRY 1950F 1.7E	SPF
AD - AA	2x8	DRY 1950F 1.7E	SPF
AA - X	2x8	DRY 1950F 1.7E	SPF
X - U	2x8	DRY 1950F 1.7E	SPF
U - Q	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

J - W 2x6 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	5461	0	5786	145
B	5461	0	5786	0
Q	5461	0	5786	0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1202 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT Q FOR 1186 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 145 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
B	4320	2801 / 0	780 / 0	0 / 0	0 / -1473	956 / 0	0 / 0	0 / 0
Q	4320	2801 / 0	780 / 0	0 / 0	0 / -1462	956 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS

B	0 / 0	0 / 0	0 / 0	103 / -103	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
DL = 5.0 PSF

BOT CH. LL = 10.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED

- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
CALCULATED VERT. DEFL.(LL)= L/866 (1.08")
ALLOWABLE DEFL.(TL)= L/240 (3.90")
CALCULATED VERT. DEFL.(TL)= L/637 (1.47")

CSI: TC=0.44 (G-I:2), BC=0.64 (W-Y:3),
WB=0.89 (E-AC:2), SSI=0.28 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, G, N, P						
C	TMWW-t	MT20	6.0	8.0		
D	TS-t	MT20	7.0	8.0		
F	TTWW+m	MT20	10.0	16.0	Edge	
H	TS-t	MT20	7.0	12.0		
I	TMW+w	MT20	3.0	6.0		
J	TMWWW-t	MT20	7.0	8.0	3.50	2.00
K	TS-t	MT20	7.0	12.0		
L	TMW+w	MT20	3.0	6.0		
M	TTWW+m	MT20	10.0	16.0	Edge	
O	TS-t	MT20	7.0	8.0		
Q	TMBMW1-I	MT20	10.0	16.0	3.75	
S	BMWW-t	MT20	7.0	12.0	3.00	6.00
T, V, AC, AE						
T	BMWW-t	MT20	6.0	8.0		
U, X, AA, AD						
U	BS-t	MT20	9.0	20.0		
W	BMWWW-t	MT20	7.0	12.0	3.25	3.75
Y	BMW+w	MT20	3.0	6.0		
Z	BMWWW-t	MT20	7.0	12.0		
AB	BMWW-t	MT20	6.0	8.0	2.50	2.25
AF	BMWW-t	MT20	7.0	12.0	3.00	6.00

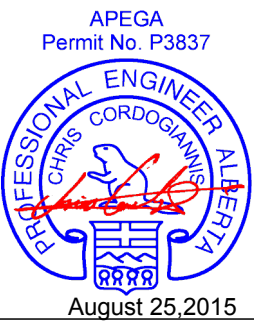
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	FACTORED VERT. (PLF)	MAX. VERT. (PLF)	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO						
A-B	-3 / 0	-107.1	-107.1	0.05 (2)	10.00	AF-C -975 / 310
B-AH	-8025 / 1606	-107.1	-107.1	0.21 (2)	4.16	C-AE -189 / 1125
AH-C	-11933 / 2488	-107.1	-107.1	0.32 (2)	3.44	AE-E -312 / 210
C-D	-12448 / 2656	-107.1	-107.1	0.33 (2)	3.38	E-AC -921 / 366
D-E	-12448 / 2656	-107.1	-107.1	0.33 (2)	3.38	AC-F -98 / 821
E-F	-11704 / 2554	-107.1	-107.1	0.29 (2)	3.51	F-AB -811 / 3896
F-G	-13329 / 2954	-107.1	-107.1	0.42 (2)	3.23	AB-G -2513 / 665
G-H	-14221 / 3144	-107.1	-107.1	0.44 (2)	3.14	G-Z -408 / 2039
H-I	-14221 / 3144	-107.1	-107.1	0.44 (2)	3.14	Z-I -903 / 316
I-J	-14221 / 3144	-107.1	-107.1	0.43 (2)	3.14	Z-J -453 / 387
J-K	-13313 / 2888	-107.1	-107.1	0.39 (3)	3.27	Y-J 0 / 373
K-L	-13313 / 2888	-107.1	-107.1	0.39 (3)	3.27	J-W -2101 / 431
L-M	-13314 / 2890	-107.1	-107.1	0.42 (3)	3.23	W-L -1094 / 376
M-N	-11706 / 2510	-107.1	-107.1	0.29 (3)	3.51	W-M -801 / 3866
N-O	-12448 / 2614	-107.1	-107.1	0.33 (3)	3.38	V-M -130 / 831
O-P	-12448 / 2614	-107.1	-107.1	0.33 (3)	3.38	V-N -918 / 387
P-AJ	-11933 / 2452	-107.1	-107.1	0.32 (3)	3.44	T-N -329 / 220
AJ-Q	-8025 / 1597	-107.1	-107.1	0.21 (3)	4.16	T-P -189 / 1123
Q-R	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	S-P -974 / 305
B-AG	-1232 / 5510	-27.5	-27.5	0.30 (2)	6.25	AG-AH -18 / 581
AG-AF	-1232 / 5510	-27.5	-27.5	0.32 (2)	6.25	AH-AF -1163 / 5854
AF-AE	-2365 / 11282	-27.5	-27.5	0.57 (2)	6.25	S-AJ -1132 / 5855
AE-AD	-2425 / 11836	-27.5	-27.5	0.53 (2)	6.25	AI-AJ -12 / 581
AD-AC	-2425 / 11836	-27.5	-27.5	0.53 (2)	6.25	
AC-AB	-2203 / 11097	-27.5	-27.5	0.49 (2)	6.25	
AB-AA	-2685 / 13328	-27.5	-27.5	0.61 (2)	6.25	
AA-Z	-2685 / 13328	-27.5	-27.5	0.61 (2)	6.25	
Z-Y	-2841 / 14244	-27.5	-27.5	0.64 (3)	6.25	
Y-X	-2841 / 14244	-27.5	-27.5	0.64 (3)	6.25	
X-W	-2841 / 14244	-27.5	-27.5	0.64 (3)	6.25	
W-V	-2056 / 11098	-27.5	-27.5	0.50 (3)	6.25	
V-U	-2279 / 11835	-27.5	-27.5	0.53 (3)	6.25	
U-T	-2279 / 11835	-27.5	-27.5	0.53 (3)	6.25	
T-S	-2221 / 11282	-27.5	-27.5	0.57 (3)	6.25	
S-AI	-1104 / 5510	-27.5	-27.5	0.32 (3)	6.25	
AI-Q	-1104 / 5510	-27.5	-27.5	0.30 (3)	6.25	



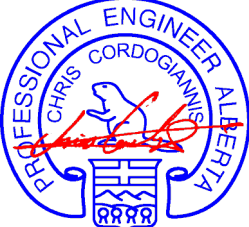
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JOB NAME G1507147	TRUSS NAME A7	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324091
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:21 2015 Page 2
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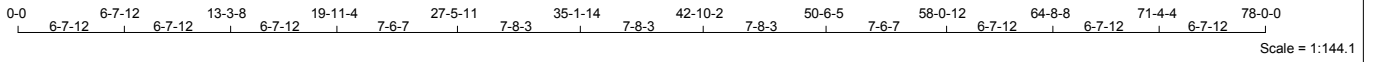
<p>WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.</p>	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM);INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>
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APEGA
Permit No. P3837

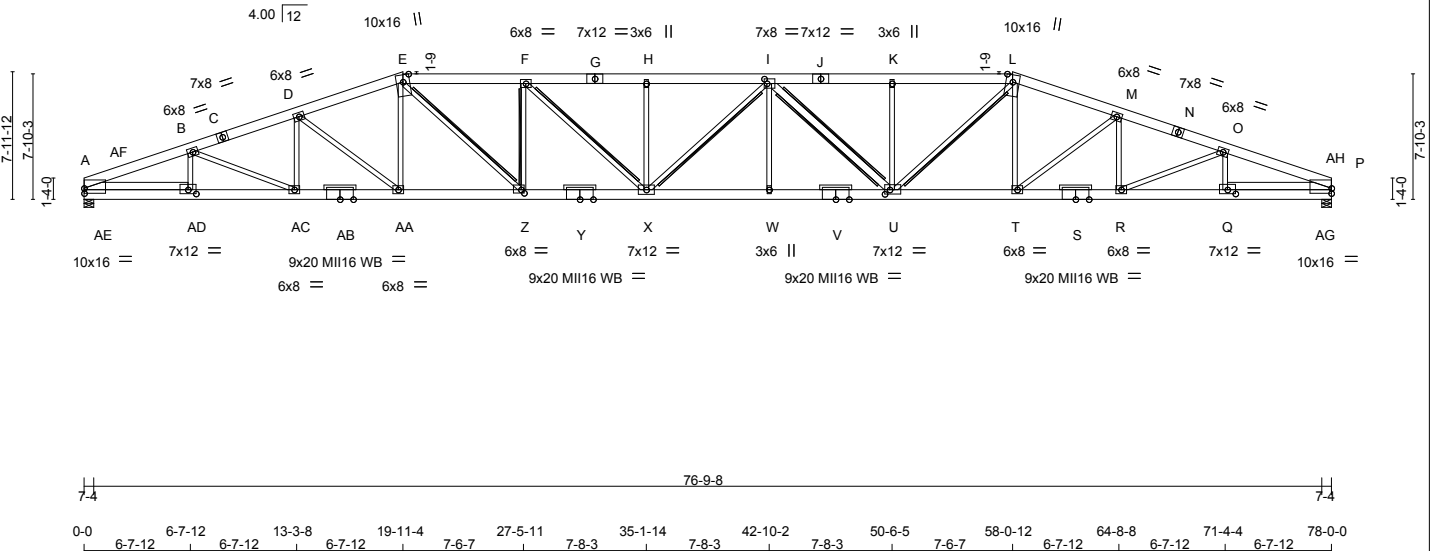


August 25, 2015

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	 100 Industrial Road Bradford, ON L3Z 3G7
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Scale = 1:144.1



TOTAL WEIGHT = 565 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - G	2x8	DRY 1950F 1.7E	SPF
G - J	2x8	DRY 1950F 1.7E	SPF
J - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
A - AB	2x8	DRY 1950F 1.7E	SPF
AB - Y	2x8	DRY 1950F 1.7E	SPF
Y - V	2x8	DRY 1950F 1.7E	SPF
V - S	2x8	DRY 1950F 1.7E	SPF
S - P	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS EXCEPT I - U

SIZE	DESCR.
2x4	DRY No.2
2x6	DRY No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	5249 0	5546 146	-1141 7-4	6-0
P	5249 0	5546 0	-1125 7-4	6-0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1141 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT P FOR 1125 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 146 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	4176	2657 / 0	780 / 0	0 / 0	0 / -1417	936 / 0	0 / 0	0 / 0
P	4176	2657 / 0	780 / 0	0 / 0	0 / -1406	936 / 0	0 / 0	0 / 0

HORIZONTAL REACTIONS

JT	REACT	REACT	REACT	REACT	REACT
A	0 / 0	0 / 0	0 / 0	104 / -104	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, P

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/866 (1.08")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/637 (1.47")

CSI: TC=0.44 (F-H:2), BC=0.64 (U-W:3),
 WB=0.89 (D-AA:2), SSI=0.28 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656
MII16	473	276	2341 1245 4454 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Z) (INPUT = 0.90)
 JSI METAL= 0.93 (P) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, F, M, O						
B	TMWW-t	MT20	6.0	8.0		
C	TS-t	MT20	7.0	8.0		
E	TTWW+m	MT20	10.0	16.0	Edge	
G	TS-t	MT20	7.0	12.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	7.0	8.0	3.50	2.00
J	TS-t	MT20	7.0	12.0		
K	TMW+w	MT20	3.0	6.0		
L	TTWW+m	MT20	10.0	16.0	Edge	
N	TS-t	MT20	7.0	8.0		
P	TMBMW1-I	MT20	10.0	16.0	3.75	
Q	BMWW-t	MT20	7.0	12.0	3.00	6.00
R, T, AA, AC						
R	BMWW-t	MT20	6.0	8.0		
S, V, Y, AB						
S	BS-t	MII16	9.0	20.0		
U	BMWW-t	MT20	7.0	12.0	3.25	3.75
W	BMW+w	MT20	3.0	6.0		
X	BMWW-t	MT20	7.0	12.0		
Z	BMWW-t	MT20	6.0	8.0	2.50	2.25
AD	BMWW-t	MT20	7.0	12.0	3.00	6.00

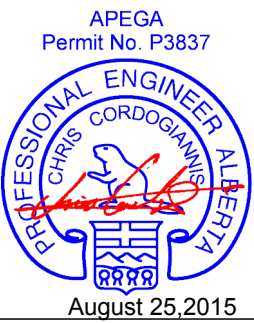
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO				
A-AF	-8025 / 1606	-107.1 -107.1 0.21 (2)	4.16	AD-B -975 / 310 0.12 (2)
AF-B	-11933 / 2488	-107.1 -107.1 0.32 (2)	3.44	B-AC -189 / 1125 0.18 (3)
B-C	-12448 / 2656	-107.1 -107.1 0.33 (2)	3.38	AC-D -312 / 210 0.08 (10)
C-D	-12448 / 2656	-107.1 -107.1 0.33 (2)	3.38	D-AA -921 / 366 0.89 (2)
D-E	-11704 / 2554	-107.1 -107.1 0.29 (2)	3.51	AA-E -98 / 821 0.13 (2)
E-F	-13329 / 2954	-107.1 -107.1 0.42 (2)	3.23	E-Z -811 / 3896 0.63 (3)
F-G	-14221 / 3144	-107.1 -107.1 0.44 (2)	3.14	Z-F -2513 / 665 0.72 (3)
G-H	-14221 / 3144	-107.1 -107.1 0.44 (2)	3.14	F-X -408 / 2039 0.33 (3)
H-I	-14221 / 3144	-107.1 -107.1 0.43 (2)	3.14	X-H -903 / 316 0.59 (3)
I-J	-13313 / 2888	-107.1 -107.1 0.39 (3)	3.27	X-I -453 / 387 0.32 (10)
J-K	-13313 / 2888	-107.1 -107.1 0.39 (3)	3.27	W-I 0 / 373 0.06 (17)
K-L	-13314 / 2890	-107.1 -107.1 0.42 (3)	3.23	I-U -2101 / 431 0.88 (2)
L-M	-11706 / 2510	-107.1 -107.1 0.29 (3)	3.51	U-K -1094 / 376 0.71 (3)
M-N	-12448 / 2614	-107.1 -107.1 0.33 (3)	3.38	U-L -801 / 3866 0.62 (2)
N-O	-12448 / 2614	-107.1 -107.1 0.33 (3)	3.38	T-L -130 / 831 0.13 (3)
O-AH	-11933 / 2452	-107.1 -107.1 0.32 (3)	3.44	T-M -918 / 387 0.89 (3)
AH-P	-8025 / 1597	-107.1 -107.1 0.21 (3)	4.16	R-M -329 / 220 0.09 (9)
				R-O -189 / 1123 0.18 (2)
				Q-O -974 / 305 0.12 (3)
A-AE	-1231 / 5510	-27.5 -27.5 0.30 (2)	6.25	AE-AF -18 / 581 0.00 (1)
AE-AD	-1231 / 5510	-27.5 -27.5 0.32 (2)	6.25	AF-AD -1163 / 5854 0.69 (2)
AD-AC	-2364 / 11282	-27.5 -27.5 0.57 (2)	6.25	Q-AH -1132 / 5855 0.69 (3)
AC-AB	-2423 / 11836	-27.5 -27.5 0.53 (2)	6.25	AG-AH -12 / 581 0.00 (1)
AB-AA	-2423 / 11836	-27.5 -27.5 0.53 (2)	6.25	
AA-Z	-2202 / 11097	-27.5 -27.5 0.49 (2)	6.25	
Z-Y	-2683 / 13328	-27.5 -27.5 0.61 (2)	6.25	
Y-X	-2683 / 13328	-27.5 -27.5 0.61 (2)	6.25	
X-W	-2840 / 14244	-27.5 -27.5 0.64 (3)	6.25	
W-V	-2840 / 14244	-27.5 -27.5 0.64 (3)	6.25	
V-U	-2840 / 14244	-27.5 -27.5 0.64 (3)	6.25	
U-T	-2054 / 11098	-27.5 -27.5 0.50 (3)	6.25	
T-S	-2277 / 11835	-27.5 -27.5 0.53 (3)	6.25	
S-R	-2277 / 11835	-27.5 -27.5 0.53 (3)	6.25	
R-Q	-2218 / 11282	-27.5 -27.5 0.57 (3)	6.25	
Q-AG	-1102 / 5510	-27.5 -27.5 0.32 (3)	6.25	
AG-P	-1102 / 5510	-27.5 -27.5 0.30 (3)	6.25	



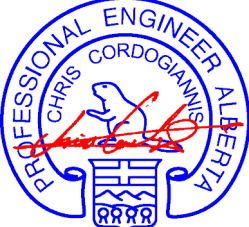
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JOB NAME G1507147	TRUSS NAME A7A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324092
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:24 2015 Page 2
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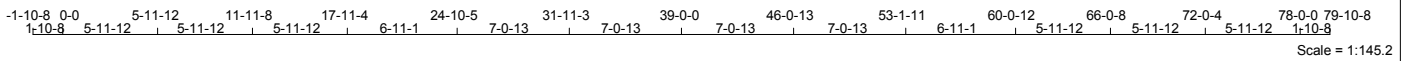
<p>WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.</p>	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM);INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>
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APEGA
Permit No. P3837



August 25, 2015

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Scale = 1:145.2

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - H	2x8	DRY 1950F 1.7E	SPF
H - J	2x8	DRY 1950F 1.7E	SPF
J - M	2x8	DRY 1950F 1.7E	SPF
M - O	2x8	DRY 1950F 1.7E	SPF
O - Q	2x8	DRY 1950F 1.7E	SPF
Q - T	2x8	DRY 1950F 1.7E	SPF
B - AG	2x8	DRY 1950F 1.7E	SPF
AG - AD	2x8	DRY 1950F 1.7E	SPF
AD - AB	2x8	DRY 1950F 1.7E	SPF
AB - Z	2x8	DRY 1950F 1.7E	SPF
Z - W	2x8	DRY 1950F 1.7E	SPF
W - S	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

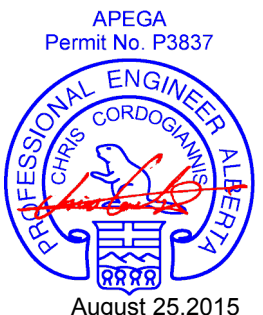
PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75
C, E, G, I, L, N, P, R					
D	TMWW-t	MT20	6.0	8.0	
D	TS-t	MT20	7.0	8.0	
F	TTWW+m	MT20	10.0	16.0	Edge
H, J, M					
H	TS-t	MT20	7.0	12.0	
K	TMW+w	MT20	3.0	6.0	
O	TTWW+m	MT20	10.0	16.0	Edge
Q	TS-t	MT20	7.0	8.0	
S	TMBMW1-I	MT20	10.0	16.0	3.75
U	BMWW-t	MT20	7.0	12.0	3.00 6.00
V, X, AA, AC, AF, AH					
V	BMWW-t	MT20	6.0	8.0	
W, Z, AD, AG					
W	BS-t	MII16	9.0	20.0	
Y	BMWW-t	MT20	7.0	8.0	3.25 2.00
AB	BSWWW-I	MII16	9.0	20.0	6.25 10.00
AE	BMWW-t	MT20	7.0	8.0	3.25 2.00
AI	BMWW-t	MT20	7.0	12.0	3.00 6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UP/LIFT	IN-SX
B	5461	0	5839	131
S	5461	0	5839	0

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1210 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT S FOR 1197 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 131 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX SNOW	MIN LIVE	COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
B	4320	2836 / 0	780 / 0	0 / 0	0 / 0	0 / -1479	956 / 0	0 / 0
S	4320	2836 / 0	780 / 0	0 / 0	0 / 0	0 / -1470	956 / 0	0 / 0

HORIZONTAL REACTIONS

JT	VERT	HORZ	UP/LIFT	IN-SX	IN-SX
B	---	0 / 0	0 / 0	0 / 0	94 / -94
S	---	0 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, S

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 2.94FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT F-AE, G-AE, G-AC, I-AB, L-AB, N-AA, N-Y, O-Y

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LIVE (LC1)	MAX UNBRAC (LC)	MEMB. FORCE (LBS)	MAX FACTORED (LC)
FR-TO		FROM	TO	LENGTH	FR-TO	
A-B	-3 / 0	-107.1	-107.1	0.05 (2)	10.00	AI-C -1120 / 327
B-AK	-7971 / 1601	-107.1	-107.1	0.19 (2)	4.19	C-AH -238 / 1339
AK-C	-11807 / 2454	-107.1	-107.1	0.29 (2)	3.47	AH-E -417 / 173
C-D	-12635 / 2678	-107.1	-107.1	0.32 (2)	3.36	E-AF -595 / 503
D-E	-12635 / 2678	-107.1	-107.1	0.32 (2)	3.36	AF-F -139 / 625
E-F	-12154 / 2625	-107.1	-107.1	0.27 (2)	3.45	F-AE -896 / 4329
F-G	-14199 / 3102	-107.1	-107.1	0.43 (2)	3.14	AE-G -2827 / 718
G-H	-15643 / 3404	-107.1	-107.1	0.47 (2)	2.99	G-AC -534 / 2632
H-I	-15643 / 3404	-107.1	-107.1	0.47 (2)	2.99	AC-I -1596 / 450
I-J	-15962 / 3480	-107.1	-107.1	0.49 (3)	2.94	I-AB -222 / 1098
J-K	-15962 / 3480	-107.1	-107.1	0.49 (3)	2.94	AB-K -859 / 303
K-L	-15962 / 3480	-107.1	-107.1	0.49 (2)	2.94	AB-L -223 / 1098
L-M	-15643 / 3344	-107.1	-107.1	0.47 (3)	2.99	AA-L -1596 / 450
M-N	-15643 / 3344	-107.1	-107.1	0.47 (3)	2.99	AA-N -533 / 2632
N-O	-14199 / 3055	-107.1	-107.1	0.43 (3)	3.14	Y-N -2827 / 718
O-P	-12154 / 2590	-107.1	-107.1	0.27 (3)	3.45	Y-O -896 / 4329
P-Q	-12635 / 2645	-107.1	-107.1	0.32 (3)	3.36	X-O -150 / 625
Q-R	-12635 / 2645	-107.1	-107.1	0.32 (3)	3.36	X-P -595 / 523
R-AM	-11807 / 2426	-107.1	-107.1	0.29 (3)	3.47	V-P -432 / 173
AM-S	-7971 / 1595	-107.1	-107.1	0.19 (3)	4.19	V-R -238 / 1339
S-T	-3 / 0	-107.1	-107.1	0.05 (3)	10.00	U-R -1120 / 323
						AJ-AK -25 / 526
B-AJ	-1203 / 5428	-27.5	-27.5	0.28 (2)	6.25	AK-AI -1160 / 5833
AJ-AI	-1203 / 5428	-27.5	-27.5	0.32 (2)	6.25	U-AM -1134 / 5833
AI-AH	-2331 / 11157	-27.5	-27.5	0.56 (2)	6.25	AL-AM -19 / 526
AH-AG	-2457 / 12012	-27.5	-27.5	0.53 (2)	6.25	
AG-AF	-2457 / 12012	-27.5	-27.5	0.53 (2)	6.25	
AF-AE	-2294 / 11536	-27.5	-27.5	0.51 (2)	6.25	
AE-AD	-2857 / 14198	-27.5	-27.5	0.64 (2)	6.25	
AD-AC	-2857 / 14198	-27.5	-27.5	0.64 (2)	6.25	
AC-AB	-3161 / 15643	-27.5	-27.5	0.71 (2)	6.21	
AB-AA	-3088 / 15643	-27.5	-27.5	0.71 (3)	6.25	
AA-Z	-2714 / 14198	-27.5	-27.5	0.64 (3)	6.25	
Z-Y	-2714 / 14198	-27.5	-27.5	0.64 (3)	6.25	
Y-X	-2163 / 11536	-27.5	-27.5	0.51 (3)	6.25	
X-W	-2326 / 12012	-27.5	-27.5	0.53 (3)	6.25	
W-V	-2326 / 12012	-27.5	-27.5	0.53 (3)	6.25	
V-U	-2200 / 11157	-27.5	-27.5	0.56 (3)	6.25	
U-AL	-1086 / 5428	-27.5	-27.5	0.32 (3)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/740 (1.27")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/546 (1.71")

CSI: TC=0.49 (I-K:3), BC=0.71 (AB-AC:2), WB=0.80 (I-AC:3), SSI=0.26 (N-O:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI
MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822
MII16	473	276	2341	1245
			4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (AE) (INPUT = 0.90)
 JSI METAL= 0.96 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A8	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324093
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:27 2015 Page 2

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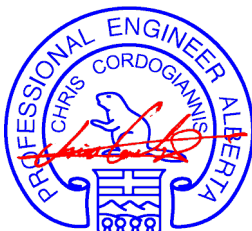
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
AL-S	-1086 / 5428	-27.5 -27.5	0.28 (3)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

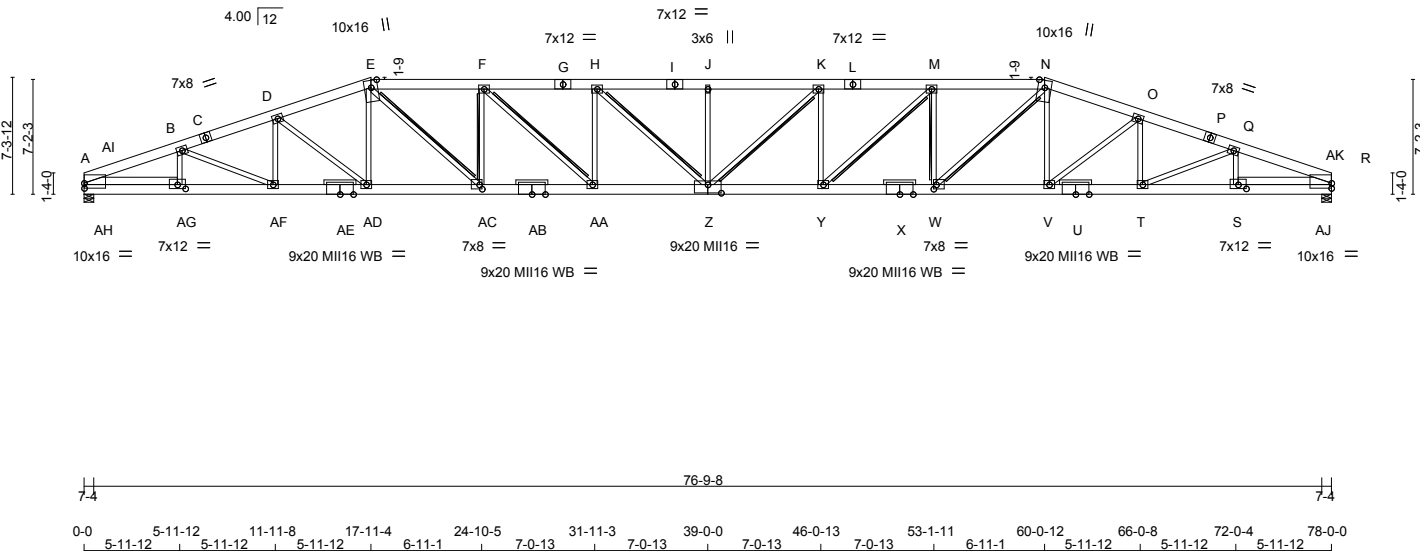
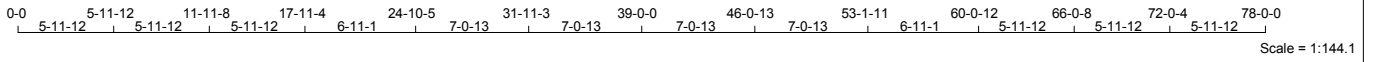
APEGA
Permit No. P3837



August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 558 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - G	2x8	DRY 1950F 1.7E	SPF
G - I	2x8	DRY 1950F 1.7E	SPF
I - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
P - R	2x8	DRY 1950F 1.7E	SPF
A - AE	2x8	DRY 1950F 1.7E	SPF
AE - AB	2x8	DRY 1950F 1.7E	SPF
AB - Z	2x8	DRY 1950F 1.7E	SPF
Z - X	2x8	DRY 1950F 1.7E	SPF
X - U	2x8	DRY 1950F 1.7E	SPF
U - R	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	HW2	SIZE	DESCR.
HW1	2x6	DRY No.2	SPF
HW2	2x6	DRY No.2	SPF

ALL WEBS 2x4 DRY SEASONED LUMBER.

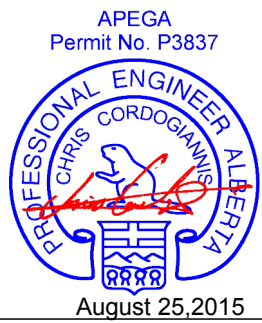
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75	
B, D, F, H, K, M, O, Q						
C	TS-t	MT20	6.0	8.0		
E	TTWW+m	MT20	10.0	16.0	Edge	
G, I, L						
J	TS-t	MT20	7.0	12.0		
N	TTWW+m	MT20	10.0	16.0	Edge	
P	TS-t	MT20	7.0	8.0		
R	TMBMW1-I	MT20	10.0	16.0	3.75	
S	BMWW-t	MT20	7.0	12.0	3.00	6.00
T, V, Y, AA, AD, AF						
U, X, AB, AE						
W	BMWW-t	MT20	9.0	8.0	3.25	2.00
Z	BSWWW-I	MIH16	9.0	20.0	6.25	10.00
AC	BMWW-t	MT20	7.0	8.0	3.25	2.00
AG	BMWW-t	MT20	7.0	12.0	3.00	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT				
A	5249	0	5599	133
R	5249	0	5599	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1149 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT R FOR 1136 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 133 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
A	4176	2692 / 0	780 / 0	0 / 0	0 / -1422	936 / 0
R	4176	2692 / 0	780 / 0	0 / 0	0 / -1413	936 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	95 / -95	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, R

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.94FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.21FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT E-AC, F-AC, F-AA, H-Z, K-Z, M-Y, M-W, N-W

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LIVE (CSI (LC))	MAX UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX FACTORED (CSI (LC))
FR-TO						
A-AI	-7971 / 1601	-107.1	-107.1	0.19 (2)	4.19	AG-B -1120 / 327
AI-B	-11807 / 2454	-107.1	-107.1	0.29 (2)	3.47	B-AF -238 / 1339
B-C	-12635 / 2678	-107.1	-107.1	0.32 (2)	3.36	AF-D -417 / 173
C-D	-12635 / 2678	-107.1	-107.1	0.32 (2)	3.36	D-AD -595 / 503
D-E	-12154 / 2625	-107.1	-107.1	0.27 (2)	3.45	AD-E -139 / 625
E-F	-14199 / 3102	-107.1	-107.1	0.43 (2)	3.14	E-AC -896 / 4329
F-G	-15643 / 3404	-107.1	-107.1	0.47 (2)	2.99	AC-F -2827 / 718
G-H	-15643 / 3404	-107.1	-107.1	0.47 (2)	2.99	F-AA -534 / 2632
H-I	-15962 / 3480	-107.1	-107.1	0.49 (3)	2.94	AA-H -1596 / 450
I-J	-15962 / 3480	-107.1	-107.1	0.49 (3)	2.94	H-Z -222 / 1098
J-K	-15962 / 3480	-107.1	-107.1	0.49 (2)	2.94	Z-J -859 / 303
K-L	-15643 / 3344	-107.1	-107.1	0.43 (2)	3.14	E-AC -896 / 4329
L-M	-15643 / 3344	-107.1	-107.1	0.47 (3)	2.99	Y-K -1596 / 450
M-N	-14199 / 3055	-107.1	-107.1	0.43 (3)	3.14	Y-M -533 / 2632
N-O	-12154 / 2590	-107.1	-107.1	0.27 (3)	3.45	W-M -2827 / 718
O-P	-12635 / 2645	-107.1	-107.1	0.32 (3)	3.36	W-N -896 / 4329
P-Q	-12635 / 2645	-107.1	-107.1	0.32 (3)	3.36	V-N -150 / 625
Q-AK	-11807 / 2426	-107.1	-107.1	0.29 (3)	3.47	V-O -595 / 523
AK-R	-7971 / 1595	-107.1	-107.1	0.19 (3)	4.19	T-O -432 / 173
						T-Q -238 / 1339
A-AH	-1202 / 5428	-27.5	-27.5	0.28 (2)	6.25	S-Q -1120 / 323
AH-AG	-1202 / 5428	-27.5	-27.5	0.32 (2)	6.25	AH-AI -25 / 526
AG-AF	-2330 / 11157	-27.5	-27.5	0.56 (2)	6.25	AI-AG -1160 / 5833
AF-AE	-2456 / 12012	-27.5	-27.5	0.53 (2)	6.25	S-AK -1134 / 5833
AE-AD	-2456 / 12012	-27.5	-27.5	0.53 (2)	6.25	AJ-AK -19 / 526
AD-AC	-2293 / 11536	-27.5	-27.5	0.51 (2)	6.25	
AC-AB	-2856 / 14198	-27.5	-27.5	0.64 (2)	6.25	
AB-AA	-2856 / 14198	-27.5	-27.5	0.64 (2)	6.25	
AA-Z	-3160 / 15643	-27.5	-27.5	0.71 (2)	6.21	
Z-Y	-3086 / 15643	-27.5	-27.5	0.71 (3)	6.25	
Y-X	-2712 / 14198	-27.5	-27.5	0.64 (3)	6.25	
X-W	-2712 / 14198	-27.5	-27.5	0.64 (3)	6.25	
W-V	-2161 / 11536	-27.5	-27.5	0.51 (3)	6.25	
V-U	-2324 / 12012	-27.5	-27.5	0.53 (3)	6.25	
U-T	-2324 / 12012	-27.5	-27.5	0.53 (3)	6.25	
T-S	-2198 / 11157	-27.5	-27.5	0.56 (3)	6.25	
S-AJ	-1084 / 5428	-27.5	-27.5	0.32 (3)	6.25	
AJ-R	-1084 / 5428	-27.5	-27.5	0.28 (3)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF

DL = 5.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED

- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")

CALCULATED VERT. DEFL.(LL) = L/740 (1.27")

ALLOWABLE DEFL.(TL)= L/240 (3.90")

CALCULATED VERT. DEFL.(TL) = L/546 (1.71")

CSI: TC=0.49 (H-J:3), BC=0.71 (Z-AA:2), WB=0.80 (H-AA:3), SSI=0.26 (M-N:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
MIH16	473	276	2341

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (AC) (INPUT = 0.90)

JSI METAL= 0.92 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A8A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324094
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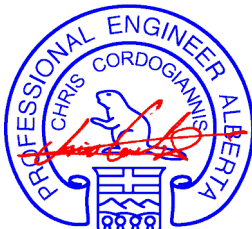
Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:29 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYjy3SS-nHlw1DP8bPOeSgXqZ_OpvFJv1noFQLcZpp6ALAykfk8

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7} PSF
AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK
COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM},INTERNAL
WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON
{OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX
AWAY FROM EAVE.

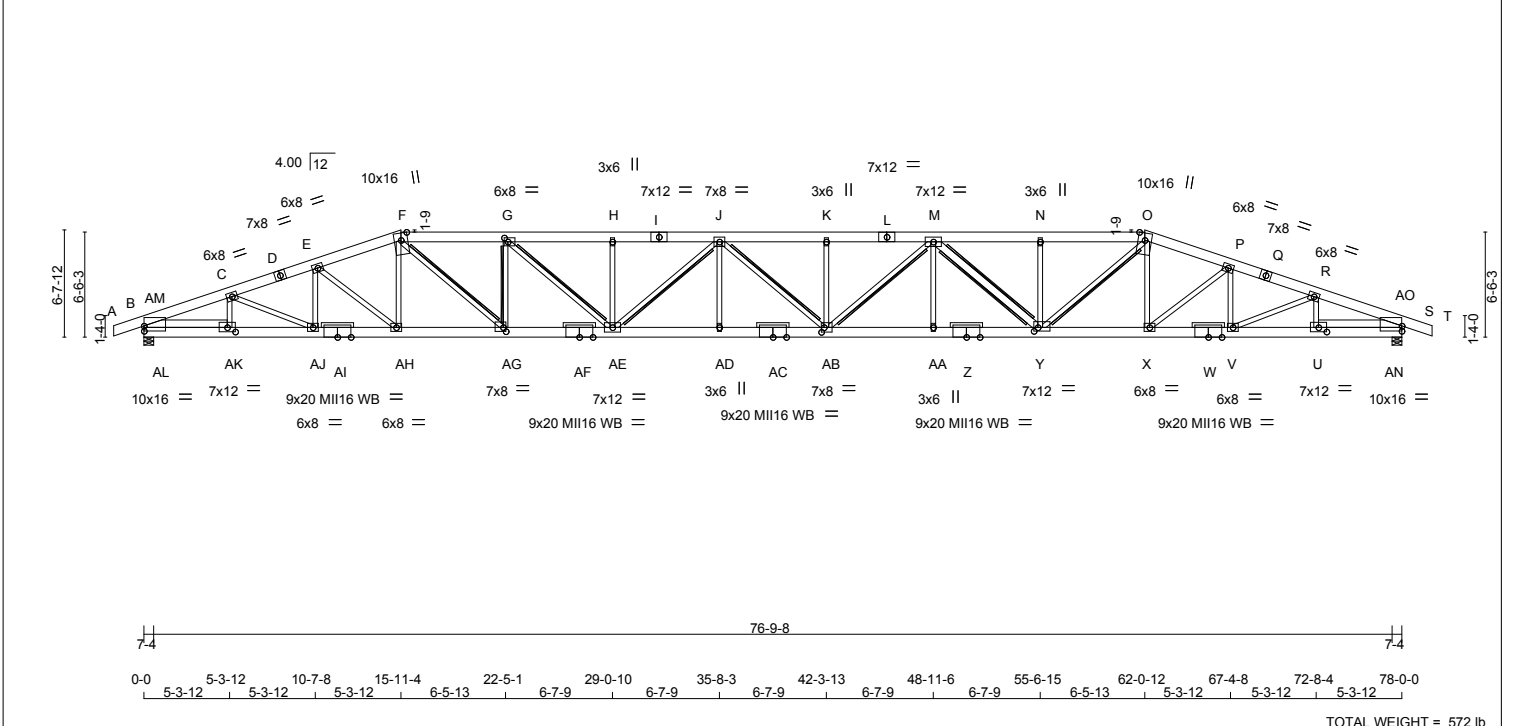
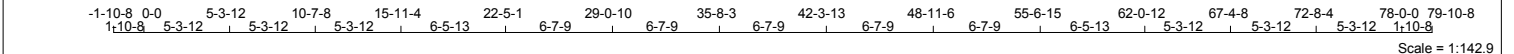
APEGA
Permit No. P3837



August 25, 2015

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 572 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA																																																																																																																																																																																																																																																																																																																																																															
<p>N. L. G. A. RULES</p> <table border="1"> <tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr> <tr><td>A - D</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>D - F</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>F - I</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>I - L</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>L - O</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>O - Q</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>Q - T</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>B - AI</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>AI - AF</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>AF - AC</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>AC - Z</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>Z - W</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> <tr><td>W - S</td><td>2x8 DRY</td><td>1950F 1.7E</td><td>SPF</td></tr> </table>				CHORDS	SIZE	LUMBER	DESCR.	A - D	2x8 DRY	1950F 1.7E	SPF	D - F	2x8 DRY	1950F 1.7E	SPF	F - I	2x8 DRY	1950F 1.7E	SPF	I - L	2x8 DRY	1950F 1.7E	SPF	L - O	2x8 DRY	1950F 1.7E	SPF	O - Q	2x8 DRY	1950F 1.7E	SPF	Q - T	2x8 DRY	1950F 1.7E	SPF	B - AI	2x8 DRY	1950F 1.7E	SPF	AI - AF	2x8 DRY	1950F 1.7E	SPF	AF - AC	2x8 DRY	1950F 1.7E	SPF	AC - Z	2x8 DRY	1950F 1.7E	SPF	Z - W	2x8 DRY	1950F 1.7E	SPF	W - S	2x8 DRY	1950F 1.7E	SPF	<p>TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.</p> <p>BEARINGS</p> <table border="1"> <tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr> <tr><td>JT</td><td>4320</td><td>2868 / 0</td><td>780 / 0</td><td>0 / 0</td></tr> <tr><td>B</td><td>5461</td><td>0</td><td>5886 / 118</td><td>-1217 / 7-4</td></tr> <tr><td>S</td><td>5461</td><td>0</td><td>5886 / 0</td><td>-1207 / 7-4</td></tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1217 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT S FOR 1207 LBS FACTORED UPLIFT</p> <p>NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER</p> <p>PROVIDE FOR 118 LBS FACTORED HORIZONTAL REACTION AT JOINT B</p>					FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	4320	2868 / 0	780 / 0	0 / 0	B	5461	0	5886 / 118	-1217 / 7-4	S	5461	0	5886 / 0	-1207 / 7-4	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 31.5 PSF DL = 5.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 53.5 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OR PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF CBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED - SLOPE REDUCTION FACTOR USED</p> <p>(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p> <p>ALLOWABLE DEFL.(LL)= L/360 (2.60") CALCULATED VERT. DEFL.(LL) = L/622 (1.50") ALLOWABLE DEFL.(TL)= L/240 (3.90") CALCULATED VERT. DEFL.(TL) = L/462 (2.03")</p> <p>CSI: TC=0.60 (J-K:3), BC=0.81 (AD-AE:2), WB=0.99 (G-AG:3), SSI=0.23 (F-G:3)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LNS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10</p> <p>SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50</p> <p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</p> <p>NAIL VALUES</p> <table border="1"> <tr><th>PLATE</th><th>GRIP(DRY)</th><th>SHEAR</th><th>SECTION</th></tr> <tr><td></td><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td></tr> <tr><td></td><td>MAX</td><td>MIN</td><td>MAX</td></tr> <tr><td>MT20</td><td>618</td><td>354</td><td>1667</td></tr> <tr><td>MII16</td><td>473</td><td>276</td><td>2341</td></tr> <tr><td></td><td>822</td><td>1245</td><td>4454</td></tr> <tr><td></td><td>1656</td><td></td><td>1656</td></tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.90 (AG) (INPUT = 0.90) JSI METAL= 0.94 (S) (INPUT = 1.00)</p>				PLATE	GRIP(DRY)	SHEAR	SECTION		(PSI)	(PLI)	(PLI)		MAX	MIN	MAX	MT20	618	354	1667	MII16	473	276	2341		822	1245	4454		1656		1656																																																																																																																																																																																																																																																				
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<p>REINFORCING MEMBERS</p> <table border="1"> <tr><th>HW1</th><th>2x6 DRY</th><th>No.2</th><th>SPF</th></tr> <tr><th>HW2</th><th>2x6 DRY</th><th>No.2</th><th>SPF</th></tr> </table>				HW1	2x6 DRY	No.2	SPF	HW2	2x6 DRY	No.2	SPF	<p>UNFACTORED REACTIONS</p> <table border="1"> <tr><th>JT</th><th>1ST LCASE</th><th>MAX./MIN. COMPONENT REACTIONS</th><th>SOIL</th></tr> <tr><td>B</td><td>COMBINED</td><td>SNOW LIVE PERM.LIVE WIND DEAD</td><td></td></tr> <tr><td>B</td><td>4320</td><td>2868 / 0 780 / 0 0 / 0</td><td>0 / -1484 956 / 0 0 / 0</td></tr> <tr><td>S</td><td>4320</td><td>2868 / 0 780 / 0 0 / 0</td><td>0 / -1477 956 / 0 0 / 0</td></tr> </table> <p>HORIZONTAL REACTIONS B --- 0 / 0 0 / 0 0 / 0 84 / -84 0 / 0 0 / 0</p> <p>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, S</p>				JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SOIL	B	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD		B	4320	2868 / 0 780 / 0 0 / 0	0 / -1484 956 / 0 0 / 0	S	4320	2868 / 0 780 / 0 0 / 0	0 / -1477 956 / 0 0 / 0	<p>74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p>																																																																																																																																																																																																																																																																																																																																							
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<p>ALL WEBS EXCEPT M - Y</p> <table border="1"> <tr><th>M - Y</th><th>2x6 DRY</th><th>No.2</th><th>SPF</th></tr> </table> <p>DRY: SEASONED LUMBER.</p>				M - Y	2x6 DRY	No.2	SPF	<p>BRACING</p> <p>MAX. UNBRACED TOP CHORD LENGTH = 2.71FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.89FT. OR RIGID CEILING DIRECTLY APPLIED.</p> <p>2x3 DRY SPF 1650F 1.5E T-BRACE AT F-AG, G-AE, J-AB, M-AB, O-Y 2x3 DRY SPF 2100F 1.8E T-BRACE AT G-AG 2x4 DRY SPF No.2 T-BRACE AT J-AE 2x4 DRY SPF No.2 I-BRACE AT M-Y</p> <p>FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.</p>				<p>ALLOWABLE DEFL.(LL)= L/360 (2.60") CALCULATED VERT. DEFL.(LL) = L/622 (1.50") ALLOWABLE DEFL.(TL)= L/240 (3.90") CALCULATED VERT. DEFL.(TL) = L/462 (2.03")</p>																																																																																																																																																																																																																																																																																																																																																											
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<p>PLATES (table is in inches)</p> <table border="1"> <tr><th>JT TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr> <tr><td>B</td><td>TMBMW1-I</td><td>MT20</td><td>10.0</td><td>16.0</td><td>3.75</td></tr> <tr><td>C, E, P, R</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>D</td><td>TMWW-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td></td></tr> <tr><td>D</td><td>TS-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td></td></tr> <tr><td>F</td><td>TTWW+m</td><td>MT20</td><td>10.0</td><td>16.0</td><td>Edge</td></tr> <tr><td>G</td><td>TMWW-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td>3.00 2.75</td></tr> <tr><td>H, K, N</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>H</td><td>TMW+w</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td></tr> <tr><td>I</td><td>TS-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td></td></tr> <tr><td>J</td><td>TMWWW-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td></td></tr> <tr><td>L</td><td>TS-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td></td></tr> <tr><td>M</td><td>TMWWW-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td></td></tr> <tr><td>O</td><td>TTWW+m</td><td>MT20</td><td>10.0</td><td>16.0</td><td>Edge</td></tr> <tr><td>Q</td><td>TS-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td></td></tr> <tr><td>S</td><td>TMBMW1-I</td><td>MT20</td><td>10.0</td><td>16.0</td><td>3.75</td></tr> <tr><td>U</td><td>BMWW-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td>3.25 6.00</td></tr> <tr><td>V, X, AH, AJ</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>V</td><td>BMWW-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td></td></tr> <tr><td>W, Z, AC, AF, AI</td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>W</td><td>BS-t</td><td>MII16</td><td>9.0</td><td>20.0</td><td></td></tr> <tr><td>Y</td><td>BMWWW-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td>2.75 3.00</td></tr> <tr><td>AA</td><td>BMW+w</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td></tr> <tr><td>AB</td><td>BMWWW-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td>3.50 2.00</td></tr> <tr><td>AD</td><td>BMW+w</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td></tr> <tr><td>AE</td><td>BMWWW-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td></td></tr> <tr><td>AG</td><td>BMWW-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td>3.00 2.00</td></tr> <tr><td>AK</td><td>BMWW-t</td><td>MT20</td><td>7.0</td><td>12.0</td><td>3.25 6.00</td></tr> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD. WB - INDICATES BLOCKING REQUIRED</p>				JT TYPE	PLATES	W	LEN	Y	X	B	TMBMW1-I	MT20	10.0	16.0	3.75	C, E, P, R						D	TMWW-t	MT20	6.0	8.0		D	TS-t	MT20	7.0	8.0		F	TTWW+m	MT20	10.0	16.0	Edge	G	TMWW-t	MT20	6.0	8.0	3.00 2.75	H, K, N						H	TMW+w	MT20	3.0	6.0		I	TS-t	MT20	7.0	12.0		J	TMWWW-t	MT20	7.0	8.0		L	TS-t	MT20	7.0	12.0		M	TMWWW-t	MT20	7.0	12.0		O	TTWW+m	MT20	10.0	16.0	Edge	Q	TS-t	MT20	7.0	8.0		S	TMBMW1-I	MT20	10.0	16.0	3.75	U	BMWW-t	MT20	7.0	12.0	3.25 6.00	V, X, AH, AJ						V	BMWW-t	MT20	6.0	8.0		W, Z, AC, AF, AI						W	BS-t	MII16	9.0	20.0		Y	BMWWW-t	MT20	7.0	12.0	2.75 3.00	AA	BMW+w	MT20	3.0	6.0		AB	BMWWW-t	MT20	7.0	8.0	3.50 2.00	AD	BMW+w	MT20	3.0	6.0		AE	BMWWW-t	MT20	7.0	12.0		AG	BMWW-t	MT20	7.0	8.0	3.00 2.00	AK	BMWW-t	MT20	7.0	12.0	3.25 6.00	<p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <tr><th colspan="2">C H O R D S</th><th colspan="2">W E B S</th></tr> <tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. FACTORED CSI (LC)</th></tr> <tr><td>FR-TO</td><td></td><td>FROM TO</td><td>LENGTH FR-TO</td></tr> <tr><td>A-B</td><td>-3 / 0</td><td>-107.1 -107.1</td><td>0.05 (2) 10.00</td></tr> <tr><td>B-AM</td><td>-7889 / 1589</td><td>-107.1 -107.1</td><td>0.17 (2) 4.23</td></tr> <tr><td>AM-C</td><td>-11599 / 2404</td><td>-107.1 -107.1</td><td>0.25 (2) 3.53</td></tr> <tr><td>C-D</td><td>-12746 / 2687</td><td>-107.1 -107.1</td><td>0.30 (2) 3.34</td></tr> <tr><td>D-E</td><td>-12746 / 2687</td><td>-107.1 -107.1</td><td>0.30 (2) 3.34</td></tr> <tr><td>E-F</td><td>-12548 / 2685</td><td>-107.1 -107.1</td><td>0.26 (2) 3.41</td></tr> <tr><td>F-G</td><td>-15103 / 3260</td><td>-107.1 -107.1</td><td>0.46 (2) 3.03</td></tr> <tr><td>G-H</td><td>-17099 / 3668</td><td>-107.1 -107.1</td><td>0.54 (2) 2.81</td></tr> <tr><td>H-I</td><td>-17099 / 3668</td><td>-107.1 -107.1</td><td>0.54 (2) 2.81</td></tr> <tr><td>I-J</td><td>-17099 / 3668</td><td>-107.1 -107.1</td><td>0.54 (2) 2.81</td></tr> <tr><td>J-K</td><td>-18011 / 3833</td><td>-107.1 -107.1</td><td>0.60 (3) 2.71</td></tr> <tr><td>K-L</td><td>-18011 / 3833</td><td>-107.1 -107.1</td><td>0.59 (3) 2.72</td></tr> <tr><td>L-M</td><td>-18011 / 3833</td><td>-107.1 -107.1</td><td>0.59 (3) 2.72</td></tr> <tr><td>M-N</td><td>-15075 / 3211</td><td>-107.1 -107.1</td><td>0.44 (3) 3.06</td></tr> <tr><td>N-O</td><td>-15076 / 3213</td><td>-107.1 -107.1</td><td>0.47 (3) 3.03</td></tr> <tr><td>O-P</td><td>-12553 / 2659</td><td>-107.1 -107.1</td><td>0.26 (3) 3.41</td></tr> <tr><td>P-Q</td><td>-12745 / 2661</td><td>-107.1 -107.1</td><td>0.30 (3) 3.34</td></tr> <tr><td>Q-R</td><td>-12745 / 2661</td><td>-107.1 -107.1</td><td>0.30 (3) 3.34</td></tr> <tr><td>R-AO</td><td>-11599 / 2384</td><td>-107.1 -107.1</td><td>0.25 (3) 3.53</td></tr> <tr><td>AO-S</td><td>-7888 / 1585</td><td>-107.1 -107.1</td><td>0.17 (3) 4.23</td></tr> <tr><td>S-T</td><td>-3 / 0</td><td>-107.1 -107.1</td><td>0.05 (3) 10.00</td></tr> <tr><td>B-AL</td><td>-1170 / 5329</td><td>-27.5 -27.5</td><td>0.27 (2) 6.25</td></tr> <tr><td>AL-AK</td><td>-1170 / 5329</td><td>-27.5 -27.5</td><td>0.31 (2) 6.25</td></tr> <tr><td>AK-AJ</td><td>-2282 / 10954</td><td>-27.5 -27.5</td><td>0.55 (2) 6.25</td></tr> <tr><td>AJ-AI</td><td>-2476 / 12117</td><td>-27.5 -27.5</td><td>0.54 (2) 6.25</td></tr> <tr><td>AI-AH</td><td>-2476 / 12117</td><td>-27.5 -27.5</td><td>0.54 (2) 6.25</td></tr> <tr><td>AH-AG</td><td>-2378 / 11923</td><td>-27.5 -27.5</td><td>0.53 (2) 6.25</td></tr> <tr><td>AG-AF</td><td>-3040 / 15102</td><td>-27.5 -27.5</td><td>0.69 (2) 6.25</td></tr> <tr><td>AF-AE</td><td>-3040 / 15102</td><td>-27.5 -27.5</td><td>0.69 (2) 6.25</td></tr> <tr><td>AE-AD</td><td>-3655 / 18029</td><td>-27.5 -27.5</td><td>0.81 (2) 5.89</td></tr> <tr><td>AD-AC</td><td>-3655 / 18029</td><td>-27.5 -27.5</td><td>0.81 (2) 5.89</td></tr> <tr><td>AC-AB</td><td>-3655 / 18029</td><td>-27.5 -27.5</td><td>0.81 (2) 5.89</td></tr> <tr><td>AB-AA</td><td>-3372 / 17135</td><td>-27.5 -27.5</td><td>0.77 (3) 6.06</td></tr> <tr><td>AA-Z</td><td>-3372 / 17135</td><td>-27.5 -27.5</td><td>0.78 (3) 6.06</td></tr> <tr><td>Z-Y</td><td>-3372 / 17135</td><td>-27.5 -27.5</td><td>0.78 (3) 6.06</td></tr> </table>				C H O R D S		W E B S		MEMB.	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FACTORED CSI (LC)	FR-TO		FROM TO	LENGTH FR-TO	A-B	-3 / 0	-107.1 -107.1	0.05 (2) 10.00	B-AM	-7889 / 1589	-107.1 -107.1	0.17 (2) 4.23	AM-C	-11599 / 2404	-107.1 -107.1	0.25 (2) 3.53	C-D	-12746 / 2687	-107.1 -107.1	0.30 (2) 3.34	D-E	-12746 / 2687	-107.1 -107.1	0.30 (2) 3.34	E-F	-12548 / 2685	-107.1 -107.1	0.26 (2) 3.41	F-G	-15103 / 3260	-107.1 -107.1	0.46 (2) 3.03	G-H	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81	H-I	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81	I-J	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81	J-K	-18011 / 3833	-107.1 -107.1	0.60 (3) 2.71	K-L	-18011 / 3833	-107.1 -107.1	0.59 (3) 2.72	L-M	-18011 / 3833	-107.1 -107.1	0.59 (3) 2.72	M-N	-15075 / 3211	-107.1 -107.1	0.44 (3) 3.06	N-O	-15076 / 3213	-107.1 -107.1	0.47 (3) 3.03	O-P	-12553 / 2659	-107.1 -107.1	0.26 (3) 3.41	P-Q	-12745 / 2661	-107.1 -107.1	0.30 (3) 3.34	Q-R	-12745 / 2661	-107.1 -107.1	0.30 (3) 3.34	R-AO	-11599 / 2384	-107.1 -107.1	0.25 (3) 3.53	AO-S	-7888 / 1585	-107.1 -107.1	0.17 (3) 4.23	S-T	-3 / 0	-107.1 -107.1	0.05 (3) 10.00	B-AL	-1170 / 5329	-27.5 -27.5	0.27 (2) 6.25	AL-AK	-1170 / 5329	-27.5 -27.5	0.31 (2) 6.25	AK-AJ	-2282 / 10954	-27.5 -27.5	0.55 (2) 6.25	AJ-AI	-2476 / 12117	-27.5 -27.5	0.54 (2) 6.25	AI-AH	-2476 / 12117	-27.5 -27.5	0.54 (2) 6.25	AH-AG	-2378 / 11923	-27.5 -27.5	0.53 (2) 6.25	AG-AF	-3040 / 15102	-27.5 -27.5	0.69 (2) 6.25	AF-AE	-3040 / 15102	-27.5 -27.5	0.69 (2) 6.25	AE-AD	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89	AD-AC	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89	AC-AB	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89	AB-AA	-3372 / 17135	-27.5 -27.5	0.77 (3) 6.06	AA-Z	-3372 / 17135	-27.5 -27.5	0.78 (3) 6.06	Z-Y	-3372 / 17135	-27.5 -27.5	0.78 (3) 6.06	<p>CS: TC=0.60 (J-K:3), BC=0.81 (AD-AE:2), WB=0.99 (G-AG:3), SSI=0.23 (F-G:3)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LNS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10</p> <p>SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50</p> <p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</p> <p>NAIL VALUES</p> <table border="1"> <tr><th>PLATE</th><th>GRIP(DRY)</th><th>SHEAR</th><th>SECTION</th></tr> <tr><td></td><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td></tr> <tr><td></td><td>MAX</td><td>MIN</td><td>MAX</td></tr> <tr><td>MT20</td><td>618</td><td>354</td><td>1667</td></tr> <tr><td>MII16</td><td>473</td><td>276</td><td>2341</td></tr> <tr><td></td><td>822</td><td>1245</td><td>4454</td></tr> <tr><td></td><td>1656</td><td></td><td>1656</td></tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.90 (AG) (INPUT = 0.90) JSI METAL= 0.94 (S) (INPUT = 1.00)</p>				PLATE	GRIP(DRY)	SHEAR	SECTION		(PSI)	(PLI)	(PLI)		MAX	MIN	MAX	MT20	618	354	1667	MII16	473	276	2341		822	1245	4454		1656		1656
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B-AM	-7889 / 1589	-107.1 -107.1	0.17 (2) 4.23																																																																																																																																																																																																																																																																																																																																																																				
AM-C	-11599 / 2404	-107.1 -107.1	0.25 (2) 3.53																																																																																																																																																																																																																																																																																																																																																																				
C-D	-12746 / 2687	-107.1 -107.1	0.30 (2) 3.34																																																																																																																																																																																																																																																																																																																																																																				
D-E	-12746 / 2687	-107.1 -107.1	0.30 (2) 3.34																																																																																																																																																																																																																																																																																																																																																																				
E-F	-12548 / 2685	-107.1 -107.1	0.26 (2) 3.41																																																																																																																																																																																																																																																																																																																																																																				
F-G	-15103 / 3260	-107.1 -107.1	0.46 (2) 3.03																																																																																																																																																																																																																																																																																																																																																																				
G-H	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81																																																																																																																																																																																																																																																																																																																																																																				
H-I	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81																																																																																																																																																																																																																																																																																																																																																																				
I-J	-17099 / 3668	-107.1 -107.1	0.54 (2) 2.81																																																																																																																																																																																																																																																																																																																																																																				
J-K	-18011 / 3833	-107.1 -107.1	0.60 (3) 2.71																																																																																																																																																																																																																																																																																																																																																																				
K-L	-18011 / 3833	-107.1 -107.1	0.59 (3) 2.72																																																																																																																																																																																																																																																																																																																																																																				
L-M	-18011 / 3833	-107.1 -107.1	0.59 (3) 2.72																																																																																																																																																																																																																																																																																																																																																																				
M-N	-15075 / 3211	-107.1 -107.1	0.44 (3) 3.06																																																																																																																																																																																																																																																																																																																																																																				
N-O	-15076 / 3213	-107.1 -107.1	0.47 (3) 3.03																																																																																																																																																																																																																																																																																																																																																																				
O-P	-12553 / 2659	-107.1 -107.1	0.26 (3) 3.41																																																																																																																																																																																																																																																																																																																																																																				
P-Q	-12745 / 2661	-107.1 -107.1	0.30 (3) 3.34																																																																																																																																																																																																																																																																																																																																																																				
Q-R	-12745 / 2661	-107.1 -107.1	0.30 (3) 3.34																																																																																																																																																																																																																																																																																																																																																																				
R-AO	-11599 / 2384	-107.1 -107.1	0.25 (3) 3.53																																																																																																																																																																																																																																																																																																																																																																				
AO-S	-7888 / 1585	-107.1 -107.1	0.17 (3) 4.23																																																																																																																																																																																																																																																																																																																																																																				
S-T	-3 / 0	-107.1 -107.1	0.05 (3) 10.00																																																																																																																																																																																																																																																																																																																																																																				
B-AL	-1170 / 5329	-27.5 -27.5	0.27 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AL-AK	-1170 / 5329	-27.5 -27.5	0.31 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AK-AJ	-2282 / 10954	-27.5 -27.5	0.55 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AJ-AI	-2476 / 12117	-27.5 -27.5	0.54 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AI-AH	-2476 / 12117	-27.5 -27.5	0.54 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AH-AG	-2378 / 11923	-27.5 -27.5	0.53 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AG-AF	-3040 / 15102	-27.5 -27.5	0.69 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AF-AE	-3040 / 15102	-27.5 -27.5	0.69 (2) 6.25																																																																																																																																																																																																																																																																																																																																																																				
AE-AD	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89																																																																																																																																																																																																																																																																																																																																																																				
AD-AC	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89																																																																																																																																																																																																																																																																																																																																																																				
AC-AB	-3655 / 18029	-27.5 -27.5	0.81 (2) 5.89																																																																																																																																																																																																																																																																																																																																																																				
AB-AA	-3372 / 17135	-27.5 -27.5	0.77 (3) 6.06																																																																																																																																																																																																																																																																																																																																																																				
AA-Z	-3372 / 17135	-27.5 -27.5	0.78 (3) 6.06																																																																																																																																																																																																																																																																																																																																																																				
Z-Y	-3372 / 17135	-27.5 -27.5	0.78 (3) 6.06																																																																																																																																																																																																																																																																																																																																																																				
PLATE	GRIP(DRY)	SHEAR	SECTION																																																																																																																																																																																																																																																																																																																																																																				
	(PSI)	(PLI)	(PLI)																																																																																																																																																																																																																																																																																																																																																																				
	MAX	MIN	MAX																																																																																																																																																																																																																																																																																																																																																																				
MT20	618	354	1667																																																																																																																																																																																																																																																																																																																																																																				
MII16	473	276	2341																																																																																																																																																																																																																																																																																																																																																																				
	822	1245	4454																																																																																																																																																																																																																																																																																																																																																																				
	1656		1656																																																																																																																																																																																																																																																																																																																																																																				
<p>APEGA Permit No. P3837</p> <p>PROFESSIONAL ENGINEER ALBERTA</p> <p>CHRIS CORDOGIANNIS</p> <p>August 25, 2015</p>				<p>CONTINUED ON PAGE 2</p>																																																																																																																																																																																																																																																																																																																																																																			

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
Y-X	-2261 / 11927	-27.5 -27.5	0.54 (3)				6.25
X-W	-2358 / 12115	-27.5 -27.5	0.55 (3)				6.25
W-V	-2358 / 12115	-27.5 -27.5	0.55 (3)				6.25
V-U	-2165 / 10955	-27.5 -27.5	0.55 (3)				6.25
U-AN	-1065 / 5328	-27.5 -27.5	0.31 (3)				6.25
AN-S	-1065 / 5328	-27.5 -27.5	0.27 (3)				6.25

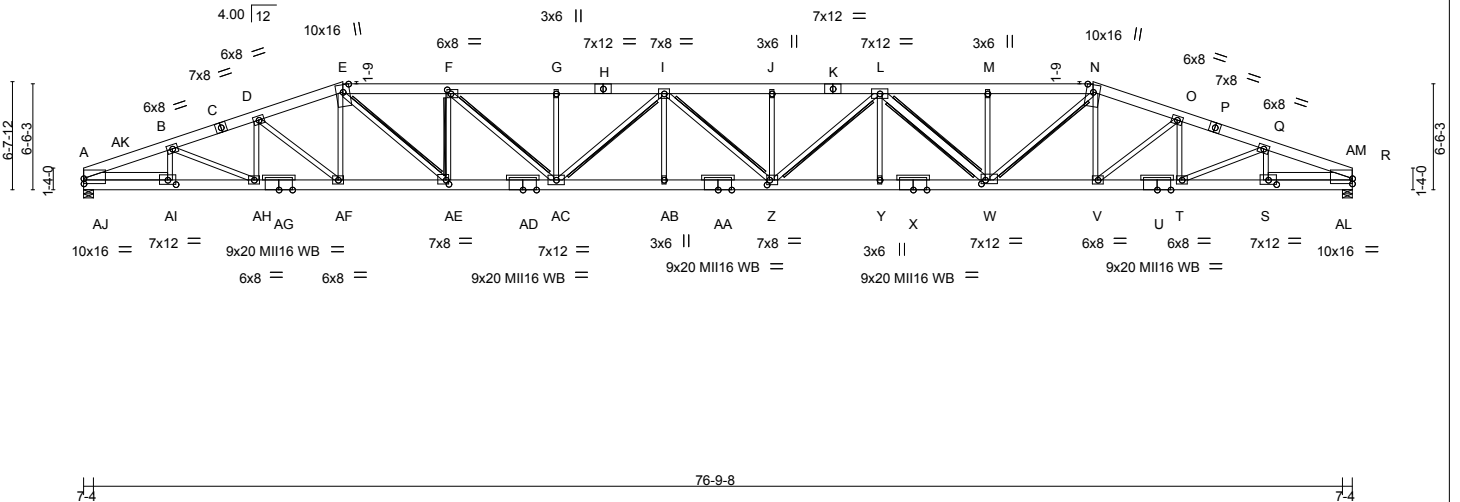
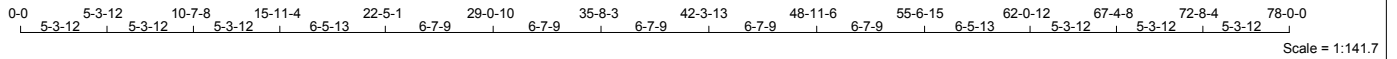
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7 } PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}; INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - E	2x8	DRY 1950F 1.7E	SPF
E - H	2x8	DRY 1950F 1.7E	SPF
H - K	2x8	DRY 1950F 1.7E	SPF
K - N	2x8	DRY 1950F 1.7E	SPF
N - P	2x8	DRY 1950F 1.7E	SPF
P - R	2x8	DRY 1950F 1.7E	SPF
A - AG	2x8	DRY 1950F 1.7E	SPF
AG - AD	2x8	DRY 1950F 1.7E	SPF
AD - AA	2x8	DRY 1950F 1.7E	SPF
AA - X	2x8	DRY 1950F 1.7E	SPF
X - U	2x8	DRY 1950F 1.7E	SPF
U - R	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT L - W 2x6 DRY No.2 SPF

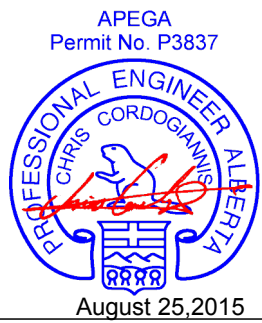
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	10.0	16.0	3.75
B, D, O, Q					
B	TMWW-t	MT20	6.0	8.0	
C	TS-t	MT20	7.0	8.0	
E	TTWW+m	MT20	10.0	16.0	Edge
F	TMWW-t	MT20	6.0	8.0	3.00 2.75
G, J, M					
G	TMW+w	MT20	3.0	6.0	
H	TS-t	MT20	7.0	12.0	
I	TMWWW-t	MT20	7.0	8.0	
K	TS-t	MT20	7.0	12.0	
L	TMWWW-t	MT20	7.0	12.0	
N	TTWW+m	MT20	10.0	16.0	Edge
P	TS-t	MT20	7.0	8.0	
R	TMBMW1-I	MT20	10.0	16.0	3.75
S	BMWW-t	MT20	7.0	12.0	3.25 6.00
T, V, AF, AH					
T	BMWW-t	MT20	6.0	8.0	
U, X, AA, AD, AG					
U	BS-t	MII16	9.0	20.0	
W	BMWWW-t	MT20	7.0	12.0	2.75 3.00
Y	BMW+w	MT20	3.0	6.0	
Z	BMWWW-t	MT20	7.0	8.0	3.50 2.00
AB	BMW+w	MT20	3.0	6.0	
AC	BMWWW-t	MT20	7.0	12.0	
AE	BMWW-t	MT20	7.0	8.0	3.00 2.00
AI	BMWW-t	MT20	7.0	12.0	3.25 6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	5249 0	5646 119	-1156 7-4	6-2
R	5249 0	5646 0	-1146 7-4	6-2

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1156 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT R FOR 1146 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 119 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. SNOW	COMPONENT LIVE	REACTIONS PERM. LIVE	WIND	DEAD	SOIL
A	4176	2724 / 0	780 / 0	0 / 0	0 / -1427	936 / 0	0 / 0
R	4176	2724 / 0	780 / 0	0 / 0	0 / -1420	936 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	85 / -85	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, R

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 2.71FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 5.89FT. OR RIGID CEILING DIRECTLY APPLIED.

2x3 DRY SPF 1650F 1.5E T-BRACE AT E-AE, F-AC, I-Z, L-Z, N-W
 2x3 DRY SPF 2100F 1.8E T-BRACE AT F-AE
 2x4 DRY SPF No.2 T-BRACE AT I-AC
 2x4 DRY SPF No.2 I-BRACE AT L-W

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING
 TOTAL LOAD CASES: (18)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LIVE CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-AK	-7889 / 1589	-107.1	-107.1	0.17 (2)	AI-B	-1281 / 347	0.14 (2)	
AK-B	-11599 / 2404	-107.1	-107.1	0.25 (2)	BA-AH	-290 / 1572	0.25 (3)	
B-C	-12746 / 2687	-107.1	-107.1	0.30 (2)	3.34	AH-D	-549 / 192	0.11 (3)
C-D	-12746 / 2687	-107.1	-107.1	0.30 (2)	3.34	D-AF	-238 / 665	0.13 (2)
D-E	-12548 / 2685	-107.1	-107.1	0.26 (2)	3.41	AF-E	-237 / 417	0.09 (10)
E-F	-15103 / 3260	-107.1	-107.1	0.46 (2)	3.03	E-AE	-998 / 4834	0.97 (14)
F-G	-17099 / 3668	-107.1	-107.1	0.54 (2)	2.81	AE-F	-3117 / 766	0.99 (3)
G-H	-17099 / 3668	-107.1	-107.1	0.54 (2)	2.81	F-AC	-645 / 3208	0.63 (14)
H-I	-17099 / 3668	-107.1	-107.1	0.54 (2)	2.81	AC-G	-784 / 273	0.30 (3)
I-J	-18011 / 3833	-107.1	-107.1	0.60 (3)	2.71	AG-I	-1780 / 372	0.82 (3)
J-K	-18011 / 3833	-107.1	-107.1	0.59 (3)	2.72	AB-I	0 / 314	0.05 (17)
K-L	-18011 / 3833	-107.1	-107.1	0.59 (3)	2.72	I-Z	-304 / 255	0.29 (9)
L-M	-15075 / 3211	-107.1	-107.1	0.44 (3)	3.06	Z-J	-771 / 272	0.29 (3)
M-N	-15076 / 3213	-107.1	-107.1	0.47 (3)	3.03	Z-L	-340 / 1702	0.33 (13)
N-O	-12553 / 2659	-107.1	-107.1	0.26 (3)	3.41	Y-L	0 / 329	0.05 (17)
O-P	-12745 / 2661	-107.1	-107.1	0.30 (3)	3.34	L-W	-3301 / 676	0.88 (2)
P-Q	-12745 / 2661	-107.1	-107.1	0.30 (3)	3.34	W-M	-953 / 325	0.36 (3)
Q-AM	-11599 / 2384	-107.1	-107.1	0.25 (3)	3.53	W-N	-984 / 4787	0.95 (13)
AM-R	-7888 / 1586	-107.1	-107.1	0.17 (3)	4.23	V-N	-228 / 433	0.09 (9)
						V-O	-247 / 698	0.13 (10)
A-AJ	-1169 / 5329	-27.5	-27.5	0.27 (2)	6.25	T-O	-567 / 195	0.11 (2)
AJ-AI	-1169 / 5329	-27.5	-27.5	0.31 (2)	6.25	T-Q	-290 / 1569	0.25 (2)
AI-AH	-2281 / 10954	-27.5	-27.5	0.55 (2)	6.25	S-Q	-1279 / 343	0.14 (3)
AH-AG	-2475 / 12117	-27.5	-27.5	0.54 (2)	6.25	AJ-AK	-27 / 452	0.00 (1)
AG-AF	-2475 / 12117	-27.5	-27.5	0.54 (2)	6.25	AK-AI	-1146 / 5760	0.68 (2)
AF-AE	-2377 / 11923	-27.5	-27.5	0.53 (2)	6.25	S-AM	-1126 / 5761	0.68 (3)
AE-AD	-3038 / 15102	-27.5	-27.5	0.69 (2)	6.25	AL-AM	-21 / 451	0.00 (1)
AD-AC	-3038 / 15102	-27.5	-27.5	0.69 (2)	6.25			
AC-AB	-3654 / 18029	-27.5	-27.5	0.81 (2)	5.89			
AB-AA	-3654 / 18029	-27.5	-27.5	0.81 (2)	5.89			
AA-Z	-3654 / 18029	-27.5	-27.5	0.81 (2)	5.89			
Z-Y	-3371 / 17135	-27.5	-27.5	0.77 (3)	6.07			
Y-X	-3371 / 17135	-27.5	-27.5	0.78 (3)	6.06			
X-W	-3371 / 17135	-27.5	-27.5	0.78 (3)	6.06			
W-V	-2259 / 11927	-27.5	-27.5	0.54 (3)	6.25			
V-U	-2355 / 12115	-27.5	-27.5	0.55 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL) = L/ 622 (1.50")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL) = L/ 462 (2.03")

CSI: TC=0.60 (I-J:3), BC=0.81 (AB-AC:2), WB=0.99 (F-AE:3), SSI=0.23 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LNS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (AE) (INPUT = 0.90)
 JSI METAL= 0.90 (R) (INPUT = 1.00)

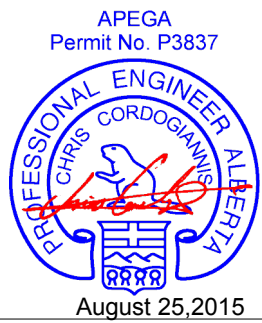
WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
U-T	-2355 / 12115	-27.5	-27.5	0.55 (3)	6.25		
T-S	-2162 / 10955	-27.5	-27.5	0.55 (3)	6.25		
S-AL	-1063 / 5328	-27.5	-27.5	0.31 (3)	6.25		
AL-R	-1063 / 5328	-27.5	-27.5	0.27 (3)	6.25		

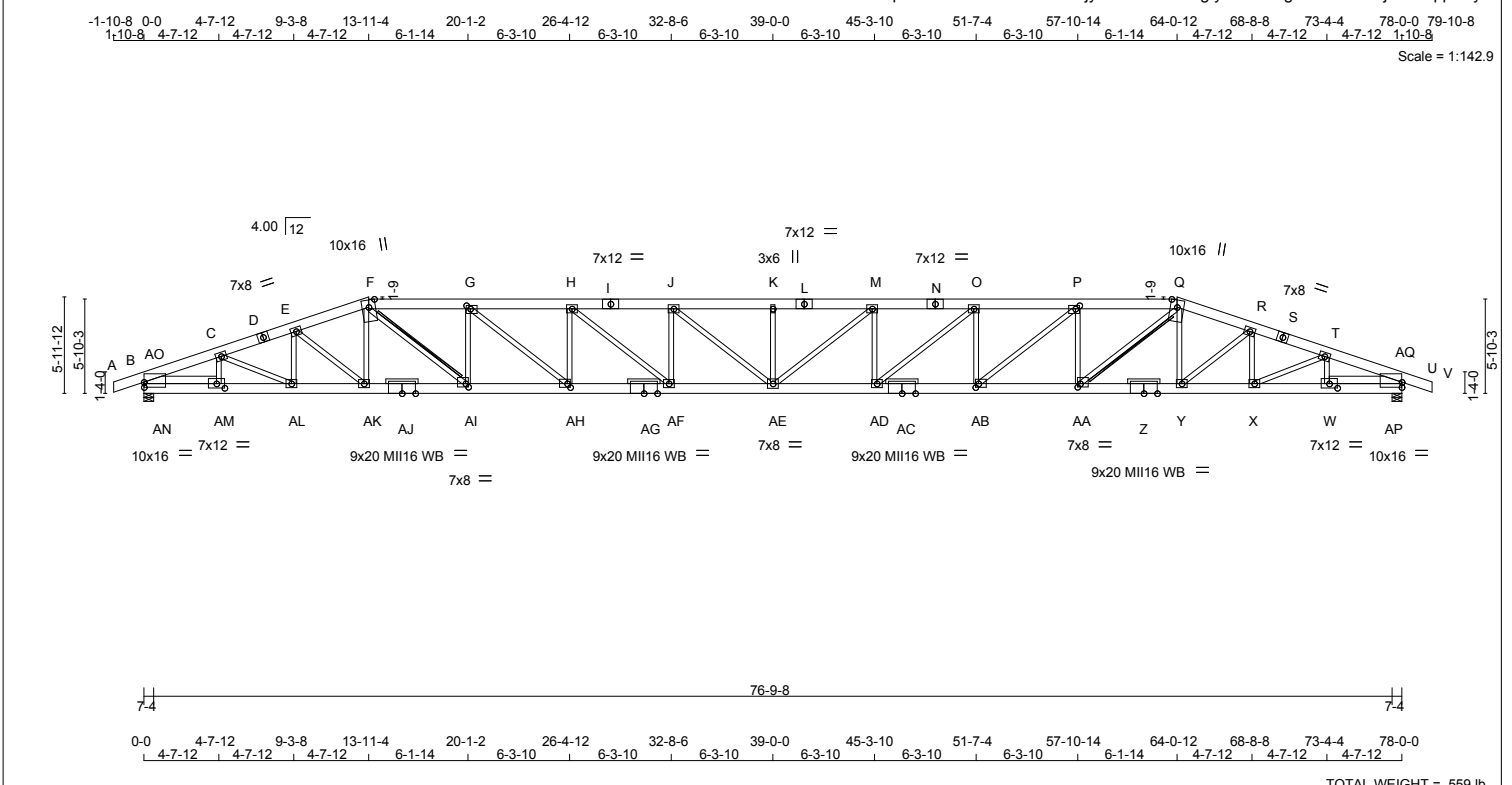
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2); BUILDING MAY BE LOCATED ON (OPEN TERRAIN); AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 559 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - F	2x8	DRY 1950F 1.7E	SPF
F - I	2x8	DRY 1950F 1.7E	SPF
I - L	2x8	DRY 1950F 1.7E	SPF
L - N	2x8	DRY 1950F 1.7E	SPF
N - Q	2x8	DRY 1950F 1.7E	SPF
Q - S	2x8	DRY 1950F 1.7E	SPF
S - V	2x8	DRY 1950F 1.7E	SPF
B - AJ	2x8	DRY 1950F 1.7E	SPF
AJ - AG	2x8	DRY 1950F 1.7E	SPF
AG - AC	2x8	DRY 1950F 1.7E	SPF
AC - Z	2x8	DRY 1950F 1.7E	SPF
Z - U	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	TYPE	NO.	DESCR.
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	16.0	3.75	
C, E, H, J, M, O, R, T						
C	TMWW-t	MT20	6.0	8.0		
D	TS-t	MT20	7.0	8.0		
F	TTWW+m	MT20	10.0	16.0	Edge	
G	TMWW-t	MT20	6.0	8.0	2.50	3.00
I, L, N						
I	TS-t	MT20	7.0	12.0		
K	TMWW+w	MT20	3.0	6.0		
P	TMWW-t	MT20	6.0	8.0	2.50	3.00
Q	TTWW+m	MT20	10.0	16.0	Edge	
S	TS-t	MT20	7.0	8.0		
U	TMBMW1-I	MT20	10.0	16.0	3.75	
W	BMWW-t	MT20	7.0	12.0	3.25	6.00
X, Y, AD, AF, AK, AL						
X	BMWW-t	MT20	6.0	8.0		
Z, AC, AG, AJ						
Z	BS-t	MII16	9.0	20.0		
AA	BMWW-t	MT20	7.0	8.0	2.50	2.00
AB	BMWW-t	MT20	6.0	8.0	2.75	2.25
AE	BMWW-t	MT20	7.0	8.0		
AH	BMWW-t	MT20	6.0	8.0	2.75	2.25
AI	BMWW-t	MT20	7.0	8.0	2.50	2.00
AM	BMWW-t	MT20	7.0	12.0	3.25	6.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	5461	0	5928	105	-1223	7-4
U	5461	0	5928	0	-1216	7-4

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1223 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1216 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 105 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SOIL				
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD	SOIL				
B	4320	2896 / 0	780 / 0	0 / 0	0 / -1488	956 / 0	0 / 0
U	4320	2896 / 0	780 / 0	0 / 0	0 / -1483	956 / 0	0 / 0

HORIZONTAL REACTIONS

JT	VERT	HORZ	UPLIFT	IN-SX	IN-SX
B	0 / 0	0 / 0	0 / 0	75 / -75	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, U

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.42FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 5.65FT. OR RIGID CEILING DIRECTLY APPLIED.

2x3 DRY SPF No.2 T-BRACE AT F-AI, Q-AA

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD	LC1 MAX	MAX UNBRAC	MEMB. FORCE (LBS)	MAX	CSI (LC)
FR-TO			FROM	TO	LENGTH	FR-TO		
A-B	-3 / 0		-107.1	-107.1	0.05 (2)	10.00	AM-C	-1460 / 372
B-AO	-7775 / 1570		-107.1	-107.1	0.15 (2)	4.28	C-AL	-346 / 1824
AO-C	-11292 / 2336		-107.1	-107.1	0.22 (2)	3.59	AL-E	-733 / 218
C-D	-12761 / 2677		-107.1	-107.1	0.29 (2)	3.34	E-AK	-117 / 887
D-E	-12761 / 2677		-107.1	-107.1	0.29 (2)	3.34	AK-F	-350 / 256
E-F	-12870 / 2732		-107.1	-107.1	0.25 (2)	3.38	F-AI	-1119 / 5436
F-G	-16060 / 3428		-107.1	-107.1	0.50 (2)	2.92	AI-G	-3419 / 822
G-H	-18703 / 3969		-107.1	-107.1	0.64 (2)	2.64	G-AH	-783 / 3864
H-I	-20236 / 4290		-107.1	-107.1	0.73 (2)	2.47	AH-H	-2301 / 580
I-J	-20236 / 4290		-107.1	-107.1	0.73 (2)	2.47	H-AF	-494 / 2417
J-K	-20625 / 4374		-107.1	-107.1	0.76 (3)	2.42	AF-J	-1368 / 393
K-L	-20625 / 4374		-107.1	-107.1	0.76 (2)	2.42	AD-M	-1368 / 393
L-M	-20625 / 4374		-107.1	-107.1	0.76 (2)	2.42	AE-K	-758 / 266
M-N	-20236 / 4243		-107.1	-107.1	0.73 (3)	2.47	AE-M	-187 / 931
N-O	-20236 / 4243		-107.1	-107.1	0.73 (3)	2.47	AD-M	-1368 / 393
O-P	-18703 / 3931		-107.1	-107.1	0.64 (3)	2.64	AD-O	-494 / 2417
P-Q	-16060 / 3399		-107.1	-107.1	0.50 (3)	2.92	AB-O	-2301 / 580
Q-R	-12871 / 2711		-107.1	-107.1	0.25 (3)	3.38	AB-P	-783 / 3864
R-S	-12761 / 2658		-107.1	-107.1	0.29 (3)	3.34	AA-P	-3419 / 822
S-T	-12761 / 2658		-107.1	-107.1	0.29 (3)	3.34	AA-Q	-1118 / 5436
T-AQ	-11292 / 2321		-107.1	-107.1	0.22 (3)	3.59	Y-Q	-365 / 257
AQ-U	-7775 / 1569		-107.1	-107.1	0.15 (3)	4.28	Y-R	-118 / 896
U-V	-3 / 0		-107.1	-107.1	0.05 (3)	10.00	X-R	-733 / 218
							X-T	-347 / 1824
B-AN	-1133 / 5208		-27.5	-27.5	0.25 (2)	6.25	W-T	-1460 / 368
AN-AM	-1133 / 5208		-27.5	-27.5	0.30 (2)	6.25	AN-AO	-22 / 353
AM-AL	-2213 / 10658		-27.5	-27.5	0.54 (2)	6.25	AO-AM	-1121 / 5629
AL-AK	-2476 / 12130		-27.5	-27.5	0.56 (2)	6.25	W-AQ	-1104 / 5629
AK-AJ	-2448 / 12247		-27.5	-27.5	0.55 (2)	6.25	AP-AQ	-16 / 354
AJ-AI	-2448 / 12247		-27.5	-27.5	0.55 (2)	6.25		
AI-AH	-3233 / 16059		-27.5	-27.5	0.73 (2)	6.16		
AH-AG	-3776 / 18703		-27.5	-27.5	0.85 (2)	5.82		
AG-AF	-3776 / 18703		-27.5	-27.5	0.85 (2)	5.82		
AF-AE	-4096 / 20236		-27.5	-27.5	0.92 (2)	5.65		
AE-AD	-4047 / 20236		-27.5	-27.5	0.92 (3)	5.67		
AD-AC	-3676 / 18703		-27.5	-27.5	0.85 (3)	5.88		
AC-AB	-3676 / 18703		-27.5	-27.5	0.85 (3)	5.88		
AB-AA	-3127 / 16059		-27.5	-27.5	0.73 (3)	6.23		
AA-Z	-2344 / 12247		-27.5	-27.5	0.55 (3)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF

DL = 5.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 4 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED

- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")

CALCULATED VERT. DEFL.(LL) = L/493 (1.90")

ALLOWABLE DEFL.(TL)= L/240 (3.90")

CALCULATED VERT. DEFL.(TL) = L/368 (2.54")

CSI: TC=0.76 (J-K:3), BC=0.92 (AD-AE:3), WB=0.99 (G-AI:3), SSI=0.22 (P-Q:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
MII16	473	276	2341

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (AH) (INPUT = 0.90)

JSI METAL= 0.93 (AG) (INPUT = 1.00)



CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A10	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324097
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:28 2015 Page 2
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LOADING

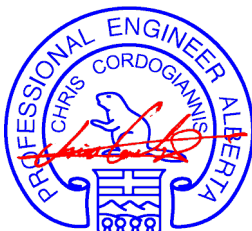
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
Z- Y	-2344 / 12247	-27.5 -27.5	0.55 (3)				6.25
Y- X	-2372 / 12130	-27.5 -27.5	0.56 (3)				6.25
X- W	-2109 / 10658	-27.5 -27.5	0.54 (3)				6.25
W-AP	-1040 / 5208	-27.5 -27.5	0.30 (3)				6.25
AP- U	-1040 / 5208	-27.5 -27.5	0.25 (3)				6.25

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2); BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

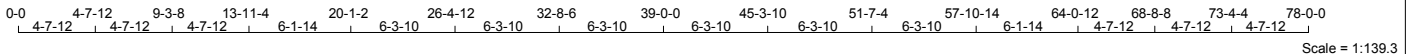
APEGA
Permit No. P3837



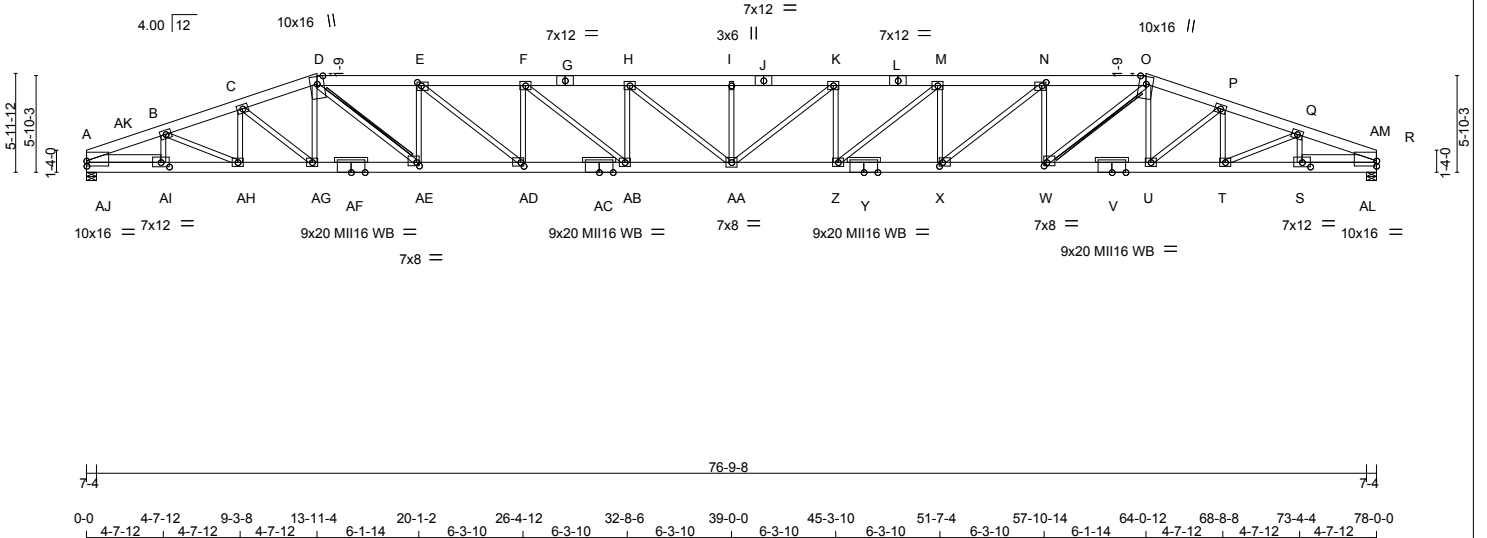
August 25, 2015

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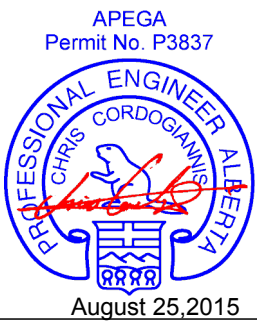


Scale = 1:139.3



TOTAL WEIGHT = 549 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - D 2x8 DRY 1950F 1.7E D - G 2x8 DRY 1950F 1.7E G - J 2x8 DRY 1950F 1.7E J - L 2x8 DRY 1950F 1.7E L - O 2x8 DRY 1950F 1.7E O - R 2x8 DRY 1950F 1.7E A - AF 2x8 DRY 1950F 1.7E AF - AC 2x8 DRY 1950F 1.7E AC - Y 2x8 DRY 1950F 1.7E Y - V 2x8 DRY 1950F 1.7E V - R 2x8 DRY 1950F 1.7E REINFORCING MEMBERS HW1 2x6 DRY No.2 SPF HW2 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON. BEARINGS <table border="1"> <thead> <tr> <th></th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>REQRD BRG</th> </tr> </thead> <tbody> <tr> <td>JT VERT</td> <td>5249</td> <td>0</td> <td>5688</td> <td>106</td> </tr> <tr> <td>Horz</td> <td>0</td> <td>5688</td> <td>0</td> <td>-1155</td> </tr> <tr> <td>UPLIFT</td> <td>0</td> <td>0</td> <td>0</td> <td>7-4</td> </tr> <tr> <td>IN-SX</td> <td>0</td> <td>0</td> <td>0</td> <td>6-3</td> </tr> </tbody> </table> PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1162 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT R FOR 1155 LBS FACTORED UPLIFT NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER PROVIDE FOR 106 LBS FACTORED HORIZONTAL REACTION AT JOINT A					FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT VERT	5249	0	5688	106	Horz	0	5688	0	-1155	UPLIFT	0	0	0	7-4	IN-SX	0	0	0	6-3	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 31.5 PSF DL = 5.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 53.5 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED - SLOPE REDUCTION FACTOR USED (74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (2.60") CALCULATED VERT. DEFL.(LL) = L/493 (1.90") ALLOWABLE DEFL.(TL)= L/240 (3.90") CALCULATED VERT. DEFL.(TL) = L/368 (2.54") CSI: TC=0.76 (H-I-3), BC=0.92 (Z-AA-3), WB=0.99 (E-AE-3), SSI=0.22 (N-O-2) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 MII16 473 276 2341 1245 4454 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (AD) (INPUT = 0.90) JSI METAL= 0.93 (AC) (INPUT = 1.00)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
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WB - INDICATES BLOCKING REQUIRED				JT TYPE	PLATES	W	LEN	Y	X	A	TMBMW1-t MT20	10.0	16.0	3.75		B, C, F, H, K, M, P, Q						B	TMWW-t MT20	6.0	8.0			D	TTWW+m MT20	10.0	16.0	Edge		E	TMWW-t MT20	6.0	8.0	2.50	3.00	G, J, L						G	TS-t MT20	7.0	12.0			I	TMW+w MT20	3.0	6.0			N	TMWW-t MT20	6.0	8.0	2.50	3.00	O	TTWW+m MT20	10.0	16.0	Edge		R	TMBMW1-t MT20	10.0	16.0	3.75		S	BMWW-t MT20	7.0	12.0	3.25	6.00	T, U, Z, AB, AG, AH						T	BMWW-t MT20	6.0	8.0			V, Y, AC, AF						V	BS-t MII16	9.0	20.0			W	BMWW-t MT20	7.0	8.0	2.50	2.00	X	BMWW-t MT20	6.0	8.0	2.75	2.25	AA	BMWW-t MT20	7.0	8.0			AD	BMWW-t MT20	6.0	8.0	2.75	2.25	AE	BMWW-t MT20	7.0	8.0	2.50	2.00	AI	BMWW-t MT20	7.0	12.0	3.25	6.00	UNFACTORED REACTIONS <table border="1"> <thead> <tr> <th>JT</th> <th>1ST LCASE</th> <th>MAX. MIN. COMPONENT REACTIONS</th> <th>SNOW</th> <th>LIVE</th> <th>PERM. LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>4176</td> <td>2752 / 0</td> <td>0 / 0</td> <td>780 / 0</td> <td>0 / 0</td> <td>0 / -1432</td> <td>936 / 0</td> <td>0 / 0</td> </tr> <tr> <td>R</td> <td>4176</td> <td>2752 / 0</td> <td>0 / 0</td> <td>780 / 0</td> <td>0 / 0</td> <td>0 / -1427</td> <td>936 / 0</td> <td>0 / 0</td> </tr> </tbody> </table> HORIZONTAL REACTIONS A --- 0 / 0 0 / 0 0 / 0 76 / -76 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, R BRACING MAX. UNBRACED TOP CHORD LENGTH = 2.42FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 5.65FT. OR RIGID CEILING DIRECTLY APPLIED. 2x3 DRY SPF No.2 T-BRACE AT D-AE, O-W FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.				JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	A	4176	2752 / 0	0 / 0	780 / 0	0 / 0	0 / -1432	936 / 0	0 / 0	R	4176	2752 / 0	0 / 0	780 / 0	0 / 0	0 / -1427	936 / 0	0 / 0	LOADING TOTAL LOAD CASES: (18) <table border="1"> <thead> <tr> <th colspan="3">CHORDS</th> <th colspan="3">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>MAX LC1 (LC)</th> <th>MAX UNBRAC LENGTH</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX CSI (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td></td> <td>FR-TO</td> <td></td> <td></td> </tr> <tr> <td>A-AK</td> <td>-7775 / 1571</td> <td>-107.1 -107.1</td> <td>0.15 (2)</td> <td>4.28</td> <td>AI-B</td> <td>-1460 / 372</td> <td>0.16 (2)</td> </tr> <tr> <td>AK-B</td> <td>-11292 / 2336</td> <td>-107.1 -107.1</td> <td>0.22 (2)</td> <td>3.59</td> <td>B-AH</td> <td>-346 / 1824</td> <td>0.29 (3)</td> </tr> <tr> <td>B-C</td> <td>-12761 / 2677</td> <td>-107.1 -107.1</td> <td>0.29 (2)</td> <td>3.34</td> <td>AH-C</td> <td>-733 / 218</td> <td>0.12 (3)</td> </tr> <tr> <td>C-D</td> <td>-12870 / 2732</td> <td>-107.1 -107.1</td> <td>0.25 (2)</td> <td>3.38</td> <td>C-AG</td> <td>-117 / 887</td> <td>0.14 (3)</td> </tr> <tr> <td>D-E</td> <td>-16060 / 3428</td> <td>-107.1 -107.1</td> <td>0.50 (2)</td> <td>2.92</td> <td>AG-D</td> <td>-350 / 256</td> <td>0.10 (10)</td> </tr> <tr> <td>E-F</td> <td>-18703 / 3969</td> <td>-107.1 -107.1</td> <td>0.64 (2)</td> <td>2.64</td> <td>D-AE</td> <td>-1119 / 5436</td> <td>0.88 (14)</td> </tr> <tr> <td>F-G</td> <td>-20236 / 4290</td> <td>-107.1 -107.1</td> <td>0.73 (2)</td> <td>2.47</td> <td>AE-E</td> <td>-3419 / 822</td> <td>0.99 (3)</td> </tr> <tr> <td>G-H</td> <td>-20236 / 4290</td> <td>-107.1 -107.1</td> <td>0.73 (2)</td> <td>2.47</td> <td>E-AD</td> <td>-783 / 3864</td> <td>0.72 (14)</td> </tr> <tr> <td>H-I</td> <td>-20625 / 4374</td> <td>-107.1 -107.1</td> <td>0.76 (3)</td> <td>2.42</td> <td>AD-F</td> <td>-2301 / 580</td> <td>0.66 (3)</td> </tr> <tr> <td>I-J</td> <td>-20625 / 4374</td> <td>-107.1 -107.1</td> <td>0.76 (2)</td> <td>2.42</td> <td>F-AB</td> <td>-494 / 2417</td> <td>0.45 (14)</td> </tr> <tr> <td>J-K</td> <td>-20625 / 4374</td> <td>-107.1 -107.1</td> <td>0.76 (2)</td> <td>2.42</td> <td>AB-H</td> <td>-1368 / 393</td> <td>0.39 (3)</td> </tr> <tr> <td>K-L</td> <td>-20236 / 4243</td> <td>-107.1 -107.1</td> <td>0.73 (3)</td> <td>2.47</td> <td>H-AA</td> <td>-186 / 931</td> <td>0.17 (14)</td> </tr> <tr> <td>L-M</td> <td>-20236 / 4243</td> <td>-107.1 -107.1</td> <td>0.73 (3)</td> <td>2.47</td> <td>AA-I</td> <td>-758 / 266</td> <td>0.22 (2)</td> </tr> <tr> <td>M-N</td> <td>-18703 / 3931</td> <td>-107.1 -107.1</td> <td>0.64 (3)</td> <td>2.64</td> <td>AA-K</td> <td>-187 / 931</td> <td>0.17 (13)</td> </tr> <tr> <td>N-O</td> <td>-16060 / 3399</td> <td>-107.1 -107.1</td> <td>0.50 (3)</td> <td>2.92</td> <td>Z-K</td> <td>-1368 / 393</td> <td>0.39 (2)</td> </tr> <tr> <td>O-P</td> <td>-12871 / 2711</td> <td>-107.1 -107.1</td> <td>0.25 (3)</td> <td>3.38</td> <td>Z-M</td> <td>-494 / 2417</td> <td>0.45 (13)</td> </tr> <tr> <td>P-Q</td> <td>-12761 / 2658</td> <td>-107.1 -107.1</td> <td>0.29 (3)</td> <td>3.34</td> <td>X-M</td> <td>-2301 / 580</td> <td>0.66 (2)</td> </tr> <tr> <td>Q-AM</td> <td>-11292 / 2321</td> <td>-107.1 -107.1</td> <td>0.22 (3)</td> <td>3.59</td> <td>X-N</td> <td>-783 / 3864</td> <td>0.72 (13)</td> </tr> <tr> <td>AM-R</td> <td>-7775 / 1569</td> <td>-107.1 -107.1</td> <td>0.15 (3)</td> <td>4.28</td> <td>W-N</td> <td>-3419 / 822</td> <td>0.99 (2)</td> </tr> <tr> <td></td> <td></td> <td></td> <td></td> <td></td> <td>W-O</td> <td>-1118 / 5436</td> <td>0.88 (13)</td> </tr> <tr> <td>A-AJ</td> <td>-1132 / 5208</td> <td>-27.5 -27.5</td> <td>0.25 (2)</td> <td>6.25</td> <td>U-O</td> <td>-365 / 257</td> <td>0.11 (9)</td> </tr> <tr> <td>AJ-AI</td> <td>-1132 / 5208</td> <td>-27.5 -27.5</td> <td>0.30 (2)</td> <td>6.25</td> <td>U-P</td> <td>-118 / 896</td> <td>0.14 (2)</td> </tr> <tr> <td>AI-AH</td> <td>-2212 / 10658</td> <td>-27.5 -27.5</td> <td>0.54 (2)</td> <td>6.25</td> <td>T-P</td> <td>-733 / 218</td> <td>0.12 (2)</td> </tr> <tr> <td>AH-AG</td> <td>-2475 / 12130</td> <td>-27.5 -27.5</td> <td>0.56 (2)</td> <td>6.25</td> <td>T-Q</td> <td>-347 / 1824</td> <td>0.29 (2)</td> </tr> <tr> <td>AG-AF</td> <td>-2447 / 12247</td> <td>-27.5 -27.5</td> <td>0.55 (2)</td> <td>6.25</td> <td>S-Q</td> <td>-1460 / 368</td> <td>0.16 (3)</td> </tr> <tr> <td>AF-AE</td> <td>-2447 / 12247</td> <td>-27.5 -27.5</td> <td>0.55 (2)</td> <td>6.25</td> <td>AJ-AK</td> <td>-22 / 353</td> <td>0.00 (1)</td> </tr> <tr> <td>AE-AD</td> <td>-3231 / 16059</td> <td>-27.5 -27.5</td> <td>0.73 (2)</td> <td>6.16</td> <td>AK-AI</td> <td>-1121 / 5629</td> <td>0.66 (2)</td> </tr> <tr> <td>AD-AC</td> <td>-3774 / 18703</td> <td>-27.5 -27.5</td> <td>0.85 (2)</td> <td>5.82</td> <td>AL-AM</td> <td>-17 / 354</td> <td>0.00 (1)</td> </tr> <tr> <td>AC-AB</td> <td>-3774 / 18703</td> <td>-27.5 -27.5</td> <td>0.85 (2)</td> <td>5.82</td> <td>AM-S</td> <td>-1104 / 5629</td> <td>0.66 (3)</td> </tr> <tr> <td>AB-AA</td> <td>-4095 / 20236</td> <td>-27.5 -27.5</td> <td>0.92 (2)</td> <td>5.65</td> <td></td> <td></td> <td></td> </tr> <tr> <td>AA-Z</td> <td>-4045 / 20236</td> <td>-27.5 -27.5</td> <td>0.92 (3)</td> <td>5.67</td> <td></td> <td></td> <td></td> </tr> <tr> <td>Z-Y</td> <td>-3675 / 18703</td> <td>-27.5 -27.5</td> <td>0.85 (3)</td> 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</table>				CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM TO			FR-TO			A-AK	-7775 / 1571	-107.1 -107.1	0.15 (2)	4.28	AI-B	-1460 / 372	0.16 (2)	AK-B	-11292 / 2336	-107.1 -107.1	0.22 (2)	3.59	B-AH	-346 / 1824	0.29 (3)	B-C	-12761 / 2677	-107.1 -107.1	0.29 (2)	3.34	AH-C	-733 / 218	0.12 (3)	C-D	-12870 / 2732	-107.1 -107.1	0.25 (2)	3.38	C-AG	-117 / 887	0.14 (3)	D-E	-16060 / 3428	-107.1 -107.1	0.50 (2)	2.92	AG-D	-350 / 256	0.10 (10)	E-F	-18703 / 3969	-107.1 -107.1	0.64 (2)	2.64	D-AE	-1119 / 5436	0.88 (14)	F-G	-20236 / 4290	-107.1 -107.1	0.73 (2)	2.47	AE-E	-3419 / 822	0.99 (3)	G-H	-20236 / 4290	-107.1 -107.1	0.73 (2)	2.47	E-AD	-783 / 3864	0.72 (14)	H-I	-20625 / 4374	-107.1 -107.1	0.76 (3)	2.42	AD-F	-2301 / 580	0.66 (3)	I-J	-20625 / 4374	-107.1 -107.1	0.76 (2)	2.42	F-AB	-494 / 2417	0.45 (14)	J-K	-20625 / 4374	-107.1 -107.1	0.76 (2)	2.42	AB-H	-1368 / 393	0.39 (3)	K-L	-20236 / 4243	-107.1 -107.1	0.73 (3)	2.47	H-AA	-186 / 931	0.17 (14)	L-M	-20236 / 4243	-107.1 -107.1	0.73 (3)	2.47	AA-I	-758 / 266	0.22 (2)	M-N	-18703 / 3931	-107.1 -107.1	0.64 (3)	2.64	AA-K	-187 / 931	0.17 (13)	N-O	-16060 / 3399	-107.1 -107.1	0.50 (3)	2.92	Z-K	-1368 / 393	0.39 (2)	O-P	-12871 / 2711	-107.1 -107.1	0.25 (3)	3.38	Z-M	-494 / 2417	0.45 (13)	P-Q	-12761 / 2658	-107.1 -107.1	0.29 (3)	3.34	X-M	-2301 / 580	0.66 (2)	Q-AM	-11292 / 2321	-107.1 -107.1	0.22 (3)	3.59	X-N	-783 / 3864	0.72 (13)	AM-R	-7775 / 1569	-107.1 -107.1	0.15 (3)	4.28	W-N	-3419 / 822	0.99 (2)						W-O	-1118 / 5436	0.88 (13)	A-AJ	-1132 / 5208	-27.5 -27.5	0.25 (2)	6.25	U-O	-365 / 257	0.11 (9)	AJ-AI	-1132 / 5208	-27.5 -27.5	0.30 (2)	6.25	U-P	-118 / 896	0.14 (2)	AI-AH	-2212 / 10658	-27.5 -27.5	0.54 (2)	6.25	T-P	-733 / 218	0.12 (2)	AH-AG	-2475 / 12130	-27.5 -27.5	0.56 (2)	6.25	T-Q	-347 / 1824	0.29 (2)	AG-AF	-2447 / 12247	-27.5 -27.5	0.55 (2)	6.25	S-Q	-1460 / 368	0.16 (3)	AF-AE	-2447 / 12247	-27.5 -27.5	0.55 (2)	6.25	AJ-AK	-22 / 353	0.00 (1)	AE-AD	-3231 / 16059	-27.5 -27.5	0.73 (2)	6.16	AK-AI	-1121 / 5629	0.66 (2)	AD-AC	-3774 / 18703	-27.5 -27.5	0.85 (2)	5.82	AL-AM	-17 / 354	0.00 (1)	AC-AB	-3774 / 18703	-27.5 -27.5	0.85 (2)	5.82	AM-S	-1104 / 5629	0.66 (3)	AB-AA	-4095 / 20236	-27.5 -27.5	0.92 (2)	5.65				AA-Z	-4045 / 20236	-27.5 -27.5	0.92 (3)	5.67				Z-Y	-3675 / 18703	-27.5 -27.5	0.85 (3)	5.88				Y-X	-3675 / 18703	-27.5 -27.5	0.85 (3)	5.88				X-W	-3125 / 16059	-27.5 -27.5	0.73 (3)	6.24				W-V	-2341 / 12247	-27.5 -27.5	0.55 (3)	6.25				V-U	-2341 / 12247	-27.5 -27.5	0.55 (3)	6.25				U-T	-2370 / 12130	-27.5 -27.5	0.56 (3)	6.25				T-S	-2107 / 10658	-27.5 -27.5	0.54 (3)	6.25				S-AL	-1038 / 5208	-27.5 -27.5	0.30 (3)	6.25			
JT TYPE	PLATES	W	LEN	Y	X																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
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MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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A-AK	-7775 / 1571	-107.1 -107.1	0.15 (2)	4.28	AI-B	-1460 / 372	0.16 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AK-B	-11292 / 2336	-107.1 -107.1	0.22 (2)	3.59	B-AH	-346 / 1824	0.29 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
B-C	-12761 / 2677	-107.1 -107.1	0.29 (2)	3.34	AH-C	-733 / 218	0.12 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
C-D	-12870 / 2732	-107.1 -107.1	0.25 (2)	3.38	C-AG	-117 / 887	0.14 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
D-E	-16060 / 3428	-107.1 -107.1	0.50 (2)	2.92	AG-D	-350 / 256	0.10 (10)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
E-F	-18703 / 3969	-107.1 -107.1	0.64 (2)	2.64	D-AE	-1119 / 5436	0.88 (14)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
F-G	-20236 / 4290	-107.1 -107.1	0.73 (2)	2.47	AE-E	-3419 / 822	0.99 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
G-H	-20236 / 4290	-107.1 -107.1	0.73 (2)	2.47	E-AD	-783 / 3864	0.72 (14)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
H-I	-20625 / 4374	-107.1 -107.1	0.76 (3)	2.42	AD-F	-2301 / 580	0.66 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
I-J	-20625 / 4374	-107.1 -107.1	0.76 (2)	2.42	F-AB	-494 / 2417	0.45 (14)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
J-K	-20625 / 4374	-107.1 -107.1	0.76 (2)	2.42	AB-H	-1368 / 393	0.39 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
K-L	-20236 / 4243	-107.1 -107.1	0.73 (3)	2.47	H-AA	-186 / 931	0.17 (14)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
L-M	-20236 / 4243	-107.1 -107.1	0.73 (3)	2.47	AA-I	-758 / 266	0.22 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
M-N	-18703 / 3931	-107.1 -107.1	0.64 (3)	2.64	AA-K	-187 / 931	0.17 (13)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
N-O	-16060 / 3399	-107.1 -107.1	0.50 (3)	2.92	Z-K	-1368 / 393	0.39 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
O-P	-12871 / 2711	-107.1 -107.1	0.25 (3)	3.38	Z-M	-494 / 2417	0.45 (13)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
P-Q	-12761 / 2658	-107.1 -107.1	0.29 (3)	3.34	X-M	-2301 / 580	0.66 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
Q-AM	-11292 / 2321	-107.1 -107.1	0.22 (3)	3.59	X-N	-783 / 3864	0.72 (13)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AM-R	-7775 / 1569	-107.1 -107.1	0.15 (3)	4.28	W-N	-3419 / 822	0.99 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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A-AJ	-1132 / 5208	-27.5 -27.5	0.25 (2)	6.25	U-O	-365 / 257	0.11 (9)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AJ-AI	-1132 / 5208	-27.5 -27.5	0.30 (2)	6.25	U-P	-118 / 896	0.14 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AI-AH	-2212 / 10658	-27.5 -27.5	0.54 (2)	6.25	T-P	-733 / 218	0.12 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AH-AG	-2475 / 12130	-27.5 -27.5	0.56 (2)	6.25	T-Q	-347 / 1824	0.29 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AG-AF	-2447 / 12247	-27.5 -27.5	0.55 (2)	6.25	S-Q	-1460 / 368	0.16 (3)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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AE-AD	-3231 / 16059	-27.5 -27.5	0.73 (2)	6.16	AK-AI	-1121 / 5629	0.66 (2)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
AD-AC	-3774 / 18703	-27.5 -27.5	0.85 (2)	5.82	AL-AM	-17 / 354	0.00 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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AB-AA	-4095 / 20236	-27.5 -27.5	0.92 (2)	5.65																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
AA-Z	-4045 / 20236	-27.5 -27.5	0.92 (3)	5.67																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
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X-W	-3125 / 16059	-27.5 -27.5	0.73 (3)	6.24																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
W-V	-2341 / 12247	-27.5 -27.5	0.55 (3)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
V-U	-2341 / 12247	-27.5 -27.5	0.55 (3)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
U-T	-2370 / 12130	-27.5 -27.5	0.56 (3)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
T-S	-2107 / 10658	-27.5 -27.5	0.54 (3)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										
S-AL	-1038 / 5208	-27.5 -27.5	0.30 (3)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																										



CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A10A	QUANTITY 1	PLY 1	JOB DESC. W/O# 090	DRWG NO. P4324098
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:31 2015 Page 2
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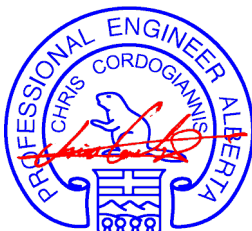
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
AL-R	-1038 / 5208	-27.5	-27.5	0.25 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, C_pC_g, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

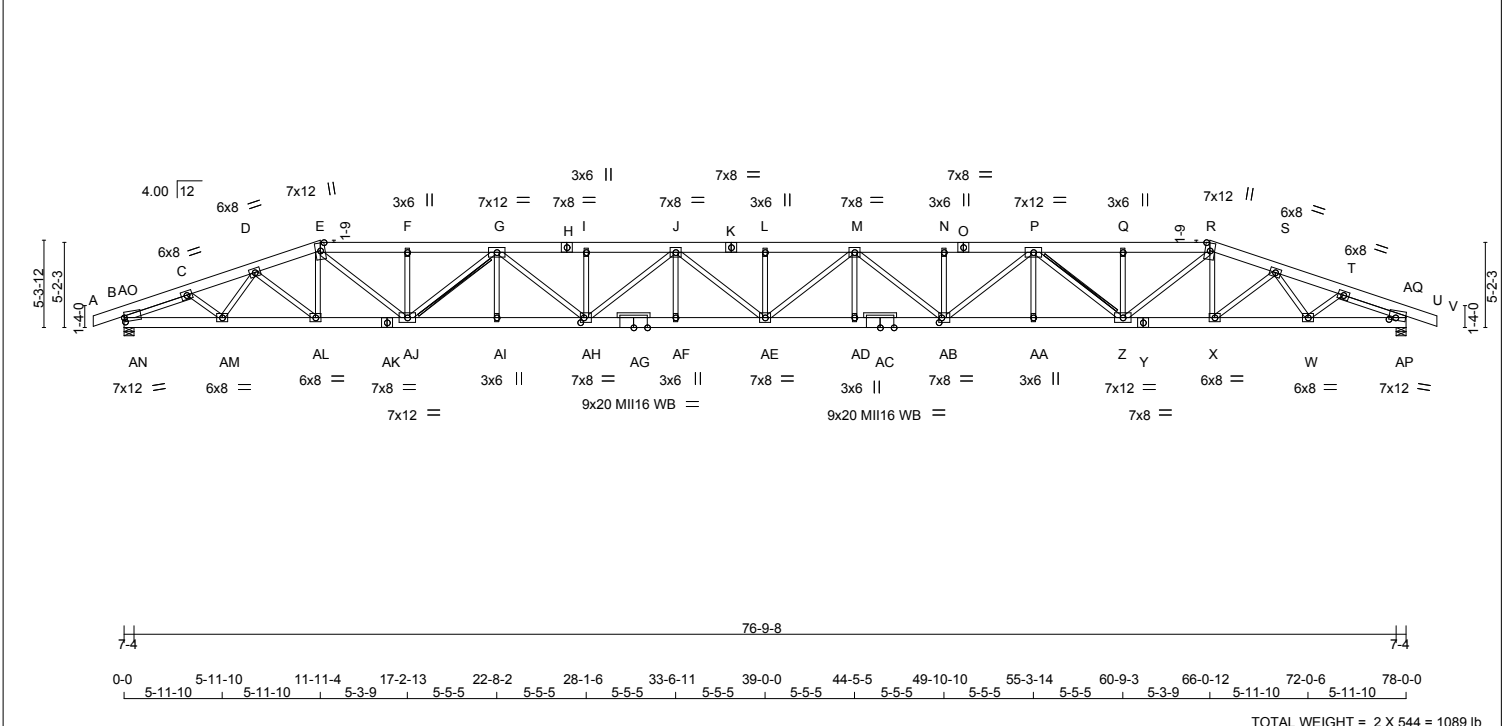
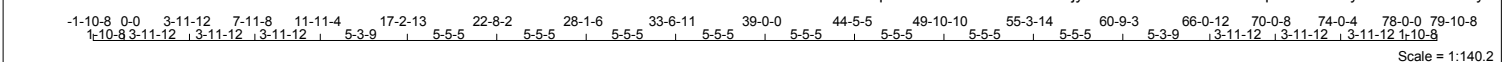
APEGA
Permit No. P3837



August 25, 2015

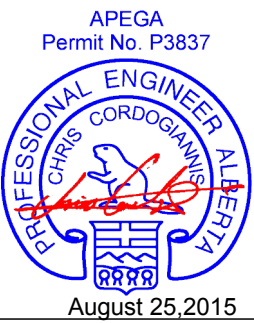
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 2 X 544 = 1089 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA					
N. L. G. A. RULES				TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.				SPECIFIED LOADS:					
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				TOP CH. LL = 31.5 PSF					
A - E	2x8	DRY 1950F 1.7E	SPF	FACTORED GROSS REACTION				DL = 5.0 PSF					
E - H	2x8	DRY 1950F 1.7E	SPF	MAXIMUM FACTORED GROSS REACTION				BOT CH. LL = 10.0 PSF					
H - K	2x8	DRY 1950F 1.7E	SPF	INPUT BRG				DL = 7.0 PSF					
K - O	2x8	DRY 1950F 1.7E	SPF	REQRD BRG				TOTAL LOAD = 53.5 PSF					
O - R	2x8	DRY 1950F 1.7E	SPF	JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX				SPACING = 24.0 IN. C/C					
R - V	2x8	DRY 1950F 1.7E	SPF	B 5426 0 5935 91 -1222 7-4 3-3				LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12					
B - AK	2x8	DRY 1950F 1.7E	SPF	AP(U) 5495 0 6002 0 -1231 7-4 3-4				THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBC 2010					
AK- AG	2x8	DRY 1950F 1.7E	SPF	PROVIDE ANCHORAGE AT BEARING JOINT B FOR 1222 LBS FACTORED UPLIFT				THIS DESIGN COMPLIES WITH:					
AG- AC	2x8	DRY 1950F 1.7E	SPF	PROVIDE ANCHORAGE AT BEARING JOINT U FOR 1231 LBS FACTORED UPLIFT				- PART 4 OF OBC 2012, BCBC 2012, ABC 2014					
AC- Y	2x8	DRY 1950F 1.7E	SPF	NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER				- CSA 086-09					
Y - U	2x8	DRY 1950F 1.7E	SPF	PROVIDE FOR 91 LBS FACTORED HORIZONTAL REACTION AT JOINT B				- TPIC 2011					
REINFORCING MEMBERS				UNFACTORED REACTIONS				DESIGN ASSUMPTIONS					
HW1	2x4	DRY No.2	SPF	1ST LCASE MAX/MIN COMPONENT REACTIONS				- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED					
HW2	2x4	DRY No.2	SPF	JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL				- SLOPE REDUCTION FACTOR USED					
ALL WEBS 2x4 DRY SEASONED LUMBER.				B 4293 2907 / 0 775 / 0 0 / 0 0 / -1484 950 / 0 0 / 0				74% OF 39.7 P.S.F. G.S.L. PLUS					
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:				AP(U) 4347 2938 / 0 785 / 0 0 / 0 0 / -1497 962 / 0 0 / 0				2.1 P.S.F. RAIN LOAD TIMES IMPORTANCE FACTOR EQUALS					
CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)				HORIZONTAL REACTIONS				31.5 P.S.F. SPECIFIED ROOF LIVE LOAD					
TOP CHORDS : (0.122"x3") SPIRAL NAILS				B --- 0 / 0 0 / 0 0 / 0 65 / -65 0 / 0 0 / 0				ALLOWABLE DEFL.(LL)= L/360 (2.60")					
A-E	2	12	TOP	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, AP(U)				CALCULATED VERT. DEFL.(LL) = L/786 (1.19")					
E-H	2	12	TOP	BRACING				ALLOWABLE DEFL.(TL)= L/240 (3.90")					
H-K	2	12	TOP	MAX. UNBRACED TOP CHORD LENGTH = 3.54FT.				CALCULATED VERT. DEFL.(TL) = L/590 (1.59")					
O-K	2	12	TOP	MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.				CSI: TC=0.30 (T-AQ:3), BC=0.53 (AE-AF:2), WB=0.84 (P-Z:2), SSI=0.84 (U-AP:3)					
O-R	2	12	TOP	2x4 DRY SPF No.2 T-BRACE AT G-AJ, P-Z				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10					
R-V	2	12	TOP	FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.				SNOW LOAD IMPORTANCE FACTOR = 1.00					
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS				LOADING				WIND LOAD IMPORTANCE FACTOR = 1.00					
B-AK	2	12	TOP	TOTAL LOAD CASES: (18)				LIVE LOAD IMPORTANCE FACTOR = 1.00					
AK- AG	2	12	TOP	C H O R D S				COMPANION LIVE LOAD FACTOR = 0.50					
AG- AC	2	12	TOP	MAX. FACTORED				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.					
Y- AC	2	12	TOP	MEMB. FORCE (LBS)				NAIL VALUES					
Y- U	2	12	TOP	FACTORED				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)					
WEBS : (0.122"x3") SPIRAL NAILS				W E B S				MAX MIN MAX MIN MAX MIN					
2x4	1	6	TOP	MEMB. FORCE (LBS)				MT20 618 354 1667 822 2284 1656					
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.				VERT. LOAD LC1 MAX UNBRAC				MII16 473 276 2341 1245 4454 1656					
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.				FR-TO				PLATE PLACEMENT TOL. = 0.250 inches					
PLATES (table is in inches)				FROM TO				PLATE ROTATION TOL. = 5.0 Deg.					
JT TYPE	PLATES	W	LEN	Y	X	FR-TO				JSI GRIP= 0.86 (AH) (INPUT = 0.90)			
B TMBMW1-m	MT20	7.0	12.0	3.00	0.25	FR-TO				JSI METAL= 0.70 (Y) (INPUT = 1.00)			
C, D, S, T						FR-TO							
E TMMWW-t	MT20	6.0	8.0			FR-TO							
C TTTWW+m	MT20	7.0	12.0	Edge		FR-TO							
F, I, L, N, Q						FR-TO							
F TMMWW-t	MT20	3.0	6.0			FR-TO							
G TMMWW-t	MT20	7.0	12.0			FR-TO							
H, K, O						FR-TO							
H TS-t	MT20	7.0	8.0			FR-TO							
J TMMWW-t	MT20	7.0	8.0			FR-TO							
M TMMWW-t	MT20	7.0	8.0			FR-TO							
P TMMWW-t	MT20	7.0	12.0			FR-TO							
R TTTWW+m	MT20	7.0	12.0	Edge		FR-TO							
U TMBMW1-m	MT20	7.0	12.0	2.00	4.75	FR-TO							
W, X, AL, AM						FR-TO							
W BMWW-t	MT20	6.0	8.0			FR-TO							

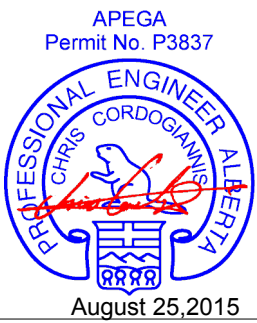


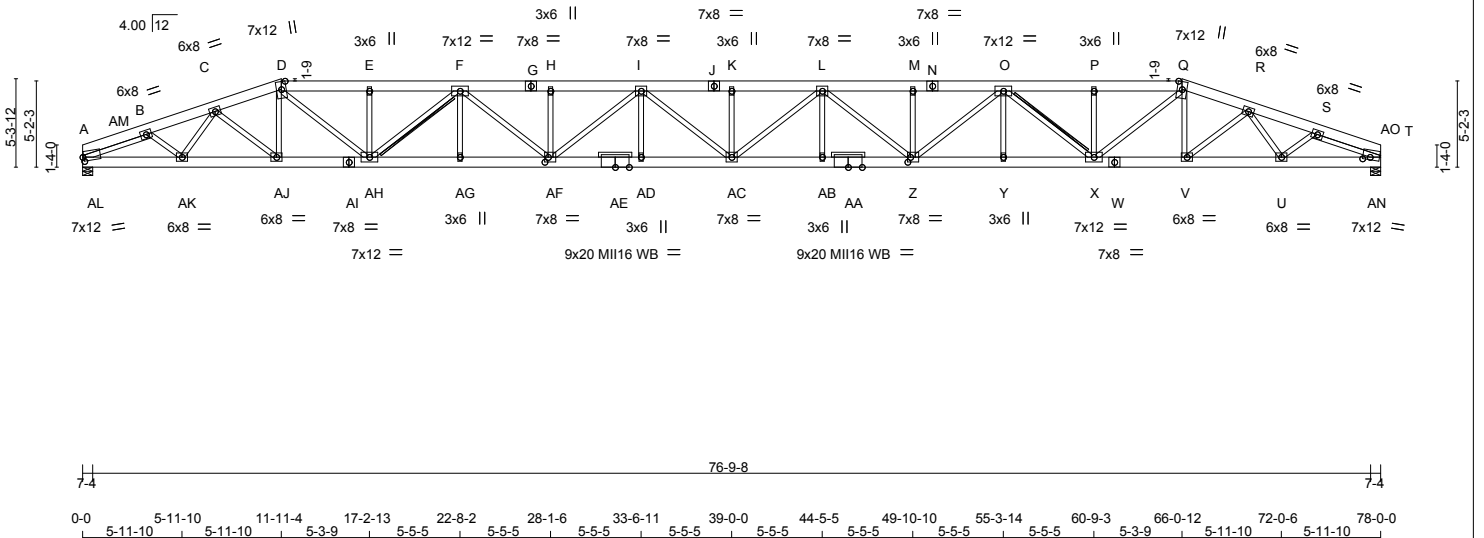
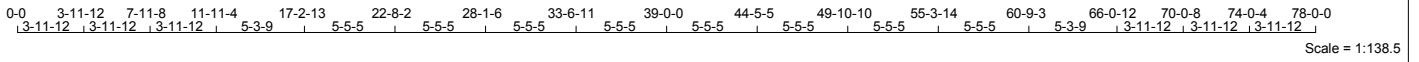
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JOB NAME G1507147	TRUSS NAME A11	QUANTITY 1	PLY 2	JOB DESC. W/O# 090	DRWG NO. P4324099
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Nelson Lumber, Grande Prairie, AB Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:37:34 2015 Page 2
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<p>PLATES (table is in inches)</p> <table border="1"> <thead> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> </thead> <tbody> <tr> <td>Y</td> <td>BS-t</td> <td>MT20</td> <td>7.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>Z</td> <td>BMWWW-t</td> <td>MT20</td> <td>7.0</td> <td>12.0</td> <td></td> <td></td> </tr> <tr> <td>AA, AD, AF, AI</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>AA</td> <td>BMW+w</td> <td>MT20</td> <td>3.0</td> <td>6.0</td> <td></td> <td></td> </tr> <tr> <td>AB</td> <td>BMWWW-t</td> <td>MT20</td> <td>7.0</td> <td>8.0</td> <td>3.50</td> <td>2.00</td> </tr> <tr> <td>AC</td> <td>BS-t</td> <td>MII16</td> <td>9.0</td> <td>20.0</td> <td></td> <td></td> </tr> <tr> <td>AE</td> <td>BMWWW-t</td> <td>MT20</td> <td>7.0</td> <td>8.0</td> <td></td> <td></td> </tr> <tr> <td>AG</td> <td>BS-t</td> <td>MII16</td> <td>9.0</td> <td>20.0</td> <td></td> <td></td> </tr> <tr> <td>AH</td> <td>BMWWW-t</td> <td>MT20</td> <td>7.0</td> <td>8.0</td> <td>3.50</td> <td>2.50</td> </tr> <tr> <td>AJ</td> <td>BMWWW-t</td> <td>MT20</td> <td>7.0</td> <td>12.0</td> <td></td> <td></td> </tr> <tr> <td>AK</td> <td>BS-t</td> <td>MT20</td> <td>7.0</td> <td>8.0</td> <td></td> <td></td> </tr> </tbody> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</p> <p>WB - INDICATES BLOCKING REQUIRED</p> <p>WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.</p>	JT	TYPE	PLATES	W	LEN	Y	X	Y	BS-t	MT20	7.0	8.0			Z	BMWWW-t	MT20	7.0	12.0			AA, AD, AF, AI							AA	BMW+w	MT20	3.0	6.0			AB	BMWWW-t	MT20	7.0	8.0	3.50	2.00	AC	BS-t	MII16	9.0	20.0			AE	BMWWW-t	MT20	7.0	8.0			AG	BS-t	MII16	9.0	20.0			AH	BMWWW-t	MT20	7.0	8.0	3.50	2.50	AJ	BMWWW-t	MT20	7.0	12.0			AK	BS-t	MT20	7.0	8.0			<p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <thead> <tr> <th colspan="4">C H O R D S</th> <th colspan="4">W E B S</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1 MAX (LC)</th> <th>MAX UNBRAC</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX. FACTORED FORCE (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td>FROM</td> <td>TO</td> <td>LENGTH</td> <td>FR-TO</td> <td></td> <td></td> </tr> <tr> <td>Z-Y</td> <td>-2306 / 11983</td> <td>-27.5</td> <td>-27.5</td> <td>0.29 (3)</td> <td></td> <td></td> <td>6.25</td> </tr> <tr> <td>Y-X</td> <td>-2306 / 11983</td> <td>-27.5</td> <td>-27.5</td> <td>0.29 (3)</td> <td></td> <td></td> <td>6.25</td> </tr> <tr> <td>X-W</td> <td>-2212 / 11241</td> <td>-27.5</td> <td>-27.5</td> <td>0.34 (3)</td> <td></td> <td></td> <td>6.25</td> </tr> <tr> <td>W-AP</td> <td>-1706 / 8553</td> <td>-27.5</td> <td>-27.5</td> <td>0.35 (3)</td> <td></td> <td></td> <td>6.25</td> </tr> <tr> <td>AP-U</td> <td>-881 / 4526</td> <td>-27.5</td> <td>-27.5</td> <td>0.27 (3)</td> <td></td> <td></td> <td>6.25</td> </tr> </tbody> </table> <p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2); BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)	FR-TO		FROM	TO	LENGTH	FR-TO			Z-Y	-2306 / 11983	-27.5	-27.5	0.29 (3)			6.25	Y-X	-2306 / 11983	-27.5	-27.5	0.29 (3)			6.25	X-W	-2212 / 11241	-27.5	-27.5	0.34 (3)			6.25	W-AP	-1706 / 8553	-27.5	-27.5	0.35 (3)			6.25	AP-U	-881 / 4526	-27.5	-27.5	0.27 (3)			6.25
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TOTAL WEIGHT = 2 X 535 = 1070 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x8	DRY 1950F 1.7E	SPF
D - G	2x8	DRY 1950F 1.7E	SPF
G - J	2x8	DRY 1950F 1.7E	SPF
J - N	2x8	DRY 1950F 1.7E	SPF
N - Q	2x8	DRY 1950F 1.7E	SPF
Q - T	2x8	DRY 1950F 1.7E	SPF
A - AI	2x8	DRY 1950F 1.7E	SPF
AI - AE	2x8	DRY 1950F 1.7E	SPF
AE - AA	2x8	DRY 1950F 1.7E	SPF
AA - W	2x8	DRY 1950F 1.7E	SPF
W - T	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

MEMBER	SIZE	QUANTITY	DESCR.
HW1	2x4	No.2	SPF
HW2	2x4	No.2	SPF

ALL WEBS 2x4 DRY SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

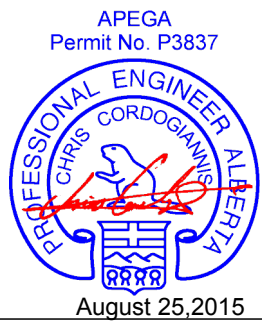
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	2 12	TOP
D-G	2 12	TOP
G-J	2 12	TOP
J-N	2 12	TOP
N-Q	2 12	TOP
Q-T	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A-AI	2 12	TOP
AI-AE	2 12	TOP
AE-AA	2 12	TOP
W-AA	2 12	TOP
W-T	2 12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	7.0	12.0	3.00 0.75
B, C, R, S					
D	TMWW-t	MT20	6.0	8.0	
B	TTWW+m	MT20	7.0	12.0	Edge
E, H, K, M, P					
E	TMW+w	MT20	3.0	6.0	
F	TMWWW-t	MT20	7.0	12.0	
G, J, N					
G	TS-t	MT20	7.0	8.0	
I	TMWWW-t	MT20	7.0	8.0	
L	TMWWW-t	MT20	7.0	8.0	
O	TMWWW-t	MT20	7.0	12.0	
Q	TTWW+m	MT20	7.0	12.0	Edge
T	TMBMW1-m	MT20	7.0	12.0	2.00 5.25
U, V, AJ, AK					
U	BMWW-t	MT20	6.0	8.0	



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
A	5217	0	5696	93
AN(T)	5282	0	5760	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1161 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT T FOR 1169 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 93 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	4150	2763 / 0	775 / 0	0 / 0	0 / -1428	930 / 0	0 / 0
AN(T)	4202	2794 / 0	785 / 0	0 / 0	0 / -1441	942 / 0	0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	66 / -66	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, AN(T)

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 3.54FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT F-AH, O-X

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

LOADING

TOTAL LOAD CASES: (18)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		MAX UNBRAC LENGTH	WEBS MAX. FACTORED FORCE (LBS)	
		FROM	TO		MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO						
A-AM	-8905 / 1761	-107.1	-107.1	0.27 (2)	5.16	B-AK -308 / 1921 0.15 (3)
AM-B	-6889 / 1444	-107.1	-107.1	0.26 (2)	5.69	AK-C -1798 / 426 0.16 (2)
B-C	-11548 / 2400	-107.1	-107.1	0.17 (2)	4.79	C-AJ -165 / 1168 0.09 (3)
C-D	-13055 / 2749	-107.1	-107.1	0.06 (2)	4.68	AJ-D -413 / 157 0.04 (10)
D-E	-16543 / 3496	-107.1	-107.1	0.15 (2)	4.20	D-AH -1180 / 5753 0.46 (3)
E-F	-16542 / 3494	-107.1	-107.1	0.15 (2)	4.20	AH-E -831 / 276 0.09 (2)
F-G	-22021 / 4616	-107.1	-107.1	0.24 (2)	3.67	AH-F -4542 / 930 0.82 (3)
G-H	-22021 / 4616	-107.1	-107.1	0.24 (2)	3.67	AG-F 0 / 263 0.02 (17)
H-I	-22021 / 4616	-107.1	-107.1	0.24 (2)	3.67	F-AF -644 / 3213 0.26 (3)
I-J	-23839 / 4955	-107.1	-107.1	0.27 (2)	3.54	AF-H -631 / 222 0.07 (3)
J-K	-23839 / 4955	-107.1	-107.1	0.27 (2)	3.54	AF-I -2000 / 418 0.62 (3)
K-L	-23639 / 4955	-107.1	-107.1	0.27 (2)	3.54	AD-I 0 / 246 0.02 (17)
L-M	-21810 / 4541	-107.1	-107.1	0.23 (3)	3.69	LAC -135 / 710 0.06 (3)
M-N	-21810 / 4541	-107.1	-107.1	0.23 (3)	3.69	AC-K -630 / 221 0.07 (3)
N-O	-21810 / 4541	-107.1	-107.1	0.23 (3)	3.69	AC-L -157 / 809 0.07 (2)
O-P	-16175 / 3401	-107.1	-107.1	0.15 (3)	4.24	AB-L 0 / 246 0.02 (17)
P-Q	-16176 / 3403	-107.1	-107.1	0.15 (3)	4.24	L-Z -2100 / 437 0.65 (2)
Q-R	-12609 / 2645	-107.1	-107.1	0.06 (3)	4.74	Z-M -631 / 222 0.07 (2)
R-S	-10514 / 2180	-107.1	-107.1	0.17 (3)	4.96	Z-O -666 / 3312 0.27 (2)
S-AO	-5191 / 1094	-107.1	-107.1	0.30 (3)	6.25	Y-O 0 / 266 0.02 (17)
AO-T	-4886 / 938	-107.1	-107.1	0.29 (3)	6.25	O-X -4646 / 950 0.84 (2)
						X-P -830 / 275 0.09 (3)
A-AL	-1321 / 6240	-27.5	-27.5	0.19 (2)	6.25	X-Q -1196 / 5831 0.47 (2)
AL-AK	-2043 / 9764	-27.5	-27.5	0.28 (2)	6.25	V-Q -559 / 158 0.06 (9)
AK-AJ	-2452 / 11975	-27.5	-27.5	0.32 (2)	6.25	V-R -229 / 1467 0.12 (2)
AJ-AI	-2486 / 12426	-27.5	-27.5	0.29 (2)	6.25	R-U -2333 / 524 0.20 (3)
AI-AH	-2486 / 12426	-27.5	-27.5	0.29 (2)	6.25	U-S -355 / 2142 0.17 (2)
AH-AG	-3994 / 19790	-27.5	-27.5	0.45 (2)	6.25	AL-AM -597 / 3419 0.00 (1)
AG-AF	-3994 / 19790	-27.5	-27.5	0.45 (2)	6.25	AL-B -4080 / 839 0.46 (2)
AF-AE	-4723 / 23324	-27.5	-27.5	0.53 (2)	6.25	AN-AO 0 / 344 0.00 (1)
AE-AD	-4723 / 23324	-27.5	-27.5	0.53 (2)	6.25	AN-S -4640 / 949 0.52 (3)
AD-AC	-4723 / 23324	-27.5	-27.5	0.53 (2)	6.25	
AC-AB	-4671 / 23189	-27.5	-27.5	0.53 (3)	6.25	
AB-AA	-4671 / 23189	-27.5	-27.5	0.52 (3)	6.25	
AA-Z	-4671 / 23189	-27.5	-27.5	0.52 (3)	6.25	
Z-Y	-3843 / 19503	-27.5	-27.5	0.44 (3)	6.25	
Y-X	-3843 / 19503	-27.5	-27.5	0.45 (3)	6.25	
X-W	-2309 / 12004	-27.5	-27.5	0.29 (3)	6.25	
W-V	-2309 / 12004	-27.5	-27.5	0.29 (3)	6.25	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF
DL = 5.0 PSF

BOT CH. LL = 10.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
CALCULATED VERT. DEFL.(LL) = L/786 (1.19")
ALLOWABLE DEFL.(TL)= L/240 (3.90")
CALCULATED VERT. DEFL.(TL) = L/590 (1.59")

CSI: TC=0.30 (S-AO:3), BC=0.53 (AC-AD:2), WB=0.84 (O-X:2), SSI=0.82 (T-AN:3)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656	
III16	473	276	2341	1245	4454	1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (AF) (INPUT = 0.90)
JSI METAL= 0.70 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME A11A	QUANTITY 1	PLY 2	JOB DESC. W/O# 090	DRWG NO. P4324100
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Nelson Lumber, Grande Prairie, AB

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ID:Uqb72Eaw6FPE10XPJ82uYjyi3SS-IHcG7RnXrQq1DhYrW3ScoHA3Dp5wEpDvsjWQVBykfy

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
W	BS-t	MT20	7.0	8.0		
X	BMWWW-t	MT20	7.0	12.0		
Y, AB, AD, AG						
Y	BMW+w	MT20	3.0	6.0		
Z	BMWWW-t	MT20	7.0	8.0	3.50	2.00
AA	BS-t	MII16	9.0	20.0		
AC	BMWWW-t	MT20	7.0	8.0		
AE	BS-t	MII16	9.0	20.0		
AF	BMWWW-t	MT20	7.0	8.0	3.50	2.50
AH	BMWWW-t	MT20	7.0	12.0		
AI	BS-t	MT20	7.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

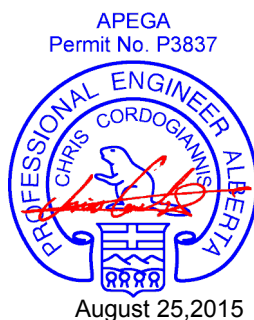
LOADING

TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
V-U	-2218 / 11274	-27.5	-27.5	0.34 (3)	6.25		
U-AN	-1717 / 8604	-27.5	-27.5	0.35 (3)	6.25		
AN-T	-897 / 4596	-27.5	-27.5	0.26 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.



JOB NAME G1507147	TRUSS NAME A12	QUANTITY 1	PLY 2	JOB DESC. W/O# 090	DRWG NO. P4324101
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Nelson Lumber, Grande Prairie, AB

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	TTWW+m	MT20	7.0	12.0	Edge	3.75
S	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
U, V, AK, AL						
U	BMWW-t	MT20	6.0	8.0		
W	BS-t	MT20	7.0	8.0		
X	BMWW-t	MT20	6.0	8.0	3.00	3.00
Y	BMWW-t	MT20	7.0	12.0		
Z, AD, AG						
Z	BS-t	MII16	9.0	20.0		
AA, AC, AF						
AA	BMW+w	MT20	3.0	6.0		
AB	BMWW-t	MT20	7.0	8.0		
AE	BMWW-t	MT20	7.0	8.0		
AH	BMWW-t	MT20	7.0	12.0		
AI	BMWW-t	MT20	6.0	8.0	3.00	3.00
AJ	BS-t	MT20	7.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO			FR-TO		
W-V	-2452 / 12616	-27.5	-27.5	0.30 (3)			6.25
V-U	-2145 / 10807	-27.5	-27.5	0.30 (3)			6.25
U-AO	-1230 / 6168	-27.5	-27.5	0.21 (3)			6.25
AO-S	-1230 / 6168	-27.5	-27.5	0.19 (3)			6.25

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM); INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2); BUILDING MAY BE LOCATED ON (OPEN TERRAIN); AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



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JOB NAME G1507147	TRUSS NAME A12A	QUANTITY 1	PLY 2	JOB DESC. W/O# 090	DRWG NO. P4324102
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Nelson Lumber, Grande Prairie, AB

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ID:Uqb72Eaw6FPE10XPJ82uYji3SS-7R_XOUslQGbAxc0?sKZ12YQ4VE6VeZZoEzZkjykykks

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	TTWW+m	MT20	7.0	12.0	Edge	3.75
R	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
S, T, AI, AJ						
S	BMWW-t	MT20	6.0	8.0		
U	BS-t	MT20	7.0	8.0		
V	BMWW-t	MT20	6.0	8.0	3.00	3.00
W	BMWW-t	MT20	7.0	12.0		
X, AB, AE						
X	BS-t	MII16	9.0	20.0		
Y, AA, AD						
Y	BMW+w	MT20	3.0	6.0		
Z	BMWW-t	MT20	7.0	8.0		
AC	BMWW-t	MT20	7.0	8.0		
AF	BMWW-t	MT20	7.0	12.0		
AG	BMWW-t	MT20	6.0	8.0	3.00	3.00
AH	BS-t	MT20	7.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX UNBRAC	MEMB. FORCE (LBS)	MAX FACTORED FORCE (LBS)	MAX FACTORED FORCE (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
S-AM	-1228 / 6168	-27.5	-27.5	0.21 (3)	6.25		
AM-R	-1228 / 6168	-27.5	-27.5	0.19 (3)	6.25		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



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JOB NAME G1507147	TRUSS NAME A13	QUANTITY 1	PLY 4	JOB DESC. W/O# 090	DRWG NO. P4324103
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Nelson Lumber, Grande Prairie, AB

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
C	TMWW-t	MT20	6.0	8.0		
D	TTWW+m	MT20	7.0	12.0	6.00	3.25
E, G, J, M, O, R, U						
E	TMW+w	MT20	3.0	6.0		
F, I, T						
F	TMWWW-t	MT20	7.0	12.0		
H, L, P, S						
H	TS-t	MT20	7.0	12.0		
K, N, Q						
K	TMWWW-t	MT20	7.0	8.0		
V	TMWW-t	MT20	6.0	8.0	3.00	3.25
W	TTWW+m	MT20	7.0	12.0	6.00	3.25
X	TMWW-t	MT20	6.0	8.0		
Y	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
AA, AB, AV, AW						
AA	BMWW-t	MT20	6.0	8.0		
AC	BMWW-t	MT20	6.0	8.0	3.00	2.75
AD	BS-t	MT20	10.0	16.0		7.50
AE, AG, AR						
AE	BMWWW-t	MT20	7.0	12.0		
AF, AI, AK, AN, AQ, AS						
AF	BMW+w	MT20	3.0	6.0		
AH, AL, AP, AT						
AH	BS-t	MII16	9.0	20.0		
AJ	BMWWW-t	MT20	7.0	8.0		
AM	BMWWW-t	MT20	7.0	8.0		
AO	BMWWW-t	MT20	7.0	8.0	3.50	2.50
AU	BMWWW-t	MT20	7.0	12.0	3.50	4.50

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 435.3 lbs FACTORED DOWN AND 66.4 lbs FACTORED UP AT 76-0-12, AND 435.3 lbs FACTORED DOWN AND 66.4 lbs FACTORED UP AT 1-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

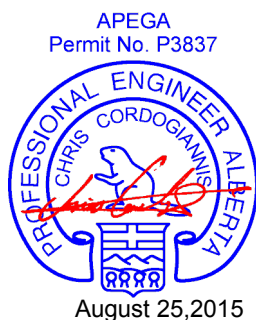
C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO		FROM	TO		LENGTH	FR-TO		
AL-AK	-11359 / 69035	-253.6	-253.6	0.88 (3)	6.25			
AK-AJ	-11359 / 69035	-253.6	-253.6	0.88 (3)	6.25			
AJ-AI	-10570 / 64351	-253.6	-253.6	0.82 (3)	6.25			
AI-AH	-10570 / 64351	-253.6	-253.6	0.82 (3)	6.25			
AH-AG	-10570 / 64351	-253.6	-253.6	0.82 (3)	6.25			
AG-AF	-8748 / 53359	-253.6	-253.6	0.69 (3)	6.25			
AF-AE	-8748 / 53359	-253.6	-253.6	0.69 (3)	6.25			
AE-AD	-5890 / 36001	-253.6	-253.6	0.48 (3)	6.25			
AD-AC	-5890 / 36001	-253.6	-253.6	0.48 (3)	6.25			
AC-AB	-4218 / 25859	-253.6	-253.6	0.34 (3)	6.25			
AB-AA	-3399 / 20779	-253.6	-253.6	0.33 (3)	6.25			
AA-BF	-2281 / 13923	-253.6	-253.6	0.25 (3)	6.25			
BF-BG	-2281 / 13923	-13.8	-13.8	0.25 (3)	6.25			
BG-AZ	-2281 / 13923	-13.8	-13.8	0.25 (3)	6.25			
AZ-Y	-2281 / 13923	-13.8	-13.8	0.25 (3)	6.25			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
BD	1-11-4	-392	-435	66	FRONT	VERT	TOTAL
BG	76-0-12	-392	-435	66	FRONT	VERT	TOTAL

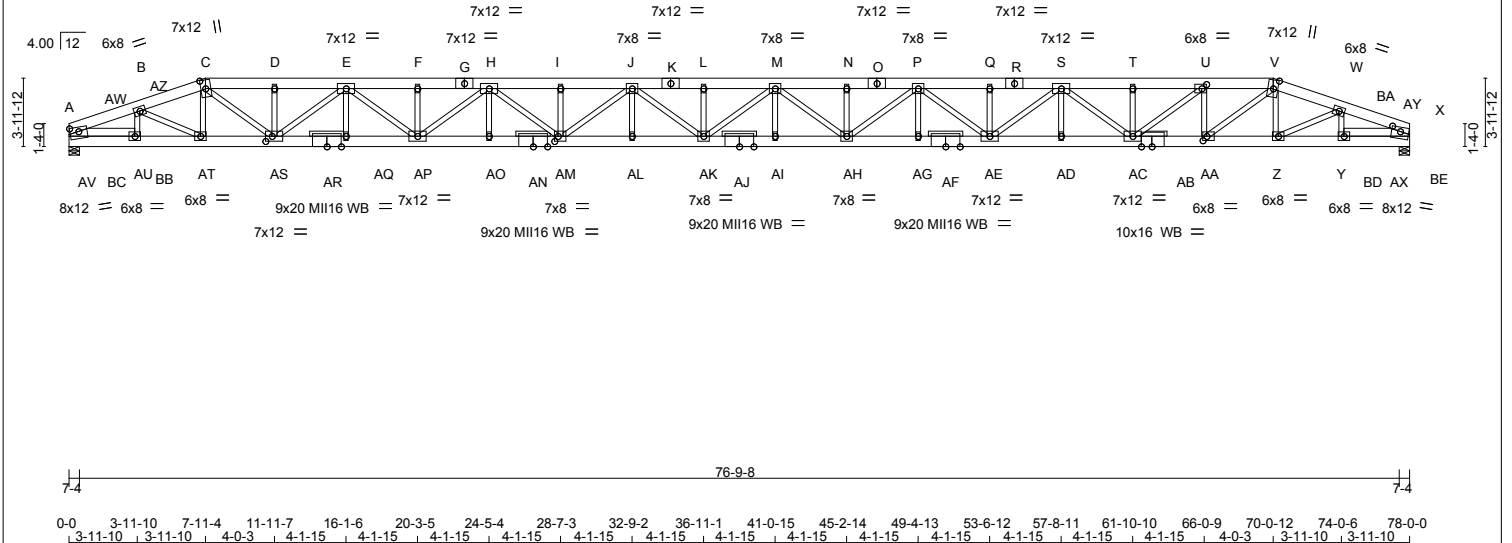
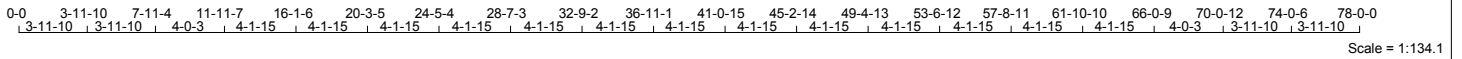
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22.0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-08 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8	DRY 1950F 1.7E	SPF
C - G	2x8	DRY 1950F 1.7E	SPF
G - K	2x8	DRY 1950F 1.7E	SPF
K - O	2x8	DRY 1950F 1.7E	SPF
O - R	2x8	DRY 1950F 1.7E	SPF
R - V	2x8	DRY 1950F 1.7E	SPF
V - X	2x8	DRY 1950F 1.7E	SPF
A - AR	2x8	DRY 1950F 1.7E	SPF
AR - AN	2x8	DRY 1950F 1.7E	SPF
AN - AJ	2x8	DRY 1950F 1.7E	SPF
AJ - AF	2x8	DRY 1950F 1.7E	SPF
AF - AB	2x8	DRY 1950F 1.7E	SPF
AB - X	2x8	DRY 1950F 1.7E	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

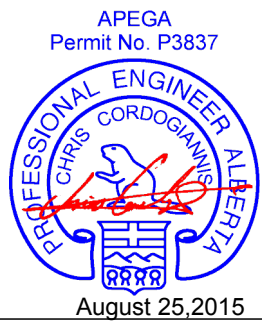
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : 10D (0.120"x3") NAILS		
A - C	2 12	SIDE(21.7)
C - G	2 12	SIDE(22.8)
G - K	2 12	SIDE(22.8)
K - O	2 12	SIDE(22.8)
R - O	2 12	SIDE(22.8)
R - V	2 12	SIDE(22.8)
V - X	2 12	SIDE(21.7)
BOTTOM CHORDS : 10D (0.120"x3") NAILS		
A - AR	2 12	SIDE(200.2)
AR - AN	2 12	SIDE(200.2)
AN - AJ	2 12	SIDE(200.2)
AJ - AF	2 12	SIDE(200.2)
AF - AB	2 12	SIDE(200.2)
AB - X	2 12	SIDE(200.2)
WEBS : 10D (0.120"x3") NAILS		
2x4	1 6	
2x6	2 6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	HORZ	DOWN	HORZ
A 10809	0	11971	34
X 10809	0	11971	0

PROVIDE ANCHORAGE AT BEARING JOINT A FOR 1974 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT X FOR 1973 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BUILDG. DESIGNER

PROVIDE FOR 34 LBS FACTORED HORIZONTAL REACTION AT JOINT A

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
A 8598	5839 / 0 1606 / 0 0 / 0 0 / -2649 1927 / 0 0 / 0
X 8598	5839 / 0 1606 / 0 0 / 0 0 / -2648 1927 / 0 0 / 0

HORIZONTAL REACTIONS

A	0 / 0	0 / 0	0 / 0	24 / -24	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, X

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.64FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (18)

C H O R D S		W E B S	
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH
A-AW	-19387 / 3190	-53.6 -53.6	0.19 (2)
AW-AZ	-22359 / 3696	-53.6 -53.6	0.20 (2)
AZ-B	-22359 / 3696	-26.8 -26.8	0.20 (2)
B-C	-27091 / 4481	-26.8 -26.8	0.19 (2)
C-D	-36005 / 5959	-26.8 -26.8	0.19 (2)
D-E	-36005 / 5959	-26.8 -26.8	0.20 (2)
E-F	-53386 / 8823	-26.8 -26.8	0.38 (2)
F-G	-53386 / 8823	-26.8 -26.8	0.39 (2)
G-H	-53386 / 8823	-26.8 -26.8	0.39 (2)
H-I	-64389 / 10636	-26.8 -26.8	0.54 (2)
I-J	-64389 / 10636	-26.8 -26.8	0.54 (2)
J-K	-69073 / 11408	-26.8 -26.8	0.61 (2)
K-L	-69073 / 11408	-26.8 -26.8	0.61 (2)
L-M	-69073 / 11408	-26.8 -26.8	0.61 (2)
M-N	-67520 / 11139	-26.8 -26.8	0.59 (3)
N-O	-67520 / 11139	-26.8 -26.8	0.59 (3)
O-P	-67520 / 11139	-26.8 -26.8	0.59 (3)
P-Q	-59677 / 9850	-26.8 -26.8	0.47 (3)
Q-R	-59677 / 9850	-26.8 -26.8	0.47 (3)
R-S	-59677 / 9850	-26.8 -26.8	0.47 (3)
S-T	-45469 / 7512	-26.8 -26.8	0.29 (3)
T-U	-45469 / 7512	-26.8 -26.8	0.29 (3)
U-V	-36002 / 5956	-26.8 -26.8	0.20 (3)
V-W	-27077 / 4476	-26.8 -26.8	0.19 (3)
W-BA	-22364 / 3695	-26.8 -26.8	0.20 (3)
BA-AY	-22364 / 3695	-53.6 -53.6	0.20 (3)
AY-X	-19387 / 3189	-53.6 -53.6	0.19 (3)
A-AV	-2311 / 13922	-13.8 -13.8	0.25 (2)
AV-BB	-2311 / 13922	-13.8 -13.8	0.25 (2)
BB-BC	-2311 / 13922	-13.8 -13.8	0.25 (2)
BC-AU	-2311 / 13922	-253.6 -253.6	0.25 (2)
AU-AT	-3430 / 20775	-253.6 -253.6	0.33 (2)
AT-AS	-4252 / 25870	-253.6 -253.6	0.34 (2)
AS-AR	-7485 / 45490	-253.6 -253.6	0.59 (2)
AR-AQ	-7485 / 45490	-253.6 -253.6	0.59 (2)
AQ-AP	-7485 / 45490	-253.6 -253.6	0.59 (2)
AP-AO	-9814 / 59640	-253.6 -253.6	0.76 (2)
AO-AN	-9814 / 59640	-253.6 -253.6	0.77 (2)
AN-AM	-9814 / 59640	-253.6 -253.6	0.77 (2)
AM-AL	-11106 / 67482	-253.6 -253.6	0.86 (2)
AL-AK	-11106 / 67482	-253.6 -253.6	0.86 (2)
AK-AJ	-11358 / 69035	-253.6 -253.6	0.88 (3)
AJ-AI	-11358 / 69035	-253.6 -253.6	0.88 (3)
AI-AH	-11358 / 69035	-253.6 -253.6	0.88 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 12.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

GIRDER TYPE: C AtlanticHip
 LEFT SUB-GIRDER SETBACK = 1-11-4
 RIGHT SUB-GIRDER SETBACK = 1-11-4
 END SETBACK = 7-11-4
 END WALL WIDTH = 7-4
 END JACK TYPE: JACK
 APPLIED TO FRONT SIDE

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

-SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 -SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.60")
 CALCULATED VERT. DEFL.(LL)= L/409 (2.29")
 ALLOWABLE DEFL.(TL)= L/240 (3.90")
 CALCULATED VERT. DEFL.(TL)= L/310 (3.02")

CSI: TC=0.61 (L-M:2), BC=0.88 (AI-AK:3),
 WB=0.93 (E-AS:3), SSI=0.29 (X-AY:3)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
MT16	473 276	2341 1245

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (AA) (INPUT = 0.90)
 JSI METAL= 0.87 (AJ) (INPUT = 1.00)

JOB NAME G1507147	TRUSS NAME A13A	QUANTITY 1	PLY 4	JOB DESC. W/O# 090	DRWG NO. P4324104
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 12:21:25 2015 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
B	TMWW-t	MT20	6.0	8.0		
C	TTWW+m	MT20	7.0	12.0	6.00	3.25
D, F, I, L, N, Q, T						
D	TMW+w	MT20	3.0	6.0		
E, H, S						
E	TMWWW-t	MT20	7.0	12.0		
G, K, O, R						
G	TS-t	MT20	7.0	12.0		
J, M, P						
J	TMWWW-t	MT20	7.0	8.0		
U	TMWW-t	MT20	6.0	8.0	3.00	3.25
V	TTWW+m	MT20	7.0	12.0	6.00	3.25
W	TMWW-t	MT20	6.0	8.0		
X	TMBMW1-m	MT20	8.0	12.0	3.00	6.00
Y, Z, AT, AU						
Y	BMWW-t	MT20	6.0	8.0		
AA	BMWW-t	MT20	6.0	8.0	3.00	2.75
AB	BS-t	MT20	10.0	16.0		7.50
AC, AE, AP						
AC	BMWWW-t	MT20	7.0	12.0		
AD, AG, AI, AL, AO, AQ						
AD	BMW+w	MT20	3.0	6.0		
AF, AJ, AN, AR						
AF	BS-t	MII16	9.0	20.0		
AH	BMWWW-t	MT20	7.0	8.0		
AK	BMWWW-t	MT20	7.0	8.0		
AM	BMWWW-t	MT20	7.0	8.0	3.50	2.50
AS	BMWWW-t	MT20	7.0	12.0	3.50	4.50

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 435.3 lbs FACTORED DOWN AND 66.4 lbs FACTORED UP AT 76-0-12, AND 435.3 lbs FACTORED DOWN AND 66.4 lbs FACTORED UP AT 1-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

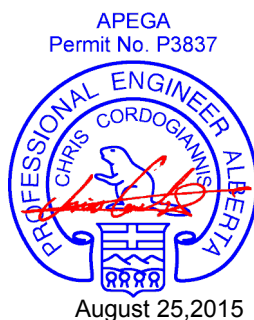
TOTAL LOAD CASES: (18)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
AH-AG	-10569 / 64351	-253.6	-253.6	0.82 (3)		6.25	
AG-AF	-10569 / 64351	-253.6	-253.6	0.82 (3)		6.25	
AF-AE	-10569 / 64351	-253.6	-253.6	0.82 (3)		6.25	
AE-AD	-8747 / 53359	-253.6	-253.6	0.69 (3)		6.25	
AD-AC	-8747 / 53359	-253.6	-253.6	0.69 (3)		6.25	
AC-AB	-5889 / 36001	-253.6	-253.6	0.48 (3)		6.25	
AB-AA	-5889 / 36001	-253.6	-253.6	0.48 (3)		6.25	
AA-Z	-4217 / 25859	-253.6	-253.6	0.34 (3)		6.25	
Z-Y	-3398 / 20779	-253.6	-253.6	0.33 (3)		6.25	
Y-BD	-2280 / 13923	-253.6	-253.6	0.25 (3)		6.25	
BD-BE	-2280 / 13923	-13.8	-13.8	0.25 (3)		6.25	
BE-AX	-2280 / 13923	-13.8	-13.8	0.25 (3)		6.25	
AX-X	-2280 / 13923	-13.8	-13.8	0.25 (3)		6.25	

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
BB	1-11-4	-392	-435	66	FRONT	VERT	TOTAL
BE	76-0-12	-392	-435	66	FRONT	VERT	TOTAL

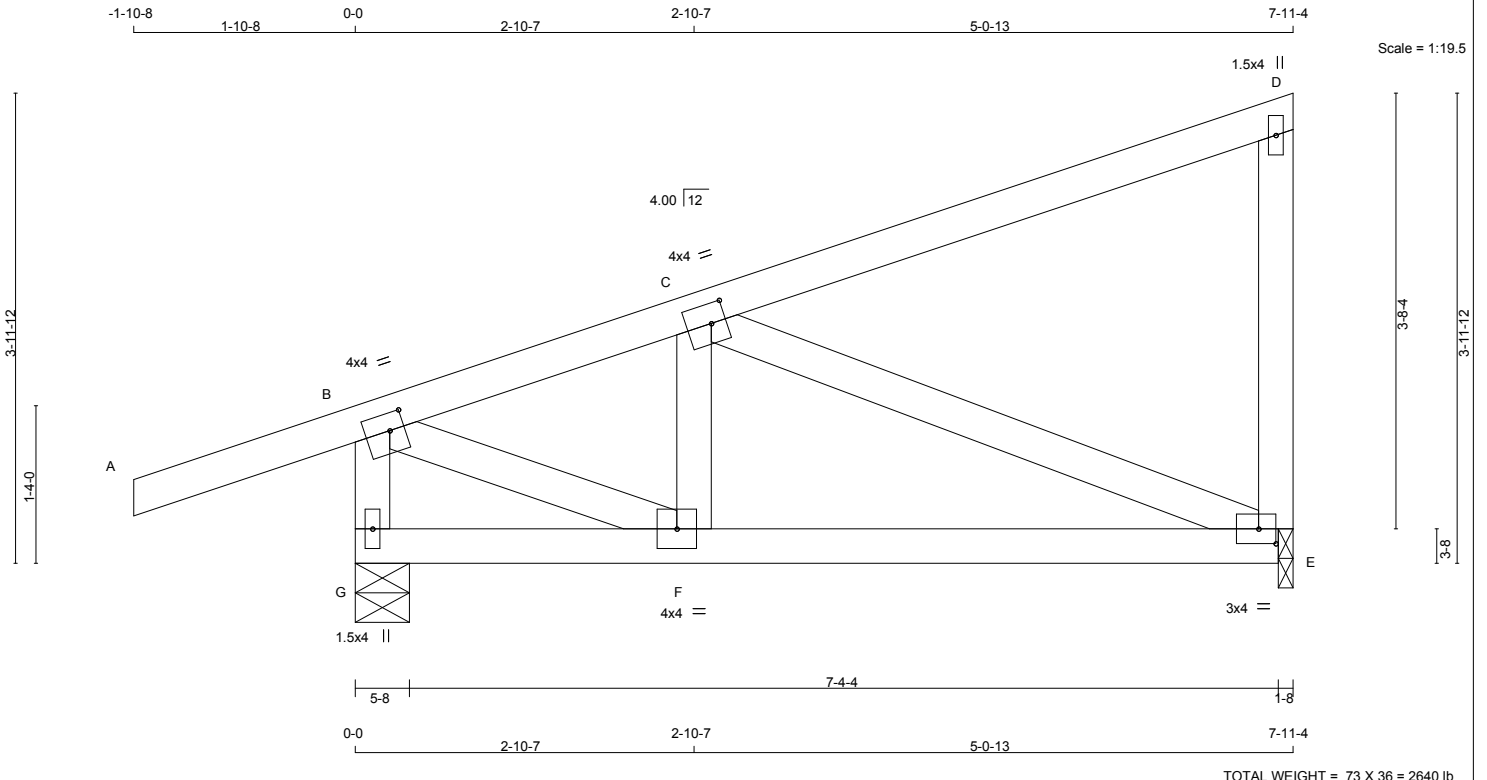
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



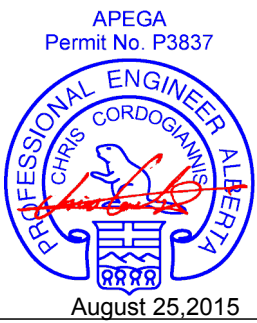
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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpic.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.

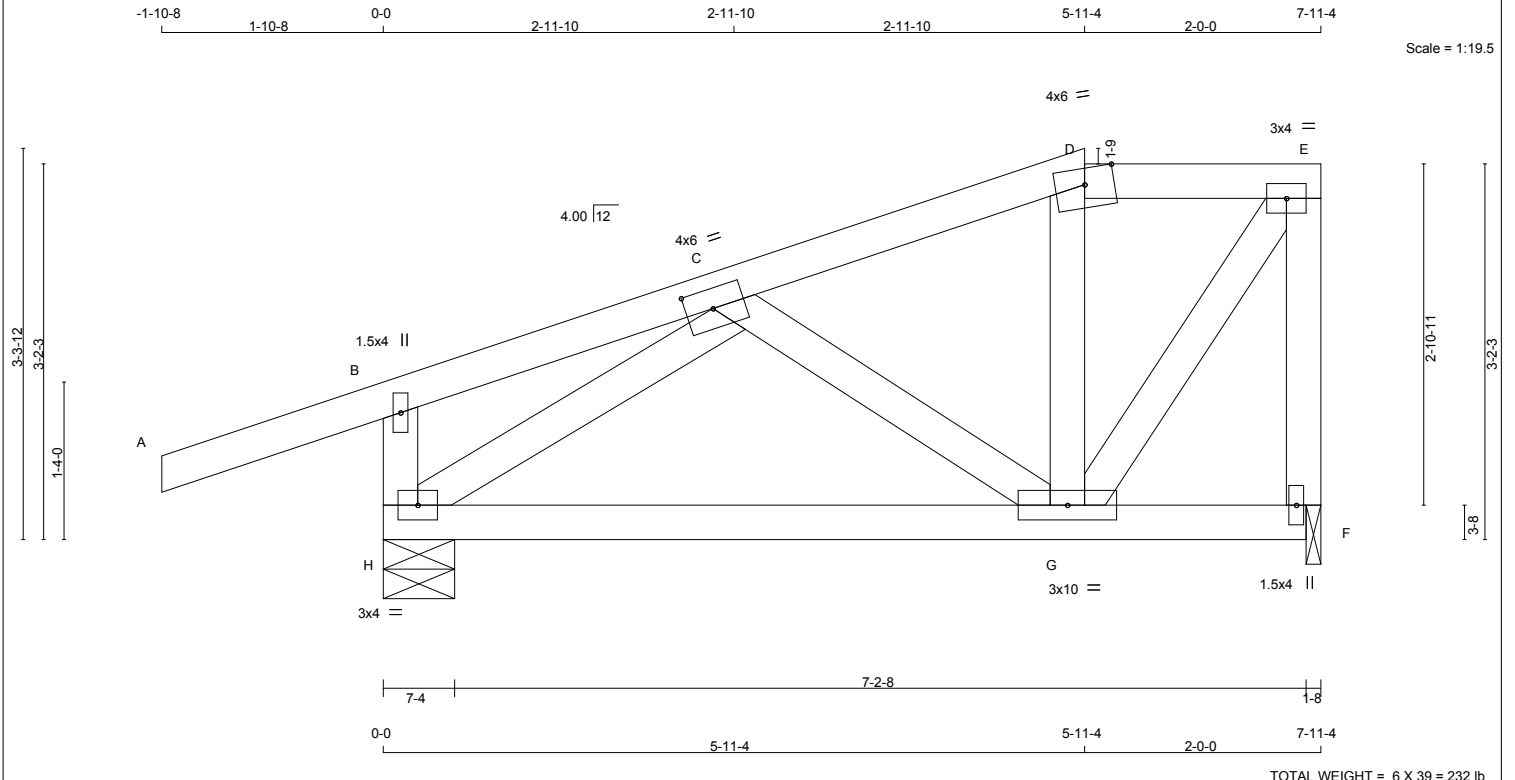




TOTAL WEIGHT = 73 X 36 = 2640 lb

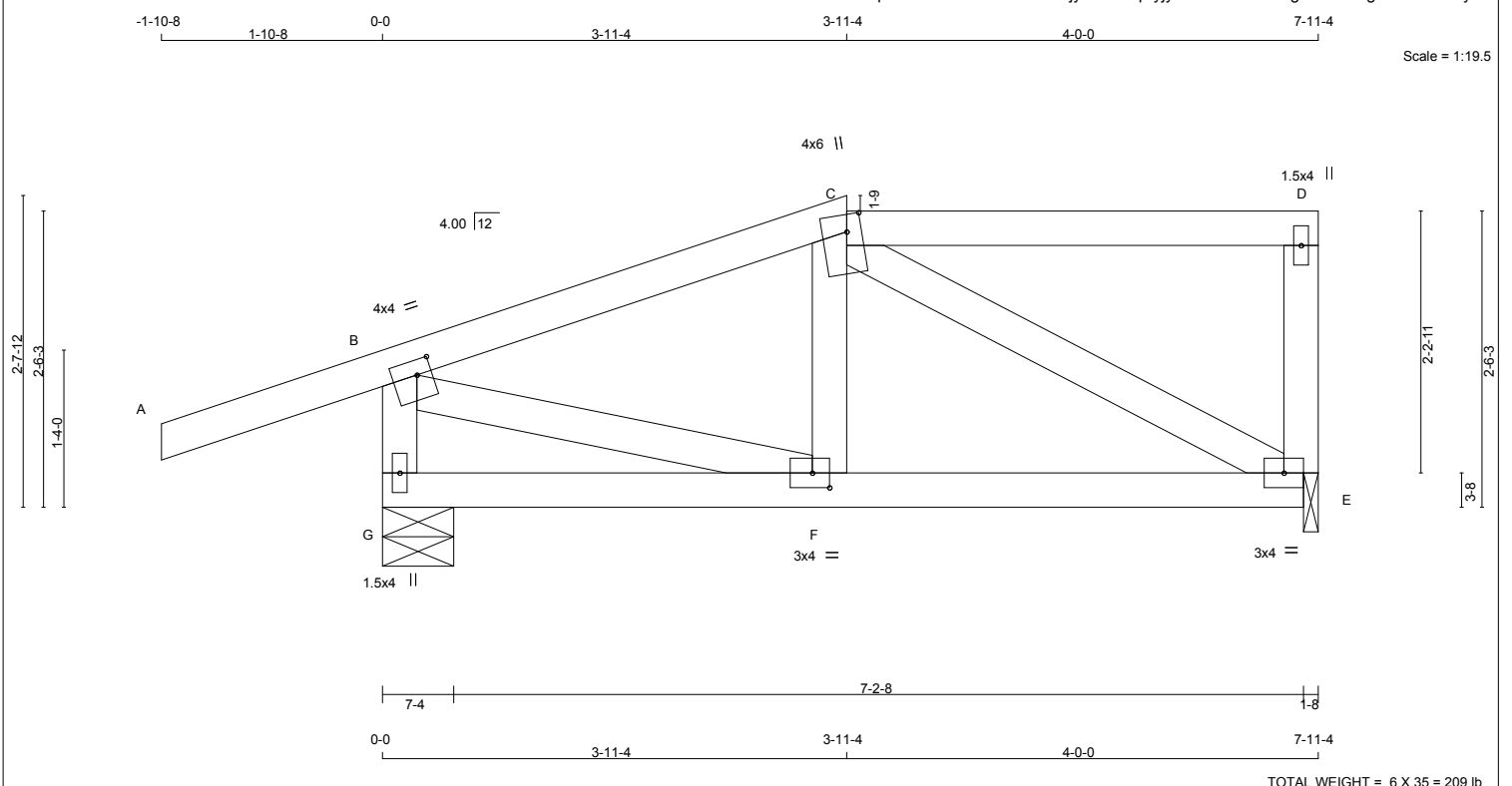
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY				DESIGN CRITERIA			
N. L. G. A. RULES				BUILDING DESIGNER				SPECIFIED LOADS:			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				TOP CH. LL = 31.5 PSF			
A - D	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	DL = 5.0 PSF			
E - D	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	BOT CH. LL = 10.0 PSF			
G - B	2x4	DRY	No.2	JT VERT	SNOW DOWN	LIVE HORZ	PERM.LIVE HORZ	DL = 7.0 PSF			
G - E	2x4	DRY	No.2	E 534	0	593	0	TOTAL LOAD = 53.5 PSF			
ALL WEBS	2x4	DRY	No.2	G 740	0	827	215	SPACING = 24.0 IN. C/C			
DRY: SEASONED LUMBER.								THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010			
PLATES (table is in inches)				UNFACTORED REACTIONS				THIS DESIGN COMPLIES WITH:			
JT	TYPE	PLATES	W L E N Y X	1ST LCASE	MAX./MIN. COMPONENT REACTIONS			- PART 4 OF OBC 2012, BCBC 2012, ABC 2014			
B	TMVW-t	MT20	4.0 4.0 1.75 1.50	JT	COMBINED	SNOW	LIVE	- CSA 086-09			
C	TMVW-t	MT20	4.0 4.0 2.00 1.50	E	425	289 / 0	79 / 0	- TPIC 2011			
D	TMV+p	MT20	1.5 4.0	G	565	429 / 0	79 / 0	DESIGN ASSUMPTIONS			
E	BMVW1-t	MT20	3.0 4.0 1.50 1.75	HORIZONTAL REACTIONS				- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED			
F	BMVW-t	MT20	4.0 4.0	G	---			- SLOPE REDUCTION FACTOR USED			
G	BMV1+p	MT20	1.5 4.0	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G				(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				BRACING				ALLOWABLE DEFL.(LL)= L/360 (0.26")			
				MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.				CALCULATED VERT. DEFL.(LL) = L/999 (0.03")			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.				ALLOWABLE DEFL.(TL)= L/240 (0.40")			
				LOADING				CALCULATED VERT. DEFL.(TL) = L/999 (0.05")			
				TOTAL LOAD CASES: (18)				CSI: TC=0.39 (B-C:2), BC=0.22 (E-F:5), WB=0.25 (C-E:2), SSI=0.27 (C-D:2)			
				C H O R D S				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				MEMB. MAX. FACTORED FORCE (LBS)				COMP=1.10 SHEAR=1.10 TENS= 1.10			
				FACTORED VERT. LOAD LC1 (PLF)				SNOW LOAD IMPORTANCE FACTOR = 1.00			
				MAX. UNBRACED LENGTH				WIND LOAD IMPORTANCE FACTOR = 1.00			
				FR-TO FROM TO				LIVE LOAD IMPORTANCE FACTOR = 1.00			
				A-B 0 / 37				COMPANION LIVE LOAD FACTOR = 0.50			
				B-C -718 / 151							
				C-D -62 / 34							
				E-D -254 / 83							
				G-B -804 / 173							
				G-F -197 / 0							
				F-E -181 / 711							
				TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)							
				WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.							
				NAIL VALUES							
				PLATE GRIP(DRY) SHEAR (PSI)							
				(PLI) (PLI) SECTION							
				MAX MIN MAX MIN MAX MIN							
				MT20 618 354 1667 822 2284 1656							
				PLATE PLACEMENT TOL. = 0.250 inches							
				PLATE ROTATION TOL. = 5.0 Deg.							
				JSI GRIP= 0.89 (E) (INPUT = 0.90)							
				JSI METAL= 0.32 (B) (INPUT = 1.00)							





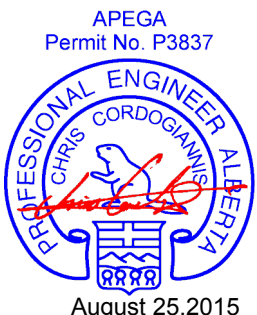
TOTAL WEIGHT = 6 X 39 = 232 lb

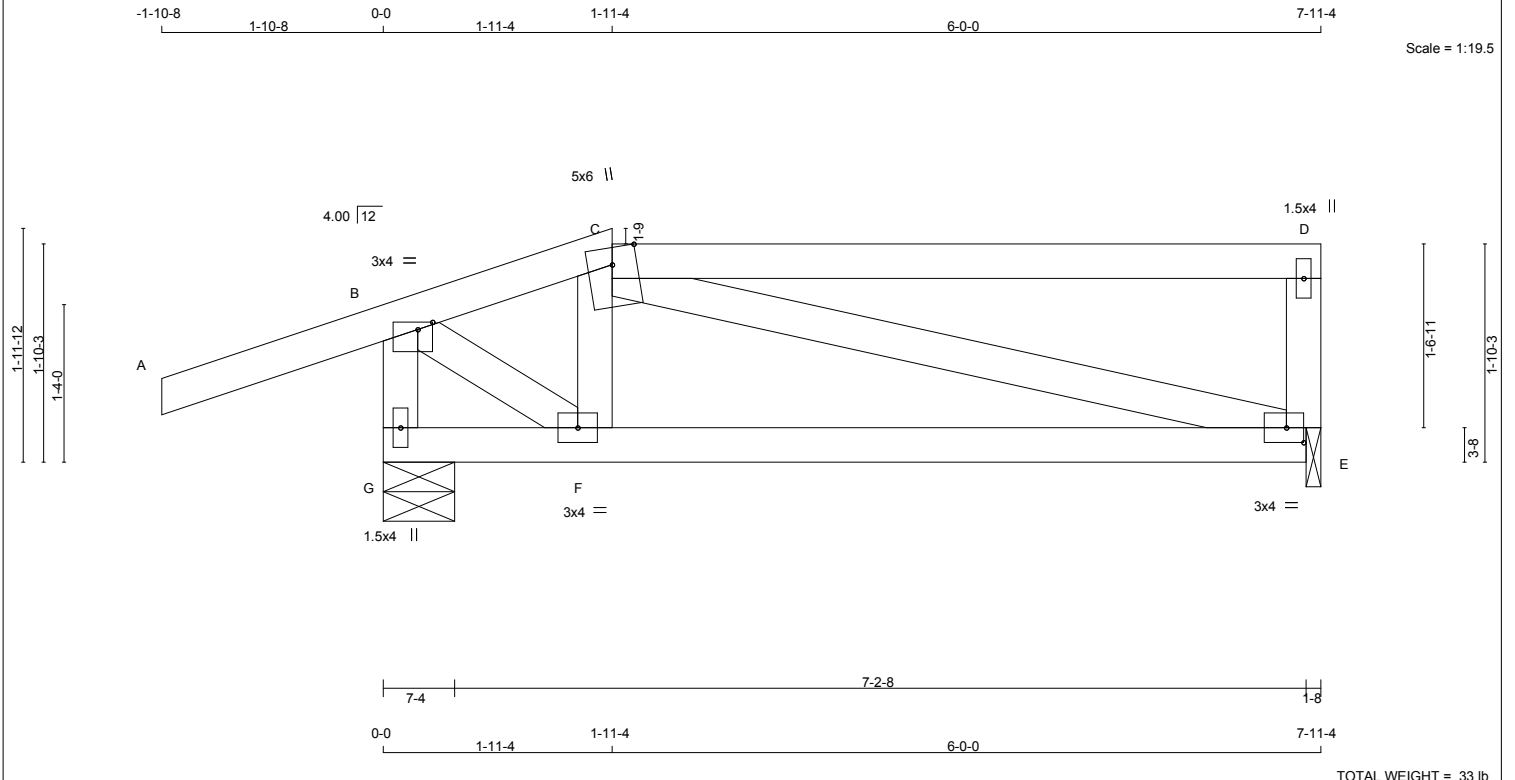
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY				DESIGN CRITERIA				
N. L. G. A. RULES				BUILDING DESIGNER								
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:				
A - D	2x4	DRY	No.2	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH. LL =	31.5	PSF		
D - E	2x4	DRY	No.2	GROSS REACTION	GROSS REACTION	BRG	BRG	DL =	5.0	PSF		
F - E	2x4	DRY	No.2	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
H - B	2x4	DRY	No.2	F	546	0	546	0	-134	1-8	1-8	
H - F	2x4	DRY	No.2	H	742	0	799	154	-169	7-4	1-8	
ALL WEBS	2x4	DRY	No.2	PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT				BOT CH. LL =				
DRY: SEASONED LUMBER.				PROVIDE ANCHORAGE AT BEARING JOINT H FOR 169 LBS FACTORED UPLIFT				10.0				
				PROVIDE FOR 154 LBS FACTORED HORIZONTAL REACTION AT JOINT H				DL =				
								7.0				
								TOTAL LOAD =				
								53.5				
								PSF				
								SPACING =				
								24.0				
								IN. C/C				
								LOADING IN FLAT SECTION BASED ON A				
								SLOPE OF 2.00/12				
								THIS TRUSS IS DESIGNED FOR COMMERCIAL				
								OR INDUSTRIAL BUILDING REQUIREMENTS				
								OF PART 4, NBCC 2010				
								THIS DESIGN COMPLIES WITH:				
								- PART 4 OF OBC 2012, BCBC 2012, ABC 2014				
								- CSA 086-09				
								- TPIC 2011				
								DESIGN ASSUMPTIONS				
								- SLIPPERY ROOFING MATERIAL AS PER NBCC				
								4.1.6.2.(6) IS USED				
								- SLOPE REDUCTION FACTOR USED				
								(74 % OF 39.7 P.S.F. G.S.L. PLUS				
								2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE				
								FACTOR EQUALS				
								31.5 P.S.F. SPECIFIED ROOF LIVE LOAD				
								ALLOWABLE DEFL.(LL)= L/360 (0.26")				
								CALCULATED VERT. DEFL.(LL) = L/999 (0.04")				
								ALLOWABLE DEFL.(TL)= L/240 (0.40")				
								CALCULATED VERT. DEFL.(TL) = L/999 (0.07")				
								CSI: TC=0.31 (A-B:2), BC=0.25 (G-H:5),				
								WB=0.10 (C-H:2), SSI=0.17 (A-B:2)				
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10				
								COMP=1.10 SHEAR=1.10 TENS=1.10				
								SNOW LOAD IMPORTANCE FACTOR = 1.00				
								WIND LOAD IMPORTANCE FACTOR = 1.00				
								LIVE LOAD IMPORTANCE FACTOR = 1.00				
								COMPANION LIVE LOAD FACTOR = 0.50				
								AUTOSOLVE RIGHT HEEL ONLY				
								TRUSS PLATE MANUFACTURER IS NOT				
								RESPONSIBLE FOR QUALITY CONTROL IN				
								THE TRUSS MANUFACTURING PLANT .				
								NAIL VALUES				
								PLATE GRIP(DRY) SHEAR SECTION				
								(PSI) (PLI) (PLI)				
								MAX MIN MAX MIN MAX MIN				
								MT20 618 354 1667 822 2284 1656				
								PLATE PLACEMENT TOL. = 0.250 inches				
								PLATE ROTATION TOL. = 5.0 Deg.				
								JSI GRIP= 0.89 (E) (INPUT = 0.90)				
								JSI METAL= 0.20 (H) (INPUT = 1.00)				



TOTAL WEIGHT = 6 X 35 = 209 lb

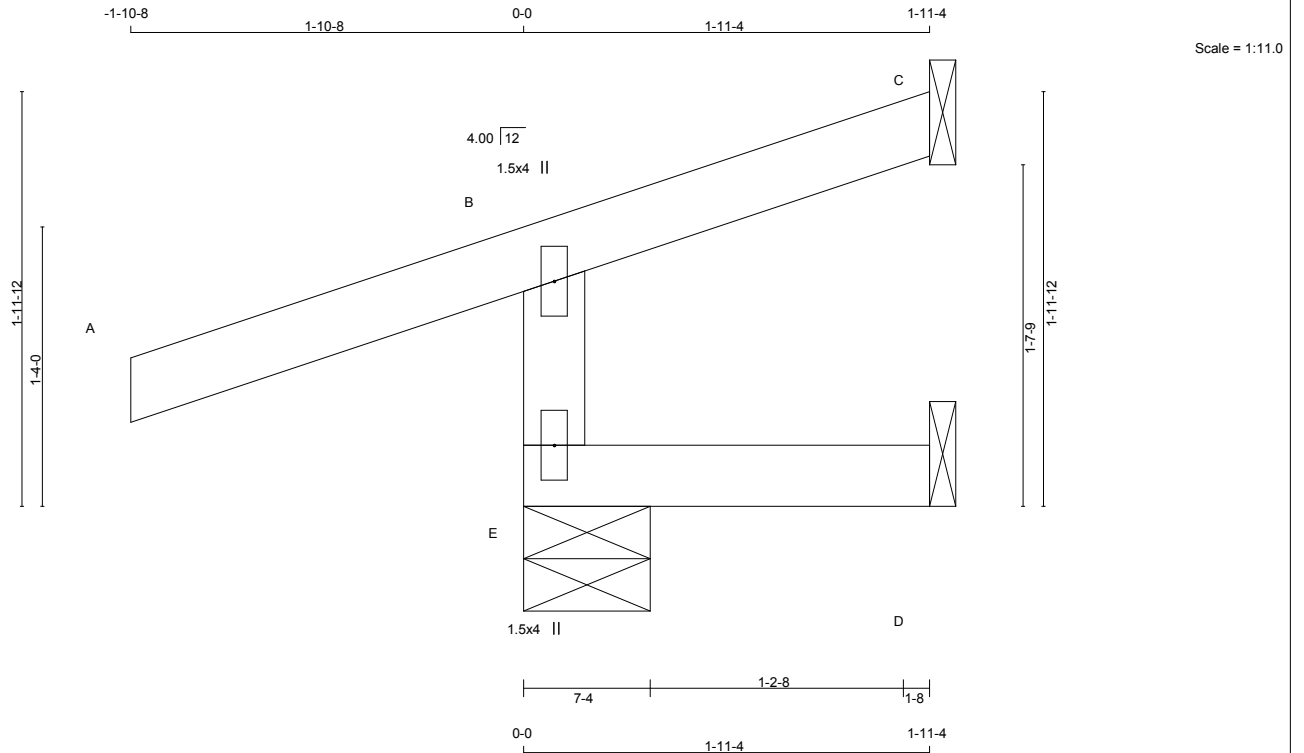
LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA						
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:						
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL =	31.5	PSF			
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	DL =	5.0	PSF				
C - D	2x4	DRY	No.2	SPF	VERT	HORZ	UPLIFT	BOT CH.	LL =	10.0	PSF			
E - D	2x4	DRY	No.2	SPF	555	0	555	DL =	7.0	PSF				
G - B	2x4	DRY	No.2	SPF	747	0	747	TOTAL LOAD =	53.5	PSF				
G - E	2x4	DRY	No.2	SPF	122	-174	7-4							
ALL WEBS 2x4 DRY				PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT				SPACING = 24.0 IN. C/C						
DRY: SEASONED LUMBER.				PROVIDE ANCHORAGE AT BEARING JOINT G FOR 174 LBS FACTORED UPLIFT				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12						
				PROVIDE FOR 122 LBS FACTORED HORIZONTAL REACTION AT JOINT G				THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010						
PLATES (table is in inches)				UNFACTORED REACTIONS				THIS DESIGN COMPLIES WITH:						
JT TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX/MIN COMPONENT REACTIONS	- PART 4 OF OBC 2012, BCBC 2012, ABC 2014						
B	TMVW-t	MT20	4.0	4.0	1.50	1.50	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	TTWW+m	MT20	4.0	6.0	1.75	1.50	E	439	264 / 0	79 / 0	0 / 0	0 / -153	95 / 0	0 / 0
D	TMV+p	MT20	1.5	4.0			G	570	376 / 0	79 / 0	0 / 0	0 / -198	114 / 0	0 / 0
E	BMVW1-t	MT20	3.0	4.0			HORIZONTAL REACTIONS							
F	BMVW-t	MT20	3.0	4.0	1.50	1.75	G	---	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0	0 / 0
G	BMV1+p	MT20	1.5	4.0			BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G							
				BRACING				DESIGN ASSUMPTIONS						
				MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.				- SLIPPERY ROOFING MATERIAL AS PER NBCC						
				MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.				4.1.6.2.(6) IS USED						
				LOADING				- SLOPE REDUCTION FACTOR USED						
				TOTAL LOAD CASES: (18)				(74 % OF 39.7 P.S.F. G.S.L. PLUS						
				C H O R D S				2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE						
				MEMB.				FACTOR EQUALS						
				MAX. FACTORED				31.5 P.S.F. SPECIFIED ROOF LIVE LOAD						
				FORCE				ALLOWABLE DEFL.(LL)= L/360 (0.26")						
				(LBS)				CALCULATED VERT. DEFL.(LL) = L/999 (0.01")						
				FACTORED				ALLOWABLE DEFL.(TL)= L/240 (0.40")						
				VERT. LOAD LC1				CALCULATED VERT. DEFL.(TL) = L/999 (0.02")						
				MAX				CSI: TC=0.33 (B-C:2), BC=0.17 (E-F:4),						
				CSI (LC)				WB=0.13 (C-E:1), SSI=0.18 (B-C:2)						
				UNBRAC				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10						
				LENGTH				COMP=1.10 SHEAR=1.10 TENS= 1.10						
				FR-TO				SNOW LOAD IMPORTANCE FACTOR = 1.00						
				FR-TO				WIND LOAD IMPORTANCE FACTOR = 1.00						
				A-B				LIVE LOAD IMPORTANCE FACTOR = 1.00						
				B-C				COMPANION LIVE LOAD FACTOR = 0.50						
				C-D				AUTOSOLVE RIGHT HEEL ONLY						
				E-D				TRUSS PLATE MANUFACTURER IS NOT						
				G-B				RESPONSIBLE FOR QUALITY CONTROL IN						
				G-F				THE TRUSS MANUFACTURING PLANT .						
				F-E				NAIL VALUES						
								PLATE GRIP(DRY) SHEAR SECTION						
								(PSI) (PLI) (PLI)						
								MAX MIN MAX MIN MAX MIN						
								MT20 618 354 1667 822 2284 1656						
								PLATE PLACEMENT TOL. = 0.250 inches						
								PLATE ROTATION TOL. = 5.0 Deg.						
								JSI GRIP= 0.76 (E) (INPUT = 0.90)						
								JSI METAL= 0.24 (B) (INPUT = 1.00)						





TOTAL WEIGHT = 33 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA							
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:							
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL =	31.5	PSF				
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	DL =	5.0	PSF					
C - D	2x4	DRY	No.2	SPF	JT VERT	535	0	535	0	-117	1-8				
E - D	2x4	DRY	No.2	SPF	JT HORZ	0	535	0	-186	7-4	1-8				
G - B	2x4	DRY	No.2	SPF	E	781	0	781	89	-186	7-4				
G - E	2x4	DRY	No.2	SPF	G	781	0	781	89	-186	7-4				
ALL WEBS	2x4	DRY	No.2	SPF	PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT				BOT CH. LL = 10.0 PSF						
DRY: SEASONED LUMBER.				PROVIDE ANCHORAGE AT BEARING JOINT G FOR 186 LBS FACTORED UPLIFT				DL = 7.0 PSF							
				PROVIDE FOR 89 LBS FACTORED HORIZONTAL REACTION AT JOINT G				TOTAL LOAD = 53.5 PSF							
PLATES (table is in inches)				UNFACTORED REACTIONS				SPACING = 24.0 IN. C/C							
JT TYPE	PLATES	W	LEN	Y	X	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12							
B	TMVW-p	MT20	3.0	4.0	0.75	1.50	JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
C	TTWW+m	MT20	5.0	6.0	Edge		E	425	253 / 0	79 / 0	0 / 0	0 / -144	93 / 0	0 / 0	
D	TMV+p	MT20	1.5	4.0			G	593	397 / 0	79 / 0	0 / 0	0 / -208	117 / 0	0 / 0	
E	BMVW1-t	MT20	3.0	4.0	1.50	1.75	HORIZONTAL REACTIONS				THIS DESIGN COMPLIES WITH:				
F	BMVW-t	MT20	3.0	4.0			G	---	0 / 0	0 / 0	0 / 0	64 / -6	0 / 0	0 / 0	- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
G	BMV1+p	MT20	1.5	4.0			BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G				- CSA 086-09				
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.				BRACING				LOADING				- TPIC 2011			
				MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.				TOTAL LOAD CASES: (18)				DESIGN ASSUMPTIONS			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.				C H O R D S				- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED			
				MEMB.				MAX. FACTORED				- SLOPE REDUCTION FACTOR USED			
				FORCE				FACTORED				- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
				(LBS)				VERT. LOAD LC1				(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD			
				(PLF)				MAX				ALLOWABLE DEFL.(LL)= L/360 (0.26")			
				CSI (LC)				UNBRAC				CALCULATED VERT. DEFL.(LL) = L / 999 (0.05")			
				LENGTH				FR-TO				ALLOWABLE DEFL.(TL)= L/240 (0.40")			
				FR-TO				MEMB.				CALCULATED VERT. DEFL.(TL) = L / 999 (0.09")			
				A- B				FORCE				CSI: TC=0.70 (C-D:1), BC=0.29 (E-F:17), WB=0.23 (C-E:1), SSI=0.27 (C-D:1)			
				B- C				(LBS)				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10			
				C- D				CSI (LC)				SNOW LOAD IMPORTANCE FACTOR = 1.00			
				E- D				MAX				WIND LOAD IMPORTANCE FACTOR = 1.00			
				G- B				UNBRAC				LIVE LOAD IMPORTANCE FACTOR = 1.00			
				G- F				FR-TO				COMPANION LIVE LOAD FACTOR = 0.50			
				F- E				MEMB.				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
				-				FORCE				NAIL VALUES			
				-48 / 456				CSI (LC)				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
								MAX				MAX MIN MAX MIN MAX MIN			
								UNBRAC				MT20 618 354 1667 822 2284 1656			
								FR-TO				PLATE PLACEMENT TOL. = 0.250 inches			
								MEMB.				PLATE ROTATION TOL. = 5.0 Deg.			
								FORCE				JSI GRIP= 0.88 (B) (INPUT = 0.90)			
								CSI (LC)				JSI METAL= 0.20 (G) (INPUT = 1.00)			
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
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								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
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								FORCE							
								CSI (LC)							
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								CSI (LC)							
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								UNBRAC							
								FR-TO							
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								CSI (LC)							
								MAX							
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								MEMB.							
								FORCE							
								CSI (LC)							
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								UNBRAC							
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								CSI (LC)							
								MAX							
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								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
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								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
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								FR-TO							
								MEMB.							
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								CSI (LC)							
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								FORCE							
								CSI (LC)							
								MAX							
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								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							
								MEMB.							
								FORCE							
								CSI (LC)							
								MAX							
								UNBRAC							
								FR-TO							



TOTAL WEIGHT = 16 X 8 = 126 lb [P][M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	1.5	4.0		
E	BMV1+p	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	438	495	90	1-8
C	26	30	-73	1-8
D	3	42	-17	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 90 LBS FACTORED HORIZONTAL REACTION AT JOINT E

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
E	314	280 / 0	19 / 0	0 / 0	0 / -98	52 / 0	0 / 0
C	19	16 / -49	1 / 0	0 / 0	4 / -29	3 / 0	0 / 0
D	15	0 / -20	18 / 0	0 / 0	5 / -9	11 / 0	0 / 0

HORIZONTAL REACTIONS

JT	MAX	MIN	WIND	DEAD	SOIL
E	0 / 0	0 / 0	64 / -6	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (19)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX FACTORED (LC)
FR-TO						
E-B	-441 / 110	0.0	0.0	0.09 (19)	7.81	
A-B	0 / 37	-107.1	-107.1	0.31 (2)	10.00	
B-C	-28 / 0	-107.1	-107.1	0.23 (2)	6.25	
E-D	0 / 0	-27.5	-27.5	0.09 (19)	10.00	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 31.5 PSF

DL = 5.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL)= L/240 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.31 (A-B:2), BC=0.09 (D-E:19), WB=0.00 (n/a:0), SSI=0.17 (A-B:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

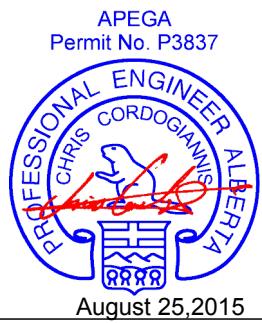
PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MAX	MIN	MAX
618	354	1667
822	2284	1656

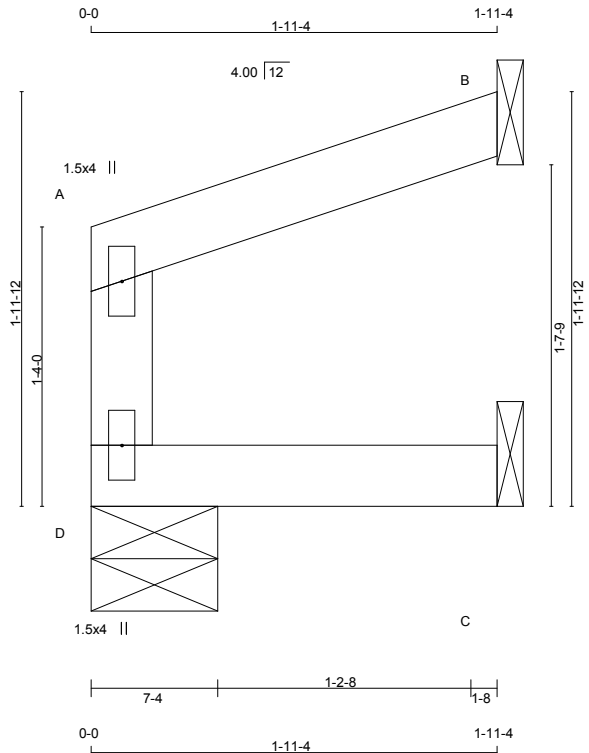
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)

JSI METAL= 0.11 (E) (INPUT = 1.00)





TOTAL WEIGHT = 3 X 6 = 17 lb [P][M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4 DRY	No.2	SPF
A - B	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	1.5	4.0		
D	BMV1+p	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	130	0	145	65
B	98	0	111	0
C	32	0	46	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C
 PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 65 LBS FACTORED HORIZONTAL REACTION AT JOINT D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
D	104	71 / 0	19 / 0	0 / 0	0 / -23	23 / 0	0 / 0
B	68	66 / 0	1 / 0	0 / 0	0 / -46	10 / 0	0 / 0
C	36	5 / 0	18 / 0	0 / 0	1 / -13	13 / 0	0 / 0

HORIZONTAL REACTIONS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	---	0 / 0	0 / 0	0 / 0	46 / -25	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED	FACTORED	MAX	MAX	MEMB.	FORCE	MAX	
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	UNBRAC (LC)	FR-TO	(LBS)	CSI (LC)	
D-A	-125 / 33	0.0	0.0	0.05 (13)	7.81			
A-B	-20 / 0	-107.1	-107.1	0.06 (2)	6.25			
D-C	0 / 0	-27.5	-27.5	0.04 (13)	10.00			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/240 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.06 (A-B:2) , BC=0.04 (C-D:13) ,
 WB=0.00 (n/a:0) , SSI=0.09 (A-B:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

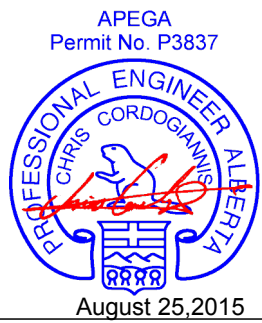
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

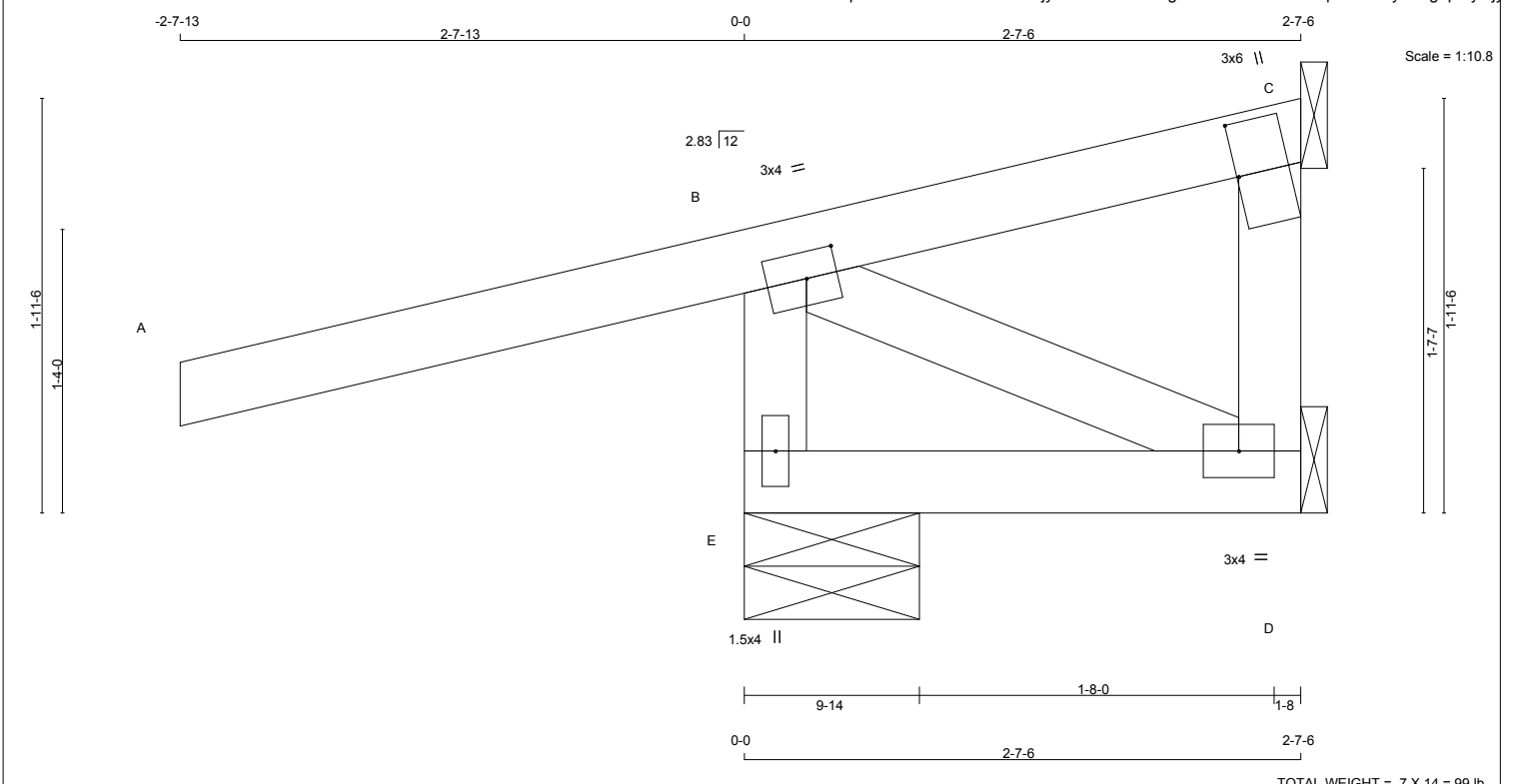
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
 JSI METAL= 0.06 (D) (INPUT = 1.00)



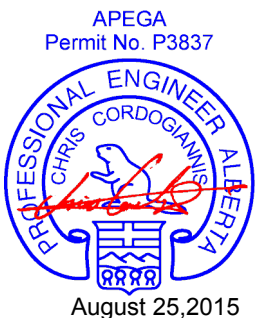
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-08 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.

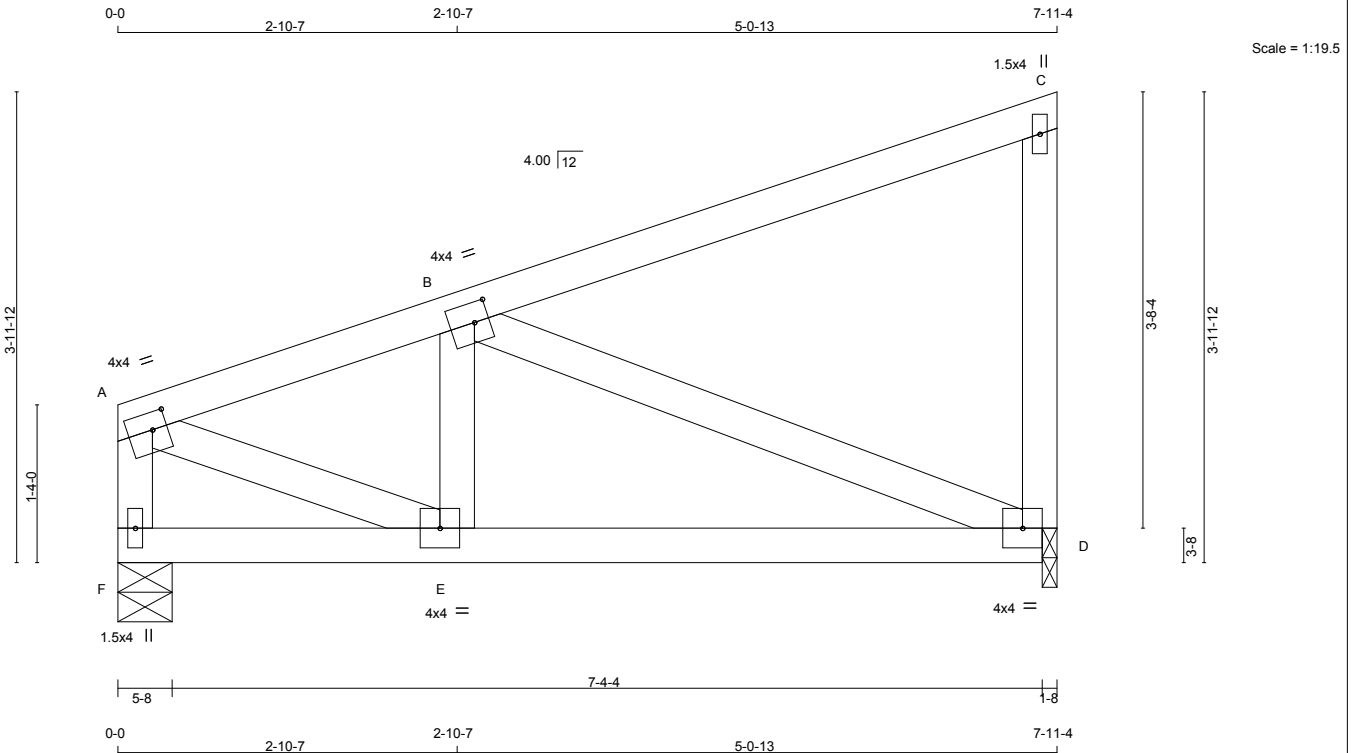




TOTAL WEIGHT = 7 X 14 = 99 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA						
N. L. G. A. RULES				BEARINGS				SPECIFIED LOADS:						
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	TOP CH.	LL =	33.9	PSF			
A - C	2x4	DRY	No.2	JT	GROSS REACTION	GROSS REACTION	BRG	DL =	5.0	PSF				
D - C	2x4	DRY	No.2	C	VERT	DOWN	IN-SX	BOT CH.	LL =	10.0	PSF			
E - B	2x4	DRY	No.2	D	HORZ	HORZ	UPLIFT	DL =	7.0	PSF				
E - D	2x4	DRY	No.2	E	UP	UP	IN-SX	TOTAL LOAD =	55.8	PSF				
ALL WEBS	2x4	DRY	No.2	SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D				SPACING = 24.0 IN. C/C						
DRY: SEASONED LUMBER.	PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT				PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT				THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010					
GABLE STUDS SPACED AT 2-0-0 OC.	PROVIDE ANCHORAGE AT BEARING JOINT E FOR 162 LBS FACTORED UPLIFT				PROVIDE FOR 95 LBS FACTORED HORIZONTAL REACTION AT JOINT E				THIS DESIGN COMPLIES WITH:					
	UNFACTORED REACTIONS				DESIGN ASSUMPTIONS				- PART 4 OF OBC 2012, BCBC 2012, ABC 2014					
	JT	TYPE	PLATES	W	LEN	Y	X	- CSA 086-09						
	B	TMVW-t	MT20	3.0	4.0	1.50	1.75	- TPIC 2011						
	C	TMV1+t	MT20	3.0	6.0	3.00		SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED						
	D	BMVW1-t	MT20	3.0	4.0			- SLOPE REDUCTION FACTOR USED						
	E	BMV1+p	MT20	1.5	4.0			- OVERHANG NOT TO BE ALTERED OR CUT OFF.						
	HORIZONTAL REACTIONS				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E				(80% OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD					
	E			0/0	0/0	0/0	68/-8	0/0	0/0	ALLOWABLE DEFL.(LL)= L/360 (0.19")				
	BRACING				MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.				CALCULATED VERT. DEFL.(LL) = L/999 (0.00")					
	MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.				LOADING				ALLOWABLE DEFL.(TL)= L/240 (0.19")					
	TOTAL LOAD CASES: (19)				TOTAL LOAD CASES: (19)				CALCULATED VERT. DEFL.(TL) = L/999 (0.00")					
	C H O R D S				W E B S				CSI: TC=0.56 (A-B:1), BC=0.06 (D-E:6), WB=0.01 (B-D:12), SSI=0.23 (B-C:1)					
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10				
	FR-TO	FROM	TO	CSI (LC)	UNBRAC	LENGTH	FR-TO	B-D	0/70	0.01 (12)	SNOW LOAD IMPORTANCE FACTOR = 1.00			
	A-B	0/35	-114.0	-114.0	0.56 (1)	10.00					WIND LOAD IMPORTANCE FACTOR = 1.00			
	B-C	-45/22	-114.0	-114.0	0.56 (1)	6.25					LIVE LOAD IMPORTANCE FACTOR = 1.00			
	D-C	0/0	0.0	0.0	0.01 (11)	10.00					COMPANION LIVE LOAD FACTOR = 0.50			
	E-B	-612/178	0.0	0.0	0.06 (1)	7.81					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
	E-D	-78/4	-27.5	-27.5	0.06 (6)	6.25					NAIL VALUES			
	TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)				CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)					
	WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.				MAX MIN MAX MIN MAX MIN				MT20 618 354 1667 822 2284 1656					
									PLATE PLACEMENT TOL. = 0.250 inches					
									PLATE ROTATION TOL. = 5.0 Deg.					
									JSI GRIP= 0.60 (B) (INPUT = 0.90)					
									JSI METAL= 0.15 (E) (INPUT = 1.00)					





TOTAL WEIGHT = 6 X 34 = 203 lb

LUMBER **DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER** **DESIGN CRITERIA**

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
D - C	2x4 DRY	No.2	SPF
F - A	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV-t	MT20	4.0	4.0	1.75	1.50
B	TMVV-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	1.5	4.0		
D	BMVV1-t	MT20	4.0	4.0		
E	BMVV-t	MT20	4.0	4.0		
F	BMV1+p	MT20	1.5	4.0		

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UPLIFT
D	562	0	626	0
F	562	0	626	224

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 166 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT
 PROVIDE FOR 224 LBS FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
		COMBINED	SNOW	LIVE
D	443	312 / 0	79 / 0	0 / 0
F	443	312 / 0	79 / 0	0 / 0

HORIZONTAL REACTIONS

JT	FACTORED	UNBRAC	WIND	DEAD	SOIL
F	0 / 0	0 / 0	160 / -8	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX UNBRAC	MEMB. FORCE (LBS)	MAX FACTORED CSI (LC)	MEMB. FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	-759 / 174	-114.0	-114.0	0.32 (2)	6.25	E-B	-174 / 156	0.02 (6)
B-C	-70 / 38	-114.0	-114.0	0.37 (2)	6.25	B-D	-807 / 262	0.27 (2)
D-C	-271 / 95	0.0	0.0	0.09 (11)	7.81	A-E	-169 / 799	0.13 (2)
F-A	-604 / 142	0.0	0.0	0.06 (2)	7.81			
F-E	-203 / 11	-27.5	-27.5	0.15 (6)	6.25			
E-D	-211 / 751	-27.5	-27.5	0.22 (5)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7 } PSF AT { 40-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 33.9 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 55.8 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED

(80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/240 (0.40")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.37 (B-C:2), BC=0.22 (D-E:5),
 WB=0.27 (B-D:2), SSI=0.29 (B-C:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

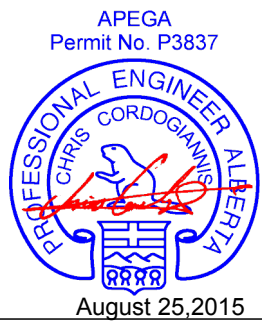
SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

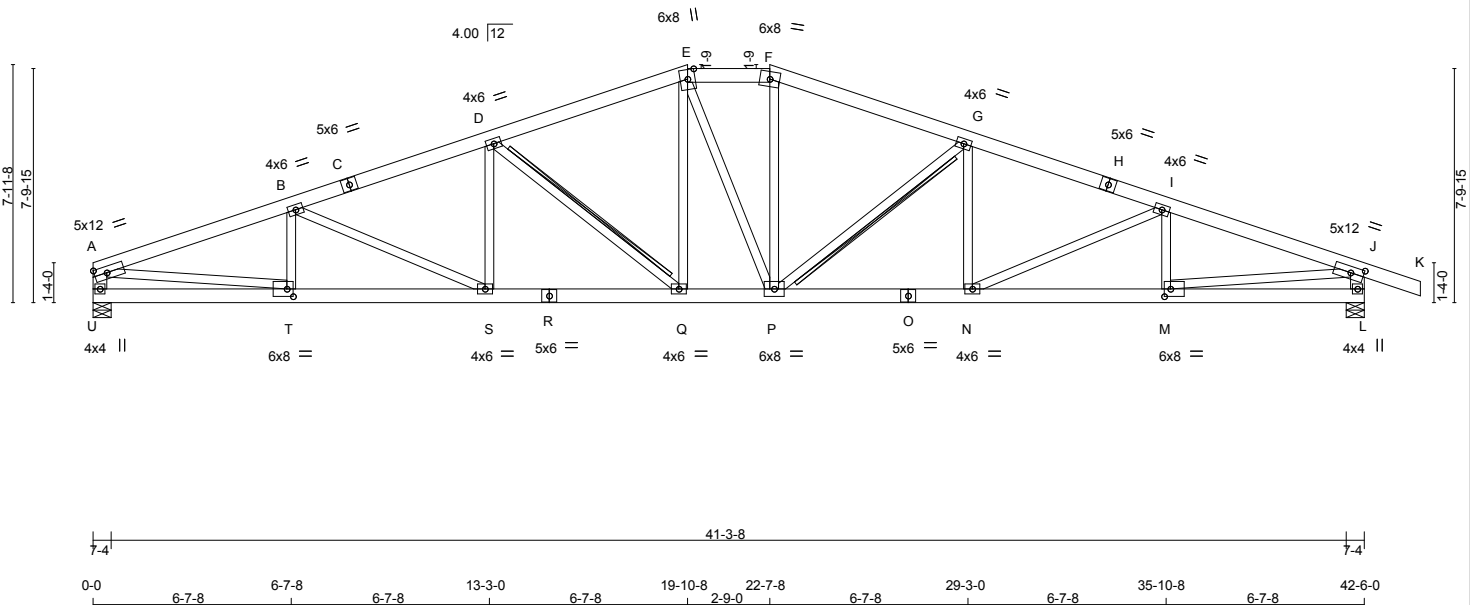
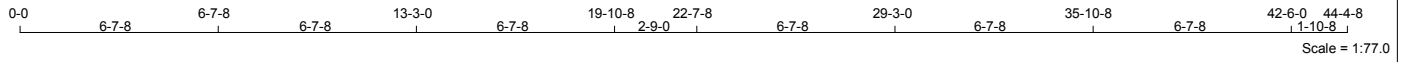
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP= 0.83 (A) (INPUT = 0.90)
 JSI METAL= 0.31 (A) (INPUT = 1.00)





TOTAL WEIGHT = 251 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY 2100F 1.8E	SPF
C - E	2x6	DRY 2100F 1.8E	SPF
E - F	2x6	DRY 2100F 1.8E	SPF
F - H	2x6	DRY 2100F 1.8E	SPF
H - K	2x6	DRY 2100F 1.8E	SPF
U - A	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
U - R	2x6	DRY 2100F 1.8E	SPF
R - O	2x6	DRY 2100F 1.8E	SPF
O - L	2x6	DRY 2100F 1.8E	SPF

ALL WEBS EXCEPT 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	5.0	12.0		Edge
B, D, G, I						
B	TMWV-t	MT20	4.0	6.0		
C	TS-t	MT20	5.0	6.0		
E	TTWV+m	MT20	6.0	8.0	Edge	
F	TTWV-m	MT20	6.0	8.0		
H	TS-t	MT20	5.0	6.0		
J	TMWV-t	MT20	5.0	12.0	2.50	5.25
L	BMV1+p	MT20	4.0	4.0		
M	BMWV-t	MT20	6.0	8.0	3.00	2.50
N, Q, S						
N	BMWV-t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWV-t	MT20	6.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWV-t	MT20	6.0	8.0	3.00	2.50
U	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2860	0	2860	-157 -568
L	3069	0	3069	0 -608

PROVIDE ANCHORAGE AT BEARING JOINT U FOR 568 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT L FOR 608 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 157 LBS FACTORED HORIZONTAL REACTION AT JOINT U

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2275	1340 / 0	425 / 0	0 / 0	0 / -734	510 / 0	0 / 0
L	2417	1463 / 0	425 / 0	0 / 0	0 / -774	529 / 0	0 / 0

HORIZONTAL REACTIONS
 U --- 0 / 0 0 / 0 84 / -112 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 4.39FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE AT D-Q, G-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (CSI (LC))	MAX UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX FACTORED (LBS)
FR-TO		FROM	TO		FR-TO	
A-B	-5552 / 1098	-107.1	-107.1	0.25 (2)	4.39	T-B -539 / 235
B-C	-5305 / 1080	-107.1	-107.1	0.24 (2)	4.49	B-S -511 / 249
C-D	-5305 / 1080	-107.1	-107.1	0.24 (2)	4.49	S-D 0 / 447
D-E	-4326 / 899	-107.1	-107.1	0.22 (2)	4.88	D-Q -1466 / 386
E-F	-4086 / 871	-107.1	-107.1	0.06 (1)	5.14	Q-E -181 / 1018
F-G	-4335 / 882	-107.1	-107.1	0.22 (3)	4.88	E-P -541 / 567
G-H	-5302 / 1029	-107.1	-107.1	0.25 (3)	4.49	P-F -138 / 896
H-I	-5302 / 1029	-107.1	-107.1	0.25 (3)	4.49	P-G -1456 / 386
I-J	-5554 / 1059	-107.1	-107.1	0.25 (3)	4.39	N-G 0 / 438
J-K	0 / 38	-107.1	-107.1	0.09 (3)	10.00	N-I -513 / 249
U-A	-2759 / 595	0.0	0.0	0.18 (1)	6.29	M-I -535 / 226
L-J	-2968 / 635	0.0	0.0	0.19 (1)	6.10	A-T -1008 / 5325
						M-J -945 / 5326
U-T	-105 / 170	-27.5	-27.5	0.08 (4)	6.25	
T-S	-1041 / 5280	-27.5	-27.5	0.31 (1)	6.25	
S-R	-903 / 5040	-27.5	-27.5	0.27 (1)	6.25	
R-Q	-903 / 5040	-27.5	-27.5	0.27 (1)	6.25	
Q-P	-607 / 4078	-27.5	-27.5	0.23 (1)	6.25	
P-O	-795 / 5037	-27.5	-27.5	0.27 (1)	6.25	
O-N	-795 / 5037	-27.5	-27.5	0.27 (1)	6.25	
N-M	-944 / 5281	-27.5	-27.5	0.31 (1)	6.25	
M-L	-7 / 15	-27.5	-27.5	0.08 (4)	10.00	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22.4-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS: CpCg. BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 31.5 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
 - SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.42")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
 ALLOWABLE DEFL.(TL)= L/240 (2.13")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.25 (I-J:3), BC=0.31 (M-N:1), WB=0.86 (J-M:1), SSI=0.28 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

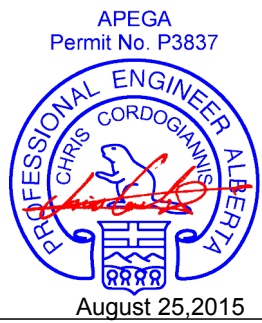
NAIL VALUES

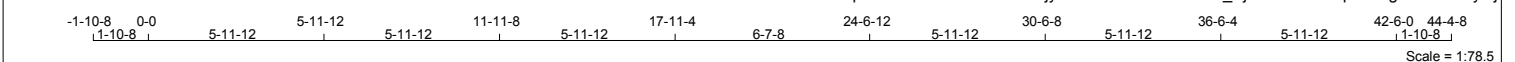
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

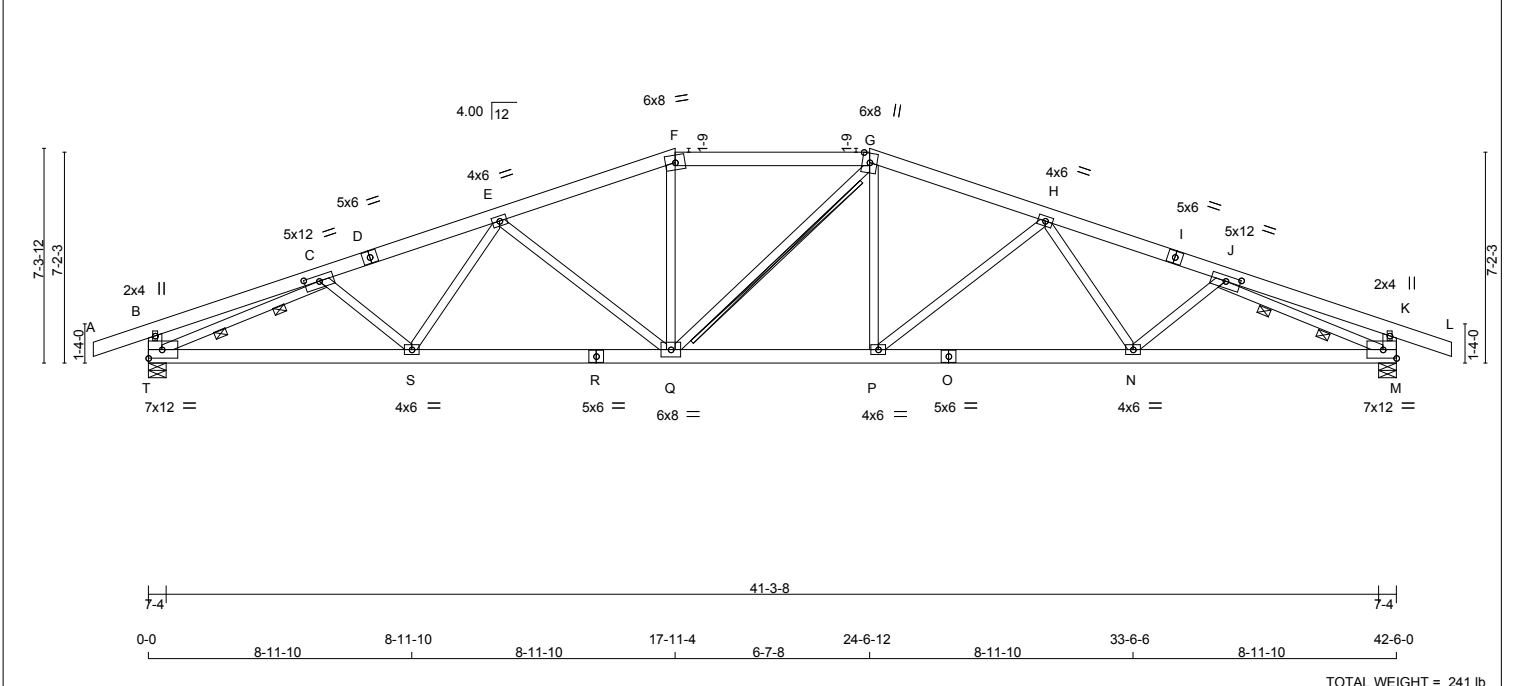
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
 JSI METAL= 0.99 (M) (INPUT = 1.00)



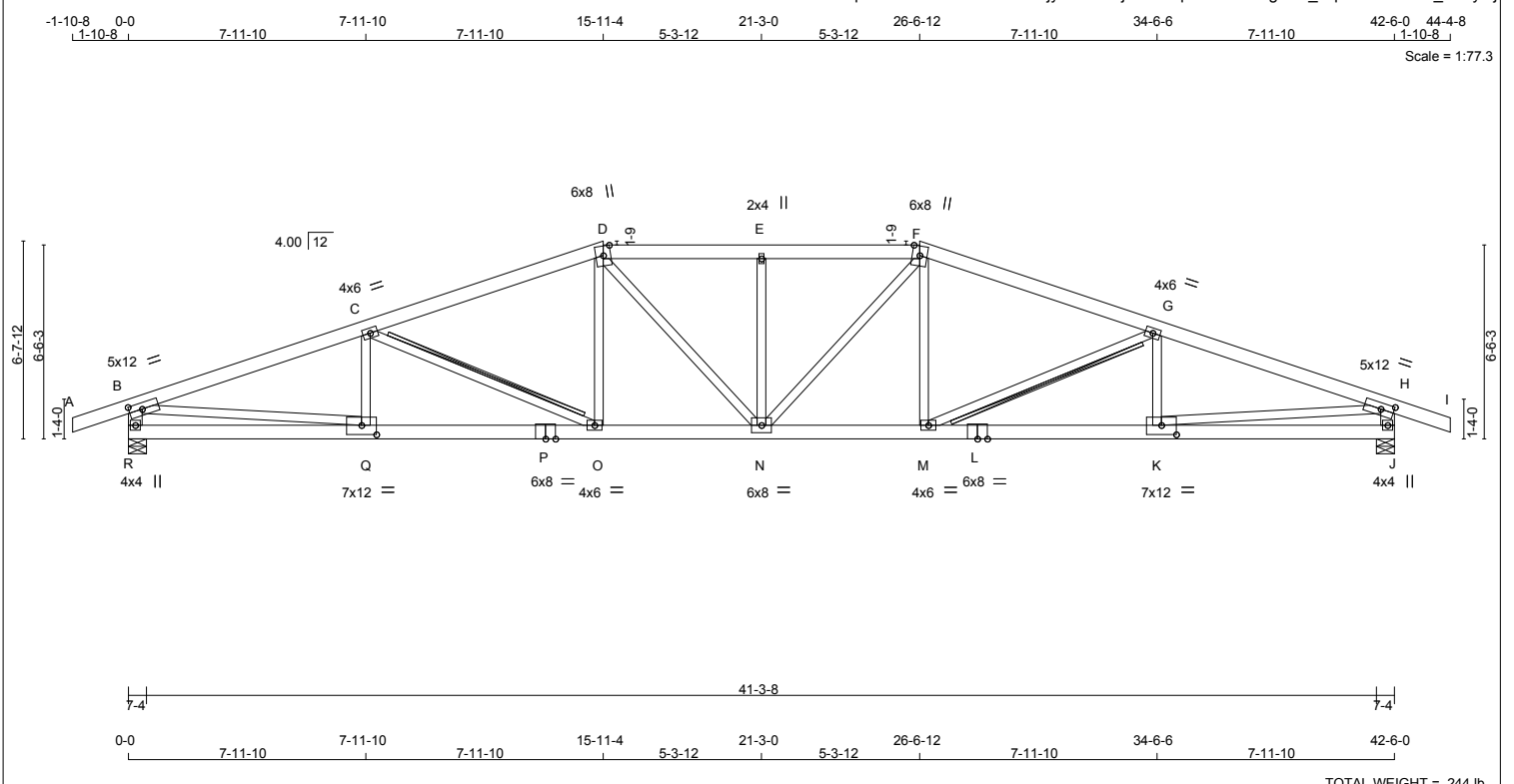


Scale = 1:78.5



TOTAL WEIGHT = 241 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA																																																																																																																																																			
<p>N. L. G. A. RULES</p> <table border="1"> <thead> <tr> <th>CHORDS</th> <th>SIZE</th> <th>LUMBER</th> <th>DESCR.</th> </tr> </thead> <tbody> <tr><td>A - D</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>D - F</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>F - G</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>G - I</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>I - L</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>T - B</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>M - K</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>T - R</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>R - O</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> <tr><td>O - M</td><td>2x6</td><td>DRY</td><td>No.2</td></tr> </tbody> </table> <p>ALL WEBS EXCEPT 2x4 DRY No.2 SPF</p> <p>DRY: SEASONED LUMBER.</p>				CHORDS	SIZE	LUMBER	DESCR.	A - D	2x6	DRY	No.2	D - F	2x6	DRY	No.2	F - G	2x6	DRY	No.2	G - I	2x6	DRY	No.2	I - L	2x6	DRY	No.2	T - B	2x6	DRY	No.2	M - K	2x6	DRY	No.2	T - R	2x6	DRY	No.2	R - O	2x6	DRY	No.2	O - M	2x6	DRY	No.2	<p>BEARINGS</p> <table border="1"> <thead> <tr> <th>FACTORED</th> <th>MAXIMUM FACTORED</th> <th>INPUT</th> <th>REQRD</th> </tr> <tr> <th>GROSS REACTION</th> <th>GROSS REACTION</th> <th>BRG</th> <th>BRG</th> </tr> <tr> <th>JT VERT</th> <th>JT HORZ</th> <th>DOWN</th> <th>UP</th> </tr> </thead> <tbody> <tr> <td>T 3069</td> <td>0</td> <td>3069</td> <td>131</td> </tr> <tr> <td>M 3069</td> <td>0</td> <td>3069</td> <td>0</td> </tr> </tbody> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT T FOR 642 LBS FACTORED UPLIFT</p> <p>PROVIDE ANCHORAGE AT BEARING JOINT M FOR 618 LBS FACTORED UPLIFT</p> <p>NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER</p> <p>PROVIDE FOR 131 LBS FACTORED HORIZONTAL REACTION AT JOINT T</p>				FACTORED	MAXIMUM FACTORED	INPUT	REQRD	GROSS REACTION	GROSS REACTION	BRG	BRG	JT VERT	JT HORZ	DOWN	UP	T 3069	0	3069	131	M 3069	0	3069	0	<p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 31.5 PSF DL = 5.0 PSF</p> <p>BOT CH. LL = 10.0 PSF DL = 7.0 PSF</p> <p>TOTAL LOAD = 53.5 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED - SLOPE REDUCTION FACTOR USED</p> <p>(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p> <p>ALLOWABLE DEFL.(LL)= L/360 (1.42") CALCULATED VERT. DEFL.(LL) = L/999 (0.32") ALLOWABLE DEFL.(TL)= L/240 (2.13") CALCULATED VERT. DEFL.(TL) = L/999 (0.44")</p> <p>CSI: TC=0.56 (F-G-2), BC=0.76 (S-T-1), WB=0.91 (H-P-3), SSI=0.25 (E-F-2)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10</p> <p>SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50</p> <p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</p> <p>NAIL VALUES</p> <table border="1"> <thead> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR</th> <th>SECTION</th> </tr> <tr> <th>(PSI)</th> <th>(PLI)</th> <th>(PLI)</th> <th>(PLI)</th> </tr> <tr> <th>MAX</th> <th>MIN</th> <th>MAX</th> <th>MIN</th> </tr> </thead> <tbody> <tr> <td>MT20</td> <td>618</td> <td>354</td> <td>1667 822 2284 1656</td> </tr> </tbody> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches</p> <p>PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.86 (R) (INPUT = 0.90) JSI METAL= 0.96 (J) (INPUT = 1.00)</p>				PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)	(PLI)	MAX	MIN	MAX	MIN	MT20	618	354	1667 822 2284 1656																																																																
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BRACE MUST COVER 90% OF WEB LENGTH.</p> <p>END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW</p> <p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <thead> <tr> <th colspan="3">CHORDS</th> <th colspan="3">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>FORCE (LBS)</th> <th>FACTORED</th> <th>MEMB.</th> <th>FORCE (LBS)</th> <th>FACTORED</th> </tr> <tr> <th>FR-TO</th> <th></th> <th>VERT. 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INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>				1ST LCASE	MAX./MIN. COMPONENT REACTIONS	JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL	T 2417	1463 / 0 425 / 0 0 / 0 0 / -799 529 / 0 0 / 0	M 2417	1463 / 0 425 / 0 0 / 0 0 / -782 529 / 0 0 / 0	T	0 / 0	0 / 0	94 / -94	0 / 0	0 / 0	CHORDS			WEBS			MEMB.	FORCE (LBS)	FACTORED	MEMB.	FORCE (LBS)	FACTORED	FR-TO		VERT. 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TOTAL WEIGHT = 244 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY No.2	SPF
D - F	2x6	DRY No.2	SPF
F - I	2x6	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
R - P	2x6	DRY No.2	SPF
P - L	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	12.0	2.50	5.25
C	TMVW-t	MT20	4.0	6.0		
D	TTVW+m	MT20	6.0	8.0	Edge	
E	TMVW-w	MT20	2.0	4.0		
F	TTVW+m	MT20	6.0	8.0	Edge	
G	TMVW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	12.0	2.50	5.25
J	BMV1+p	MT20	4.0	4.0		
K	BMVW-t	MT20	7.0	12.0	3.75	6.00
L	BS-t	MT20	6.0	8.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	6.0	8.0		
O	BMVW-t	MT20	4.0	6.0		
P	BS-t	MT20	6.0	8.0		
Q	BMVW-t	MT20	7.0	12.0	3.75	6.00
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	3069	0	3084	118
J	3069	0	3084	0

PROVIDE ANCHORAGE AT BEARING JOINT R FOR 655 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT J FOR 636 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 118 LBS FACTORED HORIZONTAL REACTION AT JOINT R

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2417	1473 / 0	425 / 0	0 / 0	0 / -808	529 / 0	0 / 0
J	2417	1473 / 0	425 / 0	0 / 0	0 / -795	529 / 0	0 / 0

HORIZONTAL REACTIONS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	---	0 / 0	0 / 0	84 / -84	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 2.87FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

2x3 DRY SPF 2100F 1.8E T-BRACE AT C-O, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

MEMB.	C H O R D S			W E B S		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIVE LOAD (PLF)	MAX UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX FACTORED FORCE (LBS)
FR-TO						
A-B	0 / 38	-107.1	-107.1	0.16 (2)	10.00	Q-C -418 / 242
B-C	-5732 / 1209	-107.1	-107.1	0.89 (2)	2.87	C-O -1077 / 326
C-D	-4961 / 1073	-107.1	-107.1	0.79 (2)	3.26	O-D -62 / 664
D-E	-5067 / 1130	-107.1	-107.1	0.35 (2)	3.63	D-N -244 / 1085
E-F	-5067 / 1130	-107.1	-107.1	0.35 (2)	3.63	N-E -881 / 303
F-G	-4961 / 1025	-107.1	-107.1	0.79 (3)	3.26	N-F -245 / 1085
G-H	-5732 / 1170	-107.1	-107.1	0.89 (3)	2.87	M-F -70 / 664
H-I	0 / 38	-107.1	-107.1	0.16 (3)	10.00	M-G -1077 / 348
R-B	-2977 / 690	0.0	0.0	0.19 (2)	6.10	K-G -412 / 236
J-H	-2977 / 671	0.0	0.0	0.19 (3)	6.10	B-Q -1079 / 5496
						K-H -1021 / 5496
R-Q	-105 / 131	-27.5	-27.5	0.19 (4)	6.25	
Q-P	-1121 / 5464	-27.5	-27.5	0.74 (1)	6.25	
P-O	-1121 / 5464	-27.5	-27.5	0.74 (1)	6.25	
O-N	-831 / 4678	-27.5	-27.5	0.64 (1)	6.25	
N-M	-744 / 4678	-27.5	-27.5	0.64 (1)	6.25	
M-L	-1002 / 5464	-27.5	-27.5	0.74 (1)	6.25	
L-K	-1002 / 5464	-27.5	-27.5	0.74 (1)	6.25	
K-J	-7 / 15	-27.5	-27.5	0.19 (4)	10.00	

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 31.5 PSF
DL = 5.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 53.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L. PLUS
2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS
31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.34")
ALLOWABLE DEFL.(TL)= L/240 (2.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.46")

CSI: TC=0.89 (B-C:2), BC=0.74 (K-M:1),
WB=0.96 (G-M:3), SSI=0.34 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

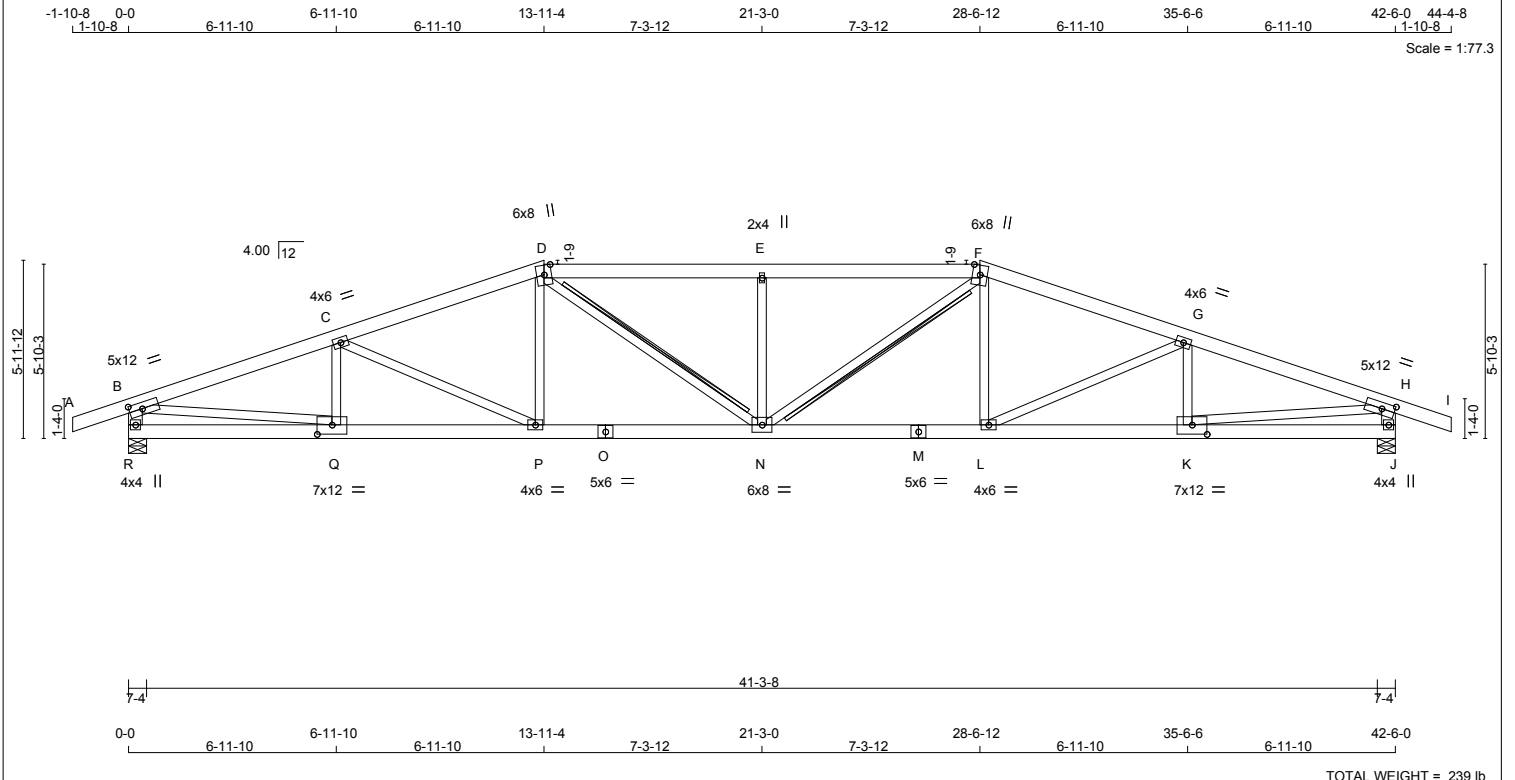
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

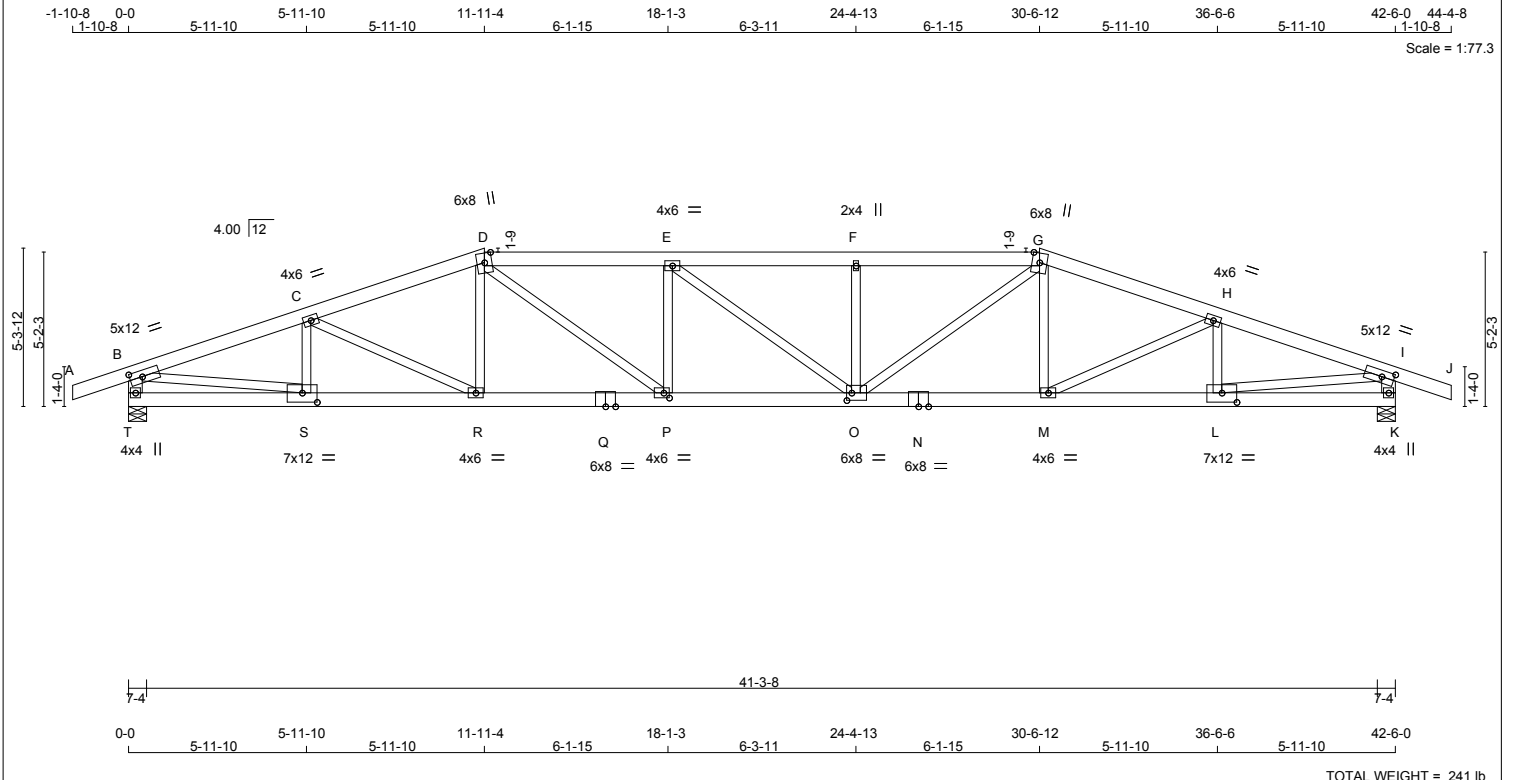
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.81 (Q) (INPUT = 1.00)





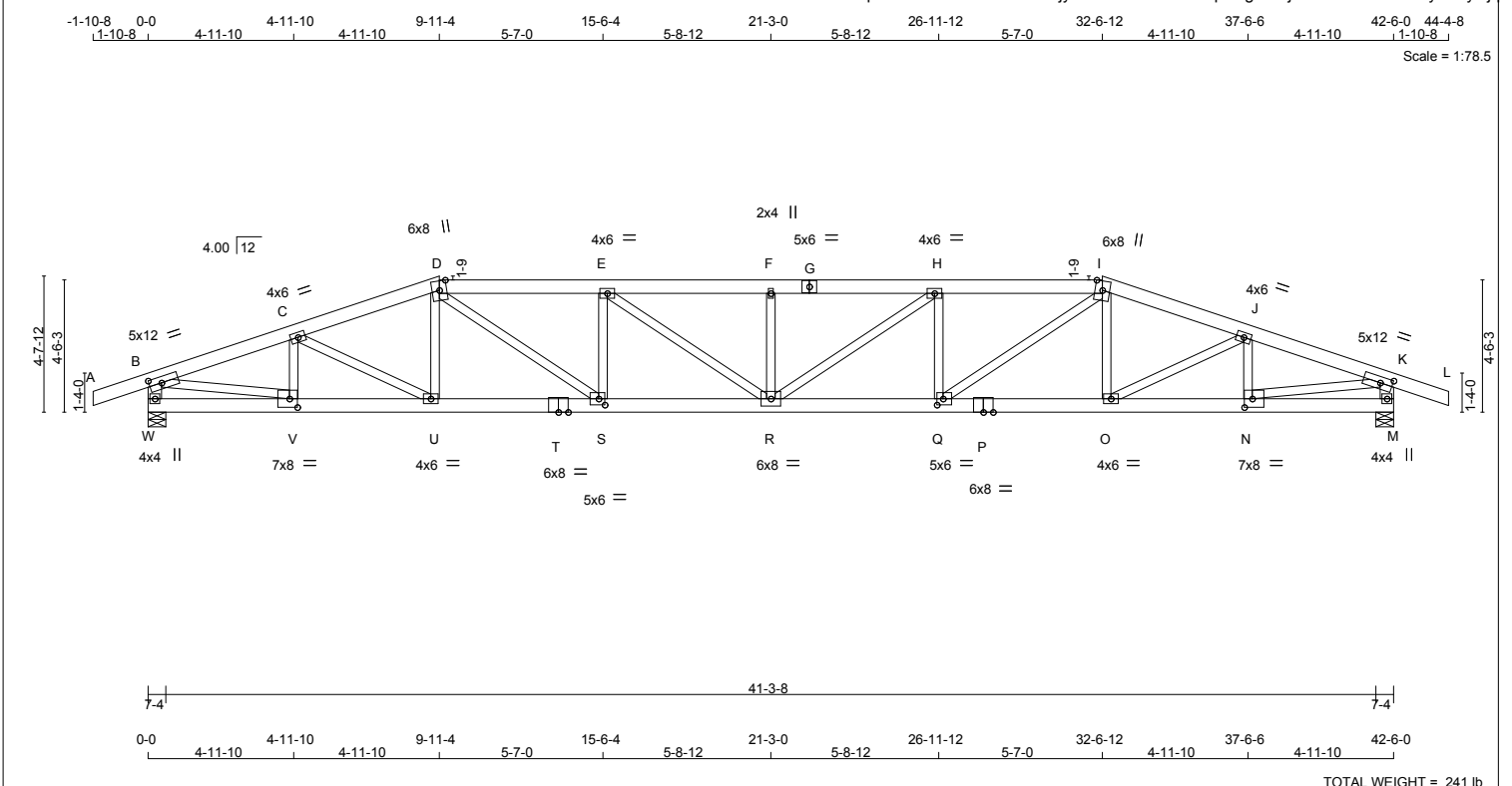
TOTAL WEIGHT = 239 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA																																																																																																																																																																																																																											
<p>N. L. G. A. RULES</p> <table border="1"> <tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr> <tr><td>A - D</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>D - F</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>F - I</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>R - B</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>J - H</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>R - O</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>O - M</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>M - J</td><td>2x6</td><td>DRY No.2</td><td>SPF</td></tr> <tr><td>ALL WEBS EXCEPT</td><td>2x4</td><td>DRY No.2</td><td>SPF</td></tr> </table> <p>DRY: SEASONED LUMBER.</p>				CHORDS	SIZE	LUMBER	DESCR.	A - D	2x6	DRY No.2	SPF	D - F	2x6	DRY No.2	SPF	F - I	2x6	DRY No.2	SPF	R - B	2x6	DRY No.2	SPF	J - H	2x6	DRY No.2	SPF	R - O	2x6	DRY No.2	SPF	O - M	2x6	DRY No.2	SPF	M - J	2x6	DRY No.2	SPF	ALL WEBS EXCEPT	2x4	DRY No.2	SPF	<p>BEARINGS</p> <table border="1"> <tr><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQRD</th></tr> <tr><td>JT GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr> <tr><td>JT VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td></tr> <tr><td>R 3069</td><td>0</td><td>3161</td><td>105</td></tr> <tr><td>J 3069</td><td>0</td><td>3161</td><td>0</td></tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT R FOR 667 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT J FOR 652 LBS FACTORED UPLIFT</p> <p>NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER</p> <p>PROVIDE FOR 105 LBS FACTORED HORIZONTAL REACTION AT JOINT R</p>				FACTORED	MAXIMUM FACTORED	INPUT	REQRD	JT GROSS REACTION	GROSS REACTION	BRG	BRG	JT VERT	HORZ	DOWN	HORZ	R 3069	0	3161	105	J 3069	0	3161	0	<p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 31.5 PSF DL = 5.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 53.5 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS - SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED - SLOPE REDUCTION FACTOR USED</p> <p>(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p> <p>ALLOWABLE DEFL.(LL)= L/360 (1.42") CALCULATED VERT. DEFL.(LL) = L/999 (0.36") ALLOWABLE DEFL.(TL)= L/240 (2.13") CALCULATED VERT. DEFL.(TL) = L/999 (0.50")</p> <p>CSI: TC=0.74 (E-F:2), BC=0.73 (P-Q:2), WB=0.88 (B-Q:2), SSI=0.33 (D-E:2)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10</p> <p>SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50</p> <p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</p> <p>NAIL VALUES</p> <table border="1"> <tr><th>PLATE</th><th>GRIP(DRY)</th><th>SHEAR</th><th>SECTION</th></tr> <tr><td>(PSI)</td><td>(PLI)</td><td>(PLI)</td><td>(PSI)</td></tr> <tr><td>MAX</td><td>MIN</td><td>MAX</td><td>MIN</td></tr> <tr><td>618</td><td>354</td><td>1667</td><td>822</td></tr> <tr><td>2284</td><td>1656</td><td></td><td></td></tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches</p> <p>PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.90 (B) (INPUT = 0.90) JSI METAL= 0.80 (Q) (INPUT = 1.00)</p>				PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)	(PLI)	(PSI)	MAX	MIN	MAX	MIN	618	354	1667	822	2284	1656																																																																																																																																										
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UNBRACED TOP CHORD LENGTH = 3.06FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.</p> <p>2x4 DRY SPF No.2 T-BRACE AT D-N, F-N</p> <p>FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.</p> <p>END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW</p>				<p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <thead> <tr> <th colspan="4">CHORDS</th> <th colspan="4">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>FACTORED LC1 MAX (CSI (LC))</th> <th>MAX UNBRAC LENGTH</th> <th>MEMB.</th> <th>MAX. 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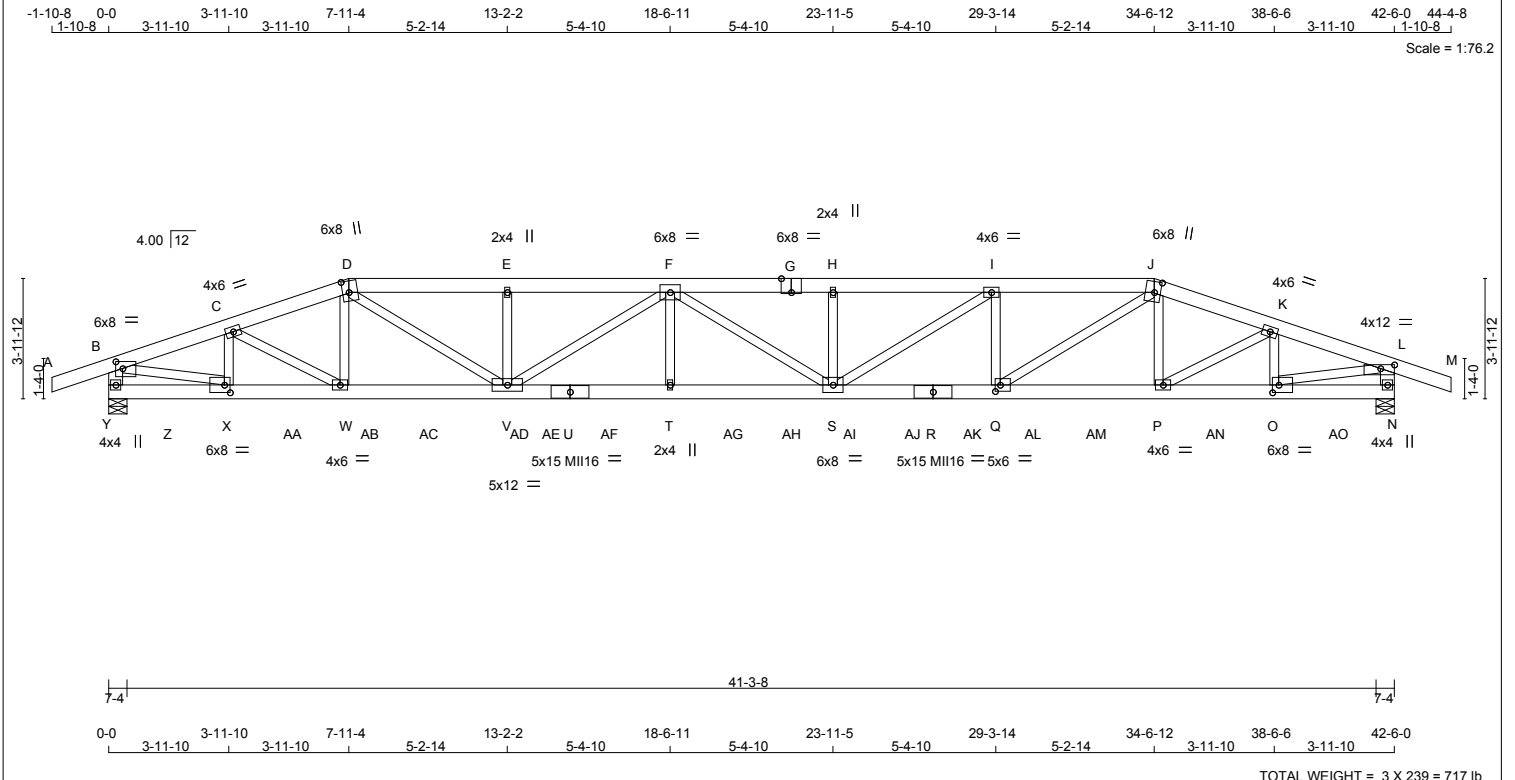
TOTAL WEIGHT = 241 lb

<p>LUMBER</p> <p>N. L. G. A. RULES</p> <table border="1"> <tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr> <tr><td>A - D</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>D - G</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>G - J</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>T - B</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>K - I</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>T - Q</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>Q - N</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>N - K</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> </table> <p>ALL WEBS 2x4 DRY No.2 SPF EXCEPT</p> <p>DRY: SEASONED LUMBER.</p>	CHORDS	SIZE	LUMBER	DESCR.	A - D	2x6 DRY	No.2	SPF	D - G	2x6 DRY	No.2	SPF	G - J	2x6 DRY	No.2	SPF	T - B	2x6 DRY	No.2	SPF	K - I	2x6 DRY	No.2	SPF	T - Q	2x6 DRY	No.2	SPF	Q - N	2x6 DRY	No.2	SPF	N - K	2x6 DRY	No.2	SPF	<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</p> <p>BEARINGS</p> <table border="1"> <tr><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQRD</th></tr> <tr><td>GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr> <tr><td>JT VERT</td><td>DOWN</td><td>HORZ</td><td>UPLIFT</td></tr> <tr><td>T</td><td>3229</td><td>0</td><td>3258</td></tr> <tr><td>K</td><td>3229</td><td>0</td><td>3258</td></tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT T FOR 823 LBS FACTORED UPLIFT</p> <p>PROVIDE ANCHORAGE AT BEARING JOINT K FOR 811 LBS FACTORED UPLIFT</p> <p>NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER</p> <p>PROVIDE FOR 103 LBS FACTORED HORIZONTAL REACTION AT JOINT T</p>	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	GROSS REACTION	GROSS REACTION	BRG	BRG	JT VERT	DOWN	HORZ	UPLIFT	T	3229	0	3258	K	3229	0	3258	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 33.9 PSF DL = 5.0 PSF</p> <p>BOT CH. LL = 10.0 PSF DL = 7.0 PSF</p> <p>TOTAL LOAD = 55.8 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED</p> <p>(80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD</p>																																																																																																																																												
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	<p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <tr><th colspan="4">C H O R D S</th><th colspan="4">W E B S</th></tr> <tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. UNBRAC. CSI (LC)</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. UNBRAC. CSI (LC)</th><th></th></tr> <tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>LENGTH</td><td>FR-TO</td><td></td><td></td></tr> <tr><td>A-B</td><td>0 / 40</td><td>-114.0</td><td>-114.0</td><td>0.17 (2)</td><td>10.00</td><td>S-C</td><td>-681 / 294</td></tr> <tr><td>B-C</td><td>-5768 / 1472</td><td>-114.0</td><td>-114.0</td><td>0.47 (2)</td><td>3.33</td><td>C-R</td><td>-257 / 578</td></tr> <tr><td>C-D</td><td>-5716 / 1490</td><td>-114.0</td><td>-114.0</td><td>0.45 (2)</td><td>3.35</td><td>R-D</td><td>-121 / 347</td></tr> <tr><td>D-E</td><td>-6699 / 1769</td><td>-114.0</td><td>-114.0</td><td>0.54 (1)</td><td>3.03</td><td>D-P</td><td>-511 / 1939</td></tr> <tr><td>E-F</td><td>-6693 / 1723</td><td>-114.0</td><td>-114.0</td><td>0.54 (1)</td><td>3.04</td><td>P-E</td><td>-1023 / 396</td></tr> <tr><td>F-G</td><td>-6694 / 1725</td><td>-114.0</td><td>-114.0</td><td>0.54 (1)</td><td>3.03</td><td>E-O</td><td>-348 / 337</td></tr> <tr><td>G-H</td><td>-5717 / 1462</td><td>-114.0</td><td>-114.0</td><td>0.45 (3)</td><td>3.35</td><td>O-F</td><td>-831 / 352</td></tr> <tr><td>H-I</td><td>-5768 / 1453</td><td>-114.0</td><td>-114.0</td><td>0.47 (3)</td><td>3.33</td><td>O-G</td><td>-507 / 1933</td></tr> <tr><td>I-J</td><td>0 / 40</td><td>-114.0</td><td>-114.0</td><td>0.17 (3)</td><td>10.00</td><td>M-G</td><td>-129 / 348</td></tr> <tr><td>T-B</td><td>-3160 / 845</td><td>0.0</td><td>0.0</td><td>0.20 (2)</td><td>5.95</td><td>M-H</td><td>-280 / 606</td></tr> <tr><td>K-I</td><td>-3160 / 833</td><td>0.0</td><td>0.0</td><td>0.20 (3)</td><td>5.95</td><td>L-H</td><td>-682 / 288</td></tr> <tr><td>T-S</td><td>-88 / 117</td><td>-27.5</td><td>-27.5</td><td>0.13 (5)</td><td>6.25</td><td>B-S</td><td>-1341 / 5545</td></tr> <tr><td>S-R</td><td>-1374 / 5488</td><td>-27.5</td><td>-27.5</td><td>0.74 (1)</td><td>6.25</td><td>L-I</td><td>-1300 / 5545</td></tr> <tr><td>R-Q</td><td>-1253 / 5416</td><td>-27.5</td><td>-27.5</td><td>0.71 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>Q-P</td><td>-1253 / 5416</td><td>-27.5</td><td>-27.5</td><td>0.71 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>P-O</td><td>-1553 / 6698</td><td>-27.5</td><td>-27.5</td><td>0.87 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>O-N</td><td>-1150 / 5416</td><td>-27.5</td><td>-27.5</td><td>0.71 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>N-M</td><td>-1150 / 5416</td><td>-27.5</td><td>-27.5</td><td>0.71 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>M-L</td><td>-1272 / 5488</td><td>-27.5</td><td>-27.5</td><td>0.74 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>L-K</td><td>-8 / 17</td><td>-27.5</td><td>-27.5</td><td>0.13 (6)</td><td>10.00</td><td></td><td></td></tr> </table>	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. CSI (LC)		FR-TO		FROM	TO	LENGTH	FR-TO			A-B	0 / 40	-114.0	-114.0	0.17 (2)	10.00	S-C	-681 / 294	B-C	-5768 / 1472	-114.0	-114.0	0.47 (2)	3.33	C-R	-257 / 578	C-D	-5716 / 1490	-114.0	-114.0	0.45 (2)	3.35	R-D	-121 / 347	D-E	-6699 / 1769	-114.0	-114.0	0.54 (1)	3.03	D-P	-511 / 1939	E-F	-6693 / 1723	-114.0	-114.0	0.54 (1)	3.04	P-E	-1023 / 396	F-G	-6694 / 1725	-114.0	-114.0	0.54 (1)	3.03	E-O	-348 / 337	G-H	-5717 / 1462	-114.0	-114.0	0.45 (3)	3.35	O-F	-831 / 352	H-I	-5768 / 1453	-114.0	-114.0	0.47 (3)	3.33	O-G	-507 / 1933	I-J	0 / 40	-114.0	-114.0	0.17 (3)	10.00	M-G	-129 / 348	T-B	-3160 / 845	0.0	0.0	0.20 (2)	5.95	M-H	-280 / 606	K-I	-3160 / 833	0.0	0.0	0.20 (3)	5.95	L-H	-682 / 288	T-S	-88 / 117	-27.5	-27.5	0.13 (5)	6.25	B-S	-1341 / 5545	S-R	-1374 / 5488	-27.5	-27.5	0.74 (1)	6.25	L-I	-1300 / 5545	R-Q	-1253 / 5416	-27.5	-27.5	0.71 (1)	6.25			Q-P	-1253 / 5416	-27.5	-27.5	0.71 (1)	6.25			P-O	-1553 / 6698	-27.5	-27.5	0.87 (1)	6.25			O-N	-1150 / 5416	-27.5	-27.5	0.71 (1)	6.25			N-M	-1150 / 5416	-27.5	-27.5	0.71 (1)	6.25			M-L	-1272 / 5488	-27.5	-27.5	0.74 (1)	6.25			L-K	-8 / 17	-27.5	-27.5	0.13 (6)	10.00			<p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.</p> <p>NAIL VALUES</p> <table border="1"> <tr><th>PLATE</th><th>GRIP(DRY)</th><th>SHEAR (PSI)</th><th>SECTION (PLI)</th></tr> <tr><td>MT20</td><td>618</td><td>354</td><td>1667</td></tr> <tr><td></td><td>822</td><td>2284</td><td>1656</td></tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches</p> <p>PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.90 (B) (INPUT = 0.90) JSI METAL = 0.81 (S) (INPUT = 1.00)</p>	PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MT20	618	354	1667		822	2284	1656
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FR-TO		FROM	TO	LENGTH	FR-TO																																																																																																																																																																																																	
A-B	0 / 40	-114.0	-114.0	0.17 (2)	10.00	S-C	-681 / 294																																																																																																																																																																																															
B-C	-5768 / 1472	-114.0	-114.0	0.47 (2)	3.33	C-R	-257 / 578																																																																																																																																																																																															
C-D	-5716 / 1490	-114.0	-114.0	0.45 (2)	3.35	R-D	-121 / 347																																																																																																																																																																																															
D-E	-6699 / 1769	-114.0	-114.0	0.54 (1)	3.03	D-P	-511 / 1939																																																																																																																																																																																															
E-F	-6693 / 1723	-114.0	-114.0	0.54 (1)	3.04	P-E	-1023 / 396																																																																																																																																																																																															
F-G	-6694 / 1725	-114.0	-114.0	0.54 (1)	3.03	E-O	-348 / 337																																																																																																																																																																																															
G-H	-5717 / 1462	-114.0	-114.0	0.45 (3)	3.35	O-F	-831 / 352																																																																																																																																																																																															
H-I	-5768 / 1453	-114.0	-114.0	0.47 (3)	3.33	O-G	-507 / 1933																																																																																																																																																																																															
I-J	0 / 40	-114.0	-114.0	0.17 (3)	10.00	M-G	-129 / 348																																																																																																																																																																																															
T-B	-3160 / 845	0.0	0.0	0.20 (2)	5.95	M-H	-280 / 606																																																																																																																																																																																															
K-I	-3160 / 833	0.0	0.0	0.20 (3)	5.95	L-H	-682 / 288																																																																																																																																																																																															
T-S	-88 / 117	-27.5	-27.5	0.13 (5)	6.25	B-S	-1341 / 5545																																																																																																																																																																																															
S-R	-1374 / 5488	-27.5	-27.5	0.74 (1)	6.25	L-I	-1300 / 5545																																																																																																																																																																																															
R-Q	-1253 / 5416	-27.5	-27.5	0.71 (1)	6.25																																																																																																																																																																																																	
Q-P	-1253 / 5416	-27.5	-27.5	0.71 (1)	6.25																																																																																																																																																																																																	
P-O	-1553 / 6698	-27.5	-27.5	0.87 (1)	6.25																																																																																																																																																																																																	
O-N	-1150 / 5416	-27.5	-27.5	0.71 (1)	6.25																																																																																																																																																																																																	
N-M	-1150 / 5416	-27.5	-27.5	0.71 (1)	6.25																																																																																																																																																																																																	
M-L	-1272 / 5488	-27.5	-27.5	0.74 (1)	6.25																																																																																																																																																																																																	
L-K	-8 / 17	-27.5	-27.5	0.13 (6)	10.00																																																																																																																																																																																																	
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<p>APEGA Permit No. P3837</p>	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7 } PSF AT { 40-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>																																																																																																																																																																																																					



TOTAL WEIGHT = 241 lb

<p>LUMBER</p> <p>N. L. G. A. RULES</p> <table border="1"> <tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr> <tr><td>A - D</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>D - G</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>G - I</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>I - L</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>W - B</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>M - K</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>W - T</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>T - P</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>P - M</td><td>2x6 DRY</td><td>No.2</td><td>SPF</td></tr> </table> <p>ALL WEBS EXCEPT 2x4 DRY No.2 SPF</p> <p>DRY: SEASONED LUMBER.</p>	CHORDS	SIZE	LUMBER	DESCR.	A - D	2x6 DRY	No.2	SPF	D - G	2x6 DRY	No.2	SPF	G - I	2x6 DRY	No.2	SPF	I - L	2x6 DRY	No.2	SPF	W - B	2x6 DRY	No.2	SPF	M - K	2x6 DRY	No.2	SPF	W - T	2x6 DRY	No.2	SPF	T - P	2x6 DRY	No.2	SPF	P - M	2x6 DRY	No.2	SPF	<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</p> <p>BEARINGS</p> <table border="1"> <tr><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQRD</th></tr> <tr><td>GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr> <tr><td>JT VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td></tr> <tr><td>UPLIFT</td><td>IN-SX</td><td>IN-SX</td><td>IN-SX</td></tr> <tr><td>W</td><td>3069</td><td>0</td><td>3284</td></tr> <tr><td>M</td><td>3069</td><td>0</td><td>3284</td></tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT W FOR 685 LBS FACTORED UPLIFT</p> <p>PROVIDE ANCHORAGE AT BEARING JOINT M FOR 678 LBS FACTORED UPLIFT</p> <p>NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER</p> <p>PROVIDE FOR 78 LBS FACTORED HORIZONTAL REACTION AT JOINT W</p>	FACTORED	MAXIMUM FACTORED	INPUT	REQRD	GROSS REACTION	GROSS REACTION	BRG	BRG	JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	W	3069	0	3284	M	3069	0	3284	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 31.5 PSF DL = 5.0 PSF</p> <p>BOT CH. LL = 10.0 PSF DL = 7.0 PSF</p> <p>TOTAL LOAD = 53.5 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p>																																																																																																											
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<p>PLATES (table is in inches)</p> <table border="1"> <tr><th>JT TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr> <tr><td>B</td><td>TMW-t</td><td>MT20</td><td>5.0</td><td>12.0</td><td>2.50</td><td>5.00</td></tr> <tr><td>C, E, H, J</td><td></td><td></td><td></td><td></td><td></td><td></td></tr> <tr><td>C</td><td>TMW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>D</td><td>TTW+m</td><td>MT20</td><td>6.0</td><td>8.0</td><td>Edge</td><td></td></tr> <tr><td>F</td><td>TMW+w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>G</td><td>TS-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>I</td><td>TTW+m</td><td>MT20</td><td>6.0</td><td>8.0</td><td>Edge</td><td></td></tr> <tr><td>K</td><td>TMW-t</td><td>MT20</td><td>5.0</td><td>12.0</td><td>2.50</td><td>5.00</td></tr> <tr><td>M</td><td>BMV1+p</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr> <tr><td>N</td><td>BMW-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td>3.50</td><td>3.25</td></tr> <tr><td>O</td><td>BMW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>P</td><td>BS-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td></td><td></td></tr> <tr><td>Q</td><td>BMW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td>2.50</td><td>2.50</td></tr> <tr><td>R</td><td>BMW-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td></td><td></td></tr> <tr><td>S</td><td>BMW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td>2.50</td><td>2.50</td></tr> <tr><td>T</td><td>BS-t</td><td>MT20</td><td>6.0</td><td>8.0</td><td></td><td></td></tr> <tr><td>U</td><td>BMW-t</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr> <tr><td>V</td><td>BMW-t</td><td>MT20</td><td>7.0</td><td>8.0</td><td>3.50</td><td>3.25</td></tr> <tr><td>W</td><td>BMV1+p</td><td>MT20</td><td>4.0</td><td>4.0</td><td></td><td></td></tr> </table> <p>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</p>	JT TYPE	PLATES	W	LEN	Y	X	B	TMW-t	MT20	5.0	12.0	2.50	5.00	C, E, H, J							C	TMW-t	MT20	4.0	6.0			D	TTW+m	MT20	6.0	8.0	Edge		F	TMW+w	MT20	2.0	4.0			G	TS-t	MT20	5.0	6.0			I	TTW+m	MT20	6.0	8.0	Edge		K	TMW-t	MT20	5.0	12.0	2.50	5.00	M	BMV1+p	MT20	4.0	4.0			N	BMW-t	MT20	7.0	8.0	3.50	3.25	O	BMW-t	MT20	4.0	6.0			P	BS-t	MT20	6.0	8.0			Q	BMW-t	MT20	5.0	6.0	2.50	2.50	R	BMW-t	MT20	6.0	8.0			S	BMW-t	MT20	5.0	6.0	2.50	2.50	T	BS-t	MT20	6.0	8.0			U	BMW-t	MT20	4.0	6.0			V	BMW-t	MT20	7.0	8.0	3.50	3.25	W	BMV1+p	MT20	4.0	4.0			<p>UNFACTORED REACTIONS</p> <table border="1"> <tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr> <tr><td>W</td><td>2417</td><td>1606 / 0</td><td>425 / 0</td><td>0 / 0</td><td>0 / -830</td><td>529 / 0</td><td>0 / 0</td></tr> <tr><td>M</td><td>2417</td><td>1606 / 0</td><td>425 / 0</td><td>0 / 0</td><td>0 / -824</td><td>529 / 0</td><td>0 / 0</td></tr> </table> <p>HORIZONTAL REACTIONS</p> <table border="1"> <tr><th>JT</th><th>W</th><th>Y</th><th>X</th></tr> <tr><td>W</td><td>0 / 0</td><td>0 / 0</td><td>56 / -56</td></tr> </table> <p>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M</p> <p>BRACING</p> <p>MAX. UNBRACED TOP CHORD LENGTH = 2.80FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.</p> <p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p>	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	W	2417	1606 / 0	425 / 0	0 / 0	0 / -830	529 / 0	0 / 0	M	2417	1606 / 0	425 / 0	0 / 0	0 / -824	529 / 0	0 / 0	JT	W	Y	X	W	0 / 0	0 / 0	56 / -56	<p>DESIGN ASSUMPTIONS</p> <p>- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED</p> <p>- SLOPE REDUCTION FACTOR USED</p> <p>(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p> <p>ALLOWABLE DEFL.(LL)= L/360 (1.42") CALCULATED VERT. DEFL.(LL) = L/999 (0.49")</p> <p>ALLOWABLE DEFL.(TL)= L/240 (2.13") CALCULATED VERT. DEFL.(TL) = L/769 (0.66")</p> <p>CSI: TC=0.58 (F-H:2), BC=0.95 (R-S:2), WB=0.86 (B-V:2), SSI=0.24 (H-12)</p> <p>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10</p> <p>SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50</p>
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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY No.2	SPF
D - G	2x6	DRY No.2	SPF
G - J	2x6	DRY No.2	SPF
J - M	2x6	DRY No.2	SPF
Y - B	2x6	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
Y - U	2x6	DRY 2100F 1.8E	SPF
U - R	2x6	DRY 2100F 1.8E	SPF
R - N	2x6	DRY 2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
Y	9091	0	9351	66
N	8835	0	9096	0

PROVIDE ANCHORAGE AT BEARING JOINT Y FOR 2416 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT N FOR 2342 LBS FACTORED UPLIFT

NOTE: ANCHORAGE REQUIRED FOR LARGE UPLIFT FORCES, SHALL BE PROVIDED BY BLDG. DESIGNER

PROVIDE FOR 66 LBS FACTORED HORIZONTAL REACTION AT JOINT Y

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	=	31.5	PSF
DL	=	5.0	PSF
BOT CH. LL	=	10.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	53.5	PSF

SPACING = 24.0 IN. C/C

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-D	2	12	TOP
D-G	2	12	TOP
G-J	2	12	TOP
J-M	2	12	TOP
Y-B	2	12	TOP
N-L	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
Y-U	2	12	TOP
U-R	2	12	TOP
R-N	2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS			
2x4	1	6	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	7001	4719 / 0	1062 / 0	0 / 0	0 / -2621	1394 / 0	0 / 0
N	6806	4589 / 0	1035 / 0	0 / 0	0 / -2545	1357 / 0	0 / 0

HORIZONTAL REACTIONS

Y	0 / 0	0 / 0	0 / 0	47 / -47	0 / 0	0 / 0
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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.56FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (18)

LOADING IN FLAT SECTION BASED ON A SLOPE OF 4.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 4 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	8.0	2.75	2.75
C, I, K						
C	TMVW-t	MT20	4.0	6.0		
D	TTVW+m	MT20	6.0	8.0	4.50	2.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	6.0	8.0		
G	TS-t	MT20	6.0	8.0	Edge	4.00
H	TMVW+w	MT20	2.0	4.0		
J	TTVW+m	MT20	6.0	8.0	4.25	2.50
L	TMVW-p	MT20	4.0	12.0	1.50	5.50
N	BMV1+p	MT20	4.0	4.0		
O	BMVW-t	MT20	6.0	8.0	3.00	2.50
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00
R	BS-t	MI16	5.0	15.0		
S	BMVW-t	MT20	6.0	8.0		
T	BMVW+w	MT20	2.0	4.0		
U	BS-t	MI16	5.0	15.0		
V	BMVW-t	MT20	5.0	12.0		
W	BMVW-t	MT20	4.0	6.0		

C H O R D S				W E B S			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED (CSI (LC))	MEMB.	FORCE (LBS)	MAX. UNBRACED (CSI (LC))	MAX. FACTORED (CSI (LC))
FR-TO				FR-TO			
A-B	0 / 38	-107.1	-107.1	0.05 (2)	10.00	X-C -2596 / 706	0.10 (2)
B-C	-16016 / 4160	-107.1	-107.1	0.23 (3)	3.65	C-W -759 / 3233	0.17 (3)
C-D	-18599 / 4848	-107.1	-107.1	0.27 (3)	3.37	W-D -172 / 754	0.04 (1)
D-E	-24319 / 6373	-107.1	-107.1	0.50 (2)	2.84	D-V -2156 / 8349	0.45 (3)
E-F	-24319 / 6373	-107.1	-107.1	0.50 (2)	2.84	V-E -598 / 222	0.03 (3)
F-G	-27624 / 7249	-107.1	-107.1	0.62 (3)	2.56	V-F -4488 / 1191	0.74 (3)
G-H	-27624 / 7249	-107.1	-107.1	0.62 (3)	2.56	T-F -427 / 1808	0.10 (2)
H-I	-27624 / 7249	-107.1	-107.1	0.62 (3)	2.56	F-S -394 / 98	0.07 (2)
I-J	-24004 / 6279	-107.1	-107.1	0.48 (3)	2.87	S-H -601 / 214	0.03 (3)
J-K	-17960 / 4669	-107.1	-107.1	0.26 (2)	3.44	S-I -1220 / 4618	0.25 (2)
K-L	-15557 / 4037	-107.1	-107.1	0.21 (2)	3.71	Q-I -3184 / 906	0.16 (2)
L-M	0 / 38	-107.1	-107.1	0.05 (3)	10.00	Q-J -2251 / 8683	0.47 (2)
Y-B	-8918 / 2319	0.0	0.0	0.19 (2)	6.10	P-J -102 / 385	0.02 (1)
N-L	-8674 / 2249	0.0	0.0	0.18 (3)	6.17	P-K -705 / 3034	0.16 (2)
Y-Z	-53 / 79	-27.5	-27.5	0.10 (2)	6.25	O-K -2447 / 661	0.09 (3)
Z-X	-53 / 79	-27.5	-27.5	0.10 (2)	6.25	B-X -3981 / 15523	0.83 (2)
X-AA	-3921 / 15169	-27.5	-27.5	0.32 (2)	6.25	O-L -3840 / 15078	0.81 (3)
AA-W	-3921 / 15169	-27.5	-27.5	0.32 (2)	6.25		
W-AB	-4498 / 17631	-27.5	-27.5	0.37 (2)	6.25		
AB-AC	-4498 / 17631	-27.5	-27.5	0.37 (2)	6.25		
AC-AD	-4498 / 17631	-27.5	-27.5	0.37 (2)	6.25		
AD-V	-4498 / 17631	-27.5	-27.5	0.37 (2)	6.25		
V-AE	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
AE-U	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
U-AF	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
AF-T	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
T-AG	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
AG-AH	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
AH-S	-7178 / 27830	-27.5	-27.5	0.54 (2)	6.25		
S-AI	-6090 / 24004	-27.5	-27.5	0.49 (3)	6.25		
AI-AJ	-6090 / 24004	-27.5	-27.5	0.49 (3)	6.25		
AJ-R	-6090 / 24004	-27.5	-27.5	0.49 (3)	6.25		
R-AK	-6090 / 24004	-27.5	-27.5	0.49 (3)	6.25		
AK-Q	-6090 / 24004	-27.5	-27.5	0.49 (3)	6.25		
Q-AL	-4274 / 17039	-27.5	-27.5	0.34 (3)	6.25		
AL-AM	-4274 / 17039	-27.5	-27.5	0.34 (3)	6.25		
AM-P	-4274 / 17039	-27.5	-27.5	0.34 (3)	6.25		
P-AN	-3740 / 14734	-27.5	-27.5	0.30 (3)	6.25		
AN-O	-3740 / 14734	-27.5	-27.5	0.30 (3)	6.25		
O-AO	-7 / 15	-27.5	-27.5	0.10 (3)	10.00		
AO-N	-7 / 15	-27.5	-27.5	0.10 (3)	10.00		

DESIGN ASSUMPTIONS

- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED
- SLOPE REDUCTION FACTOR USED

(74 % OF 39.7 P.S.F. G.S.L PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.42")

CALCULATED VERT. DEFL.(LL) = L/848 (0.60")

ALLOWABLE DEFL.(TL)= L/240 (2.13")

CALCULATED VERT. DEFL.(TL) = L/654 (0.78")

CSI: TC=0.62 (H-I:3), BC=0.54 (S-T:2), WB=0.83 (B-X:2), SSI=0.20 (Q-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00

WIND LOAD IMPORTANCE FACTOR = 1.00

LIVE LOAD IMPORTANCE FACTOR = 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
MI16	473	276	2341

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)

JSI METAL= 0.95 (X) (INPUT = 1.00)

APEGA Permit No. P3837

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	38-6-0	-527	-527	142	TOP	VERT	TOTAL
P	34-6-0	-565	-565	154	TOP	VERT	TOTAL

CONTINUED ON PAGE 2

JOB NAME G1507147	TRUSS NAME C6	QUANTITY 1	PLY 3	JOB DESC. W/O# 090	DRWG NO. P4324120
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Nelson Lumber, Grande Prairie, AB

Version 7.630 S Jul 9 2015 MiTek Industries, Inc. Mon Aug 24 11:38:50 2015 Page 2
ID:Uqb72Eaw6FPE10XPJ82uYjy3SS-fkXsSOgJfs1gTvetHuGkFghSiE0srrLfeainZSykfp

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
X	BMWW-t	MT20	6.0	8.0	3.00	2.25
Y	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

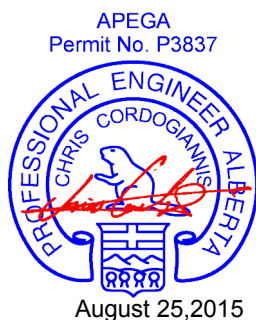
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 507.9 lbs FACTORED DOWN AND 174.1 lbs FACTORED UP AT 2-0-0, 527.3 lbs FACTORED DOWN AND 141.7 lbs FACTORED UP AT 4-0-0, 518.8 lbs FACTORED DOWN AND 146.4 lbs FACTORED UP AT 6-0-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 8-0-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 10-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 12-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 14-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 16-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 18-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 20-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 22-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 24-6-0, 598.9 lbs FACTORED DOWN AND 178.6 lbs FACTORED UP AT 26-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 28-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 30-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 32-6-0, 565.2 lbs FACTORED DOWN AND 153.6 lbs FACTORED UP AT 34-6-0, 518.8 lbs FACTORED DOWN AND 146.4 lbs FACTORED UP AT 36-6-0, AND 527.3 lbs FACTORED DOWN AND 141.7 lbs FACTORED UP AT 38-6-0, AND 507.9 lbs FACTORED DOWN AND 174.1 lbs FACTORED UP AT 40-6-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	18-6-0	-599	-599	179	TOP	VERT	TOTAL
W	8-0-0	-565	-565	154	TOP	VERT	TOTAL
X	4-0-0	-527	-527	142	TOP	VERT	TOTAL
Z	2-0-0	-508	-508	174	TOP	VERT	TOTAL
AA	6-0-0	-519	-519	146	TOP	VERT	TOTAL
AB	8-6-0	-565	-565	154	TOP	VERT	TOTAL
AC	10-6-0	-565	-565	154	TOP	VERT	TOTAL
AD	12-6-0	-565	-565	154	TOP	VERT	TOTAL
AE	14-6-0	-565	-565	154	TOP	VERT	TOTAL
AF	16-6-0	-599	-599	179	TOP	VERT	TOTAL
AG	20-6-0	-599	-599	179	TOP	VERT	TOTAL
AH	22-6-0	-599	-599	179	TOP	VERT	TOTAL
AI	24-6-0	-599	-599	179	TOP	VERT	TOTAL
AJ	26-6-0	-599	-599	179	TOP	VERT	TOTAL
AK	28-6-0	-565	-565	154	TOP	VERT	TOTAL
AL	30-6-0	-565	-565	154	TOP	VERT	TOTAL
AM	32-6-0	-565	-565	154	TOP	VERT	TOTAL
AN	36-6-0	-519	-519	146	TOP	VERT	TOTAL
AO	40-6-0	-508	-508	174	TOP	VERT	TOTAL

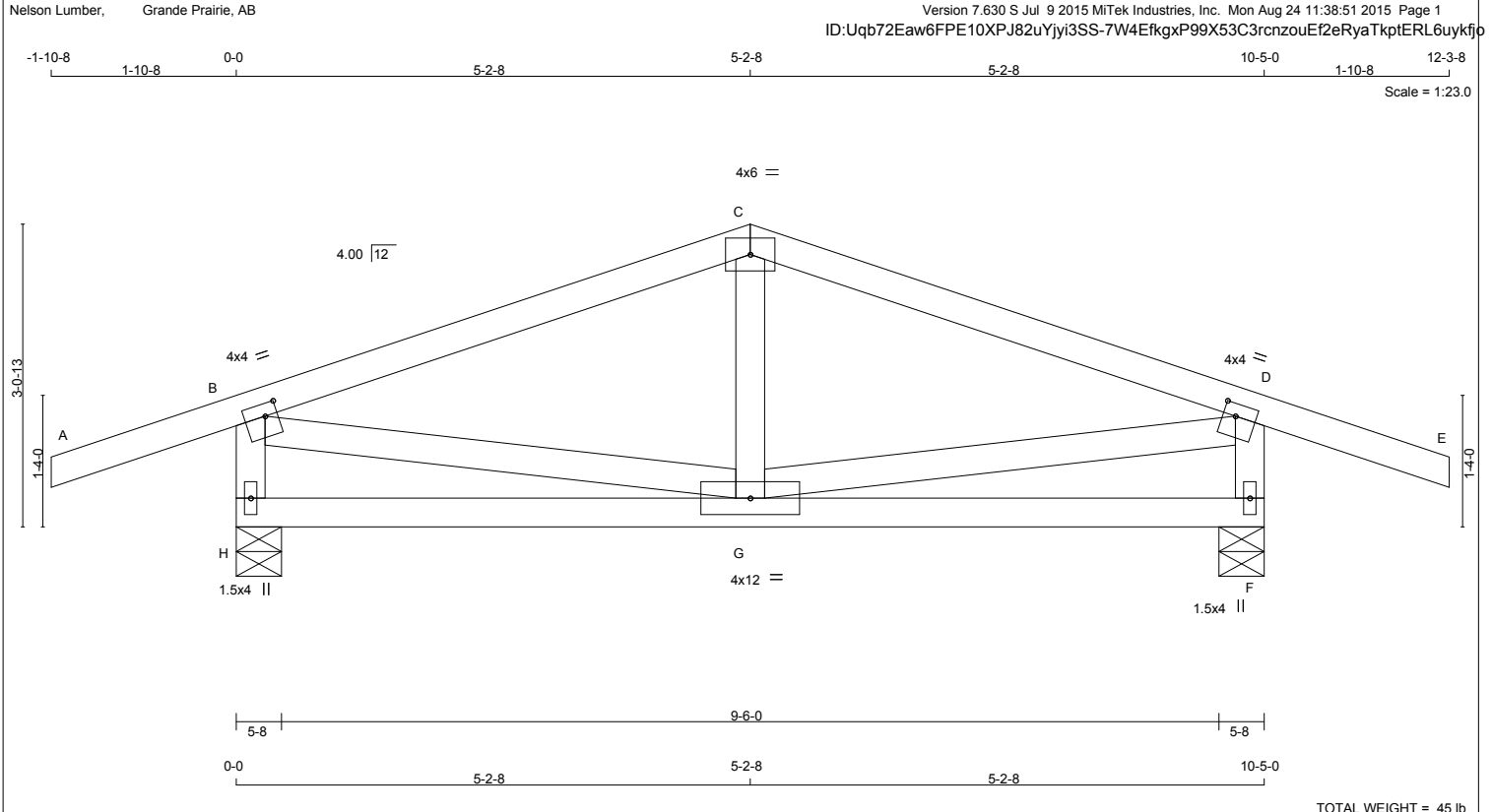
TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (22-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-'08 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 45 lb [M]F

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
ALL WEBS	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	1.50	1.50
C	TTW-p	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	4.0	1.50	1.50
F	BMV1+p	MT20	1.5	4.0		
G	BMWWW-t	MT20	4.0	12.0		
H	BMV1+p	MT20	1.5	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
H	956 0	956 54	-237 5-8	1-8
F	956 0	956 0	-237 5-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT H FOR 237 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 237 LBS FACTORED UPLIFT
 PROVIDE FOR 54 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SOIL
H	COMBINED SNOW	LIVE PERM.LIVE WIND DEAD	0/0
F	731 482 / 0	104 / 0 0 / 0 0 / -262 144 / 0	0 / 0

HORIZONTAL REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SOIL
H	---	0 / 0 0 / 0 0 / 0 38 / -38	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (18)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX FACTORED (LBS)	MAX (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 40	-114.0	-114.0	0.33 (2)	10.00	G-C	-146 / 175	0.03 (17)
B-C	-761 / 167	-114.0	-114.0	0.46 (2)	6.25	B-G	-88 / 736	0.12 (1)
C-D	-761 / 167	-114.0	-114.0	0.46 (3)	6.25	G-D	-88 / 736	0.12 (1)
D-E	0 / 40	-114.0	-114.0	0.33 (3)	10.00			
H-B	-900 / 261	0.0	0.0	0.09 (1)	7.81			
F-D	-900 / 261	0.0	0.0	0.09 (1)	7.81			
H-G	-36 / 71	-27.5	-27.5	0.23 (17)	6.25			
G-F	-10 / 20	-27.5	-27.5	0.23 (17)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 33.9 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 55.8 PSF

SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED
 (80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL)= L/240 (0.52")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.46 (C-D:3), BC=0.23 (G-H:17),
 WB=0.12 (B-G:1), SSI=0.25 (C-D:3)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

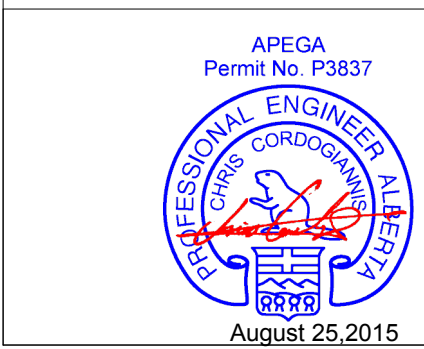
AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

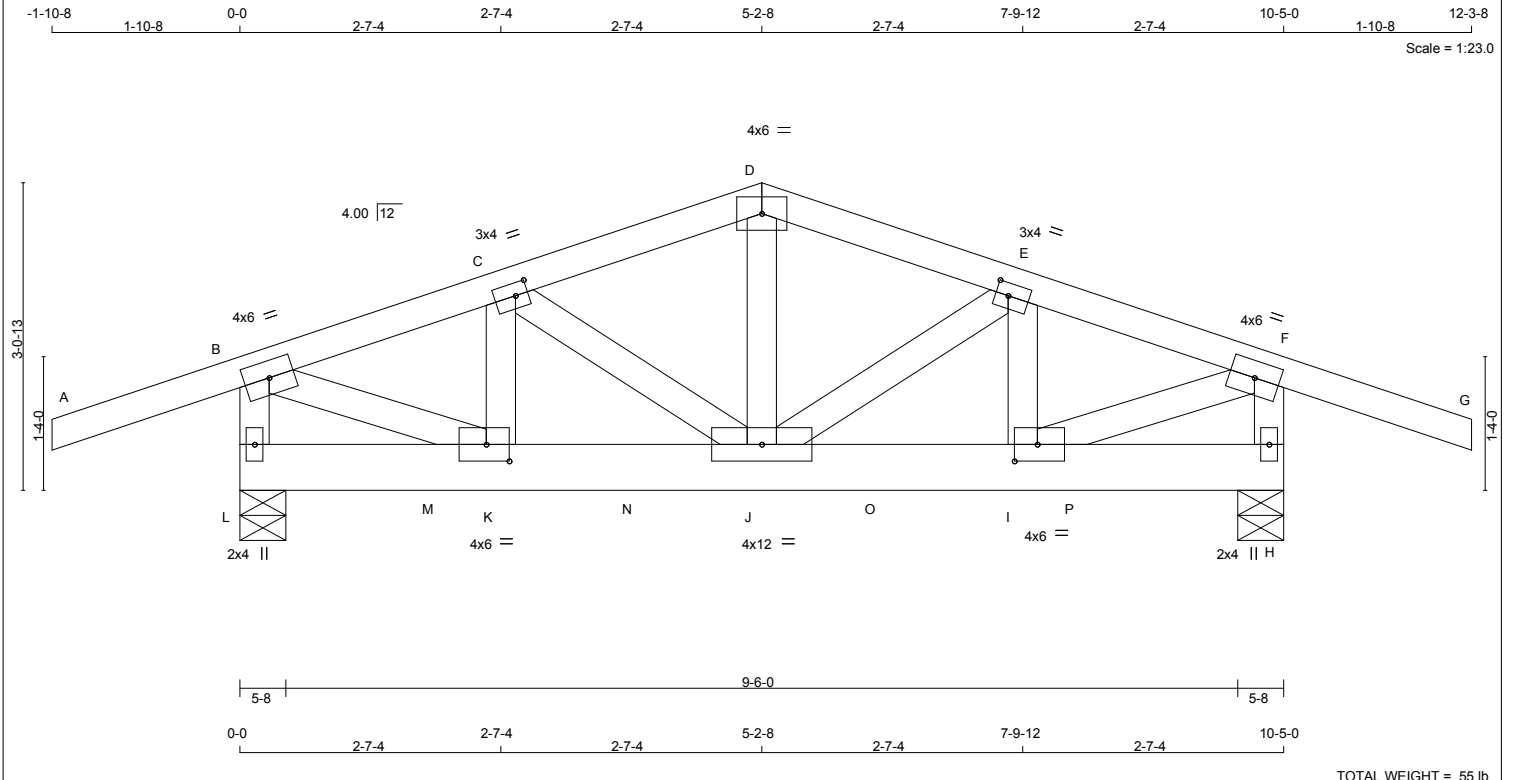
NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

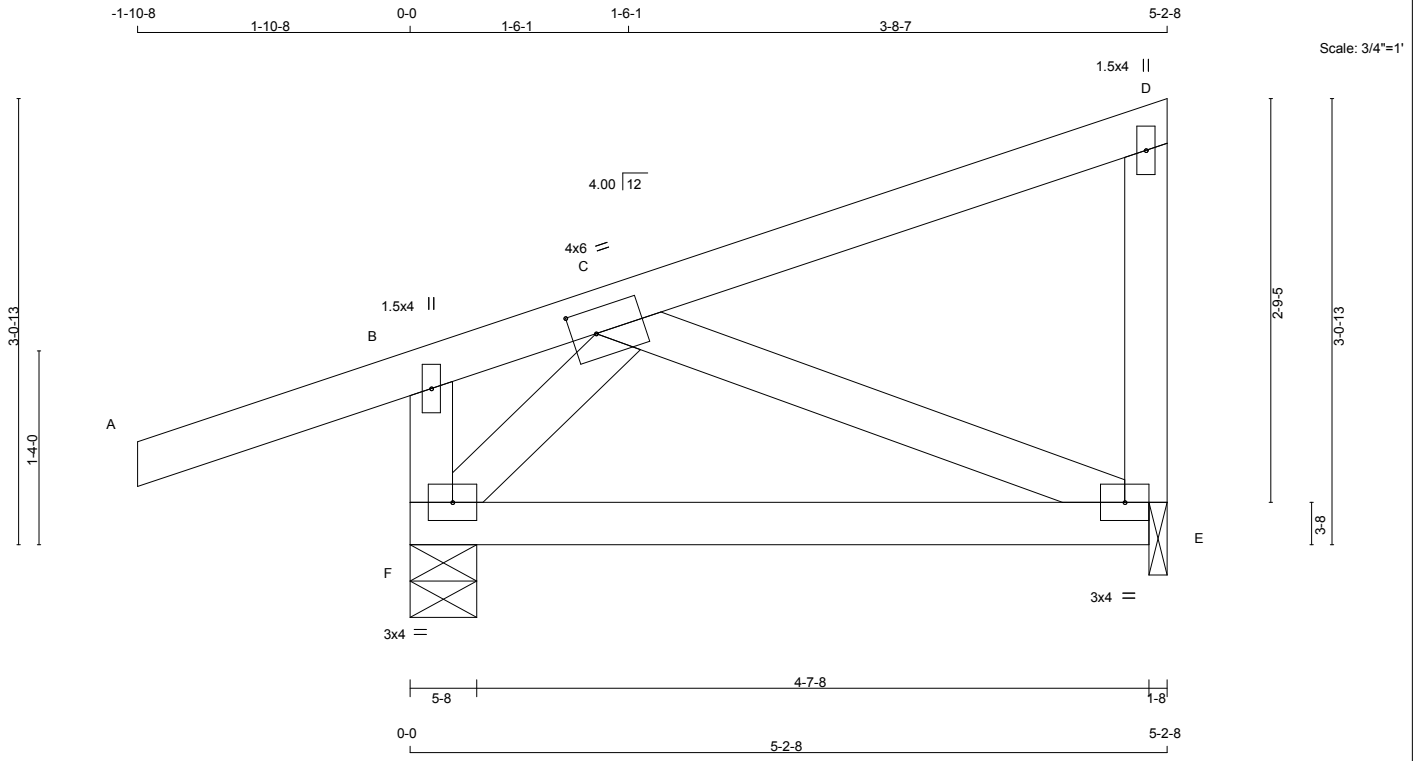
JSI GRIP= 0.72 (D) (INPUT = 0.90)
 JSI METAL= 0.35 (B) (INPUT = 1.00)





TOTAL WEIGHT = 55 lb

<p>LUMBER</p> <p>N. L. G. A. RULES</p> <table border="1"> <tr> <th>CHORDS</th> <th>SIZE</th> <th>LUMBER</th> <th>DESCR.</th> </tr> <tr> <td>A - D</td> <td>2x4</td> <td>DRY No.2</td> <td>SPF</td> </tr> <tr> <td>D - G</td> <td>2x4</td> <td>DRY No.2</td> <td>SPF</td> </tr> <tr> <td>L - B</td> <td>2x4</td> <td>DRY No.2</td> <td>SPF</td> </tr> <tr> <td>H - F</td> <td>2x4</td> <td>DRY No.2</td> <td>SPF</td> </tr> <tr> <td>L - H</td> <td>2x6</td> <td>DRY No.2</td> <td>SPF</td> </tr> </table> <p>ALL WEBS 2x4 DRY No.2 SPF</p> <p>DRY: SEASONED LUMBER.</p>	CHORDS	SIZE	LUMBER	DESCR.	A - D	2x4	DRY No.2	SPF	D - G	2x4	DRY No.2	SPF	L - B	2x4	DRY No.2	SPF	H - F	2x4	DRY No.2	SPF	L - H	2x6	DRY No.2	SPF	<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</p> <p>BEARINGS</p> <table border="1"> <tr> <th>JT</th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>REQRD BRG</th> </tr> <tr> <td>JT</td> <td>VERT</td> <td>HORZ</td> <td>DOWN</td> <td>HORZ</td> </tr> <tr> <td>L</td> <td>1583</td> <td>0</td> <td>1583</td> <td>47</td> </tr> <tr> <td>H</td> <td>1583</td> <td>0</td> <td>1583</td> <td>0</td> </tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT L FOR 487 LBS FACTORED UPLIFT</p> <p>PROVIDE ANCHORAGE AT BEARING JOINT H FOR 487 LBS FACTORED UPLIFT</p> <p>PROVIDE FOR 47 LBS FACTORED HORIZONTAL REACTION AT JOINT L</p>	JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	L	1583	0	1583	47	H	1583	0	1583	0	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 31.5 PSF DL = 5.0 PSF</p> <p>BOT CH. LL = 10.0 PSF DL = 7.0 PSF</p> <p>TOTAL LOAD = 53.5 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS</p> <p>- SLIPPERY ROOFING MATERIAL AS PER NBCC 4.1.6.2.(6) IS USED - SLOPE REDUCTION FACTOR USED - OVERHANG NOT TO BE ALTERED OR CUT OFF.</p> <p>(74 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 31.5 P.S.F. SPECIFIED ROOF LIVE LOAD</p>																																																																																																																																																
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<p>HANGERS NOTES</p> <p>1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 166.6 lbs FACTORED DOWN AND 107.0 lbs FACTORED UP AT 2-0-0, 341.0 lbs FACTORED DOWN AND 123.7 lbs FACTORED UP AT 4-0-0, 337.3 lbs FACTORED DOWN AND 115.7 lbs FACTORED UP AT 5-2-8, AND 341.0 lbs FACTORED DOWN AND 123.7 lbs FACTORED UP AT 6-5-0, AND 166.6 lbs FACTORED DOWN AND 107.0 lbs FACTORED UP AT 8-5-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.</p>	<p>LOADING</p> <p>TOTAL LOAD CASES: (18)</p> <table border="1"> <tr> <th colspan="3">C H O R D S</th> <th colspan="3">W E B S</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1</th> <th>MAX UNBRAC</th> <th>MEMB. MAX. 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MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX UNBRAC	MEMB. MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM	TO	LENGTH	FR-TO		A-B	0 / 37	-107.1	-107.1	0.34 (2)	10.00	J-D -268 / 803 0.14 (1)	B-C	-1670 / 545	-107.1	-107.1	0.38 (2)	4.69	J-E -41 / 163 0.03 (2)	C-D	-1697 / 559	-107.1	-107.1	0.17 (2)	4.91	I-E -259 / 79 0.03 (1)	D-E	-1697 / 559	-107.1	-107.1	0.17 (3)	4.91	C-J -41 / 163 0.03 (3)	E-F	-1670 / 545	-107.1	-107.1	0.38 (3)	4.69	K-C -259 / 79 0.03 (1)	F-G	0 / 37	-107.1	-107.1	0.34 (3)	10.00	B-K -502 / 1664 0.29 (1)	L-B	-1508 / 477	0.0	0.0	0.17 (1)	6.65	I-F -502 / 1664 0.29 (1)	H-F	-1508 / 477	0.0	0.0	0.17 (1)	6.65		L-M	-33 / 62	-27.5	-27.5	0.07 (1)	6.25		M-K	-33 / 62	-27.5	-27.5	0.07 (1)	6.25		K-N	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25		N-J	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25		J-O	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25		O-I	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25		I-P	-8 / 17	-27.5	-27.5	0.07 (1)	10.00		P-H	-8 / 17	-27.5	-27.5	0.07 (1)	10.00		JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	J	5-2-8	-337	-337	116	TOP	VERT	TOTAL	M	2-0-0	-167	-167	107	TOP	VERT	TOTAL	N	4-0-0	-341	-341	124	TOP	VERT	TOTAL	O	6-5-0	-341	-341	124	TOP	VERT	TOTAL	P	8-5-0	-167	-167	107	TOP	VERT	TOTAL	<p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</p> <p>NAIL VALUES</p> <table border="1"> <tr> <th>PLATE</th> <th>GRIP(DRY) (PSI)</th> <th>SHEAR (PLI)</th> <th>SECTION (PLI)</th> </tr> <tr> <td>MT20</td> <td>618</td> <td>354</td> <td>1667 822 2284 1656</td> </tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches</p> <p>PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.86 (F) (INPUT = 0.90) JSI METAL= 0.44 (F) (INPUT = 1.00)</p>	PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)	MT20	618	354	1667 822 2284 1656
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K-N	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25																																																																																																																																																																																									
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J-O	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25																																																																																																																																																																																									
O-I	-480 / 1563	-27.5	-27.5	0.34 (1)	6.25																																																																																																																																																																																									
I-P	-8 / 17	-27.5	-27.5	0.07 (1)	10.00																																																																																																																																																																																									
P-H	-8 / 17	-27.5	-27.5	0.07 (1)	10.00																																																																																																																																																																																									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE																																																																																																																																																																																							
J	5-2-8	-337	-337	116	TOP	VERT	TOTAL																																																																																																																																																																																							
M	2-0-0	-167	-167	107	TOP	VERT	TOTAL																																																																																																																																																																																							
N	4-0-0	-341	-341	124	TOP	VERT	TOTAL																																																																																																																																																																																							
O	6-5-0	-341	-341	124	TOP	VERT	TOTAL																																																																																																																																																																																							
P	8-5-0	-167	-167	107	TOP	VERT	TOTAL																																																																																																																																																																																							
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)																																																																																																																																																																																											
MT20	618	354	1667 822 2284 1656																																																																																																																																																																																											
<p>APEGA Permit No. P3837</p>	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7 } PSF AT {22-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.</p>																																																																																																																																																																																													



TOTAL WEIGHT = 24 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF
F - B	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
ALL WEBS	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	1.5	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	2.00
D	TMV+p	MT20	1.5	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
E	328	0	365	0
F	628	0	708	186

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT F FOR 168 LBS FACTORED UPLIFT
 PROVIDE FOR 186 LBS FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	263	177	-28	52 / 0	0 / 0	0 / -112	59 / 0	0 / 0
F	468	383	0	52 / 0	0 / 0	0 / -175	85 / 0	0 / 0

HORIZONTAL REACTIONS
 F --- 0 / 0 0 / 0 0 / 0 133 / 0 0 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, F

BRACING
 MAX. UNBRACED TOP CHORD LENGTH = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING
 TOTAL LOAD CASES: (19)

C H O R D S				W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)	
A-B	0 / 40	-114.0	-114.0	0.33 (2)	10.00	C-E	-226 / 167	0.05 (2)
B-C	-56 / 67	-114.0	-114.0	0.33 (19)	6.25	F-C	-331 / 158	0.03 (2)
C-D	-53 / 28	-114.0	-114.0	0.23 (2)	6.25			
E-D	-209 / 73	0.0	0.0	0.05 (11)	7.81			
F-B	-435 / 134	0.0	0.0	0.04 (19)	7.81			
F-E	-171 / 210	-27.5	-27.5	0.24 (6)	6.25			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)
 CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM). INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 33.9 PSF
 DL = 5.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 55.8 PSF

SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 4 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - SLOPE REDUCTION FACTOR USED
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL)= L/240 (0.26")
 CALCULATED VERT. DEFL.(TL) = L/820 (0.08")

CSI: TC=0.33 (A-B:2), BC=0.24 (E-F:6), WB=0.05 (C-E:2), SSI=0.21 (C-D:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
 WIND LOAD IMPORTANCE FACTOR = 1.00
 LIVE LOAD IMPORTANCE FACTOR = 1.00
 COMPANION LIVE LOAD FACTOR = 0.50

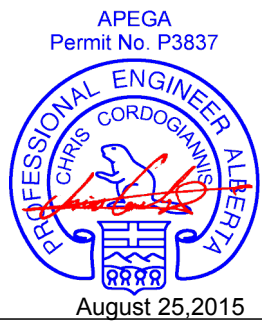
AUTOSOLVE RIGHT HEEL ONLY
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	SECTION (PLI)
MT20	618	354	1667	822
			2284	1656

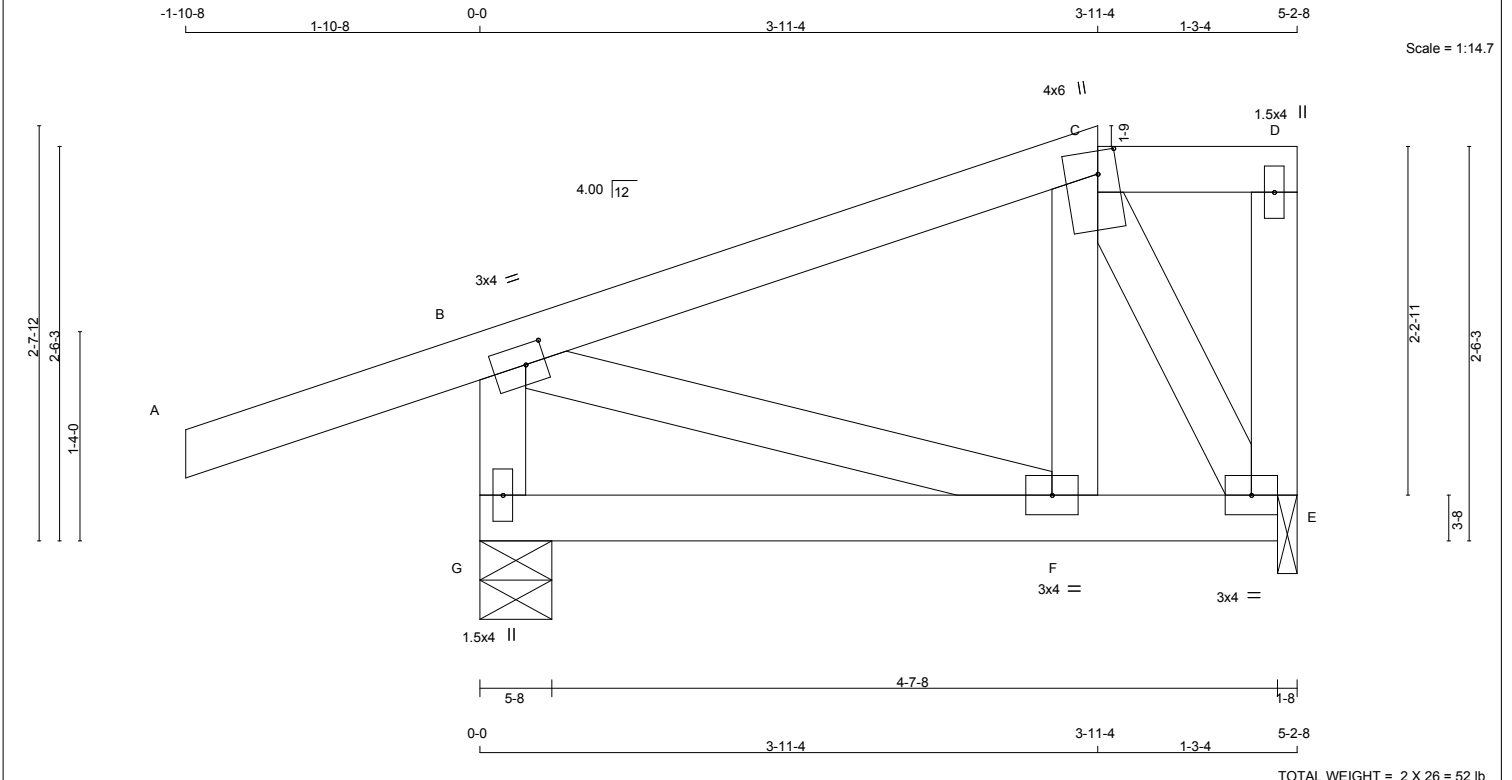
PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (F) (INPUT = 0.90)
 JSI METAL= 0.10 (B) (INPUT = 1.00)



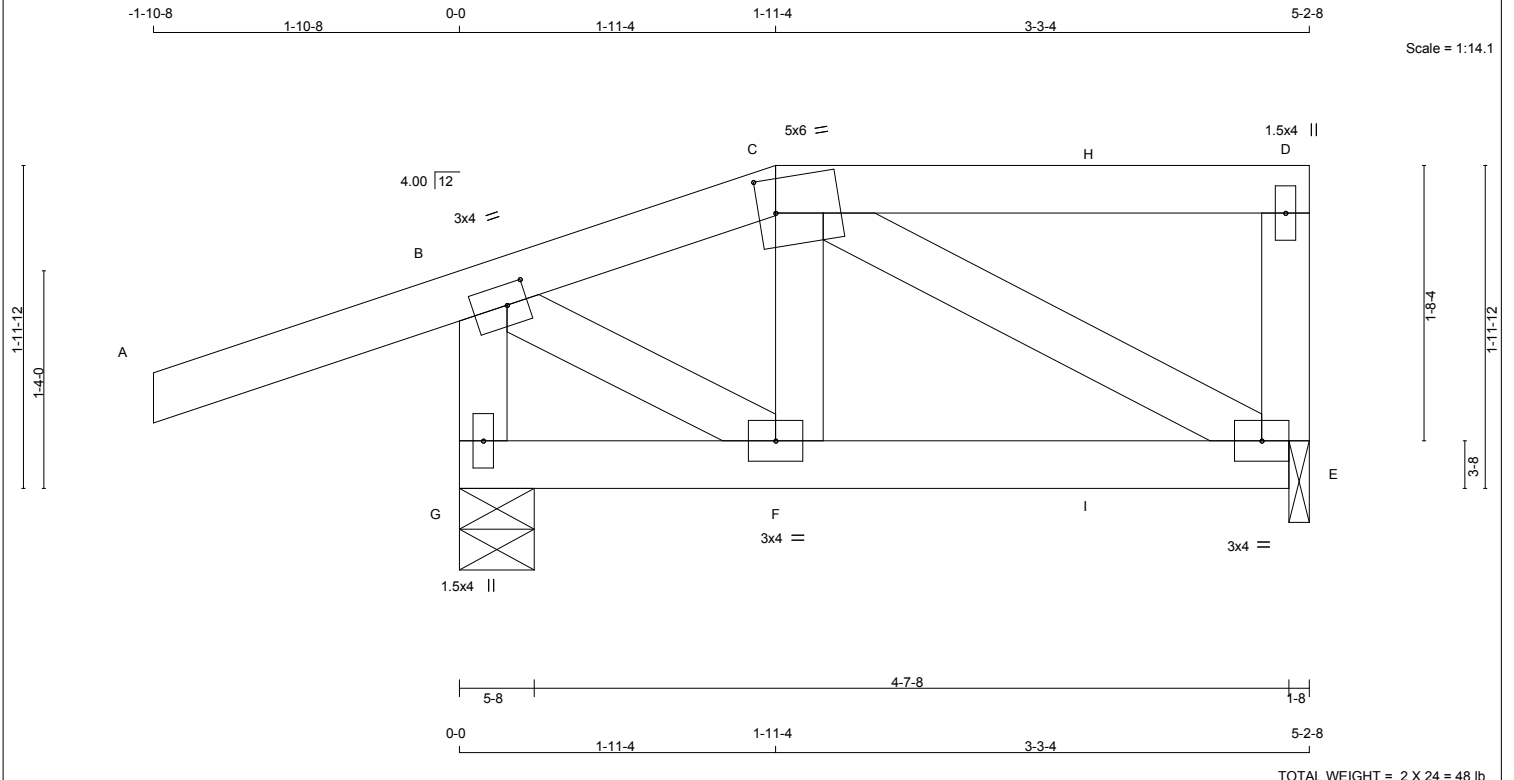
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473C rev. 10-08 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **TPIC Appendix G - Minimum Quality Manufacturing Criteria** available from www.tpica.ca and **BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOTAL WEIGHT = 2 X 26 = 52 lb

<p>LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - C 2x4 DRY No.2 SPF C - D 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF G - B 2x4 DRY No.2 SPF G - E 2x4 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.</p>	<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX E 368 0 368 0 -111 1-8 1-8 G 588 0 643 137 -155 5-8 1-8 PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT G FOR 155 LBS FACTORED UPLIFT PROVIDE FOR 137 LBS FACTORED HORIZONTAL REACTION AT JOINT G</p>	<p>DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 33.9 PSF DL = 5.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 55.8 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED (80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.01") ALLOWABLE DEFL.(TL)= L/240 (0.26") CALCULATED VERT. DEFL.(TL) = L/999 (0.01") CSI: TC=0.35 (B-C:2), BC=0.10 (E-F:17), WB=0.04 (C-E:1), SSI=0.19 (B-C:2) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 SNOW LOAD IMPORTANCE FACTOR = 1.00 WIND LOAD IMPORTANCE FACTOR = 1.00 LIVE LOAD IMPORTANCE FACTOR = 1.00 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.59 (E) (INPUT = 0.90) JSI METAL= 0.15 (G) (INPUT = 1.00)</p>
<p>PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-t MT20 3.0 4.0 1.50 1.50 C TTWW+m MT20 4.0 6.0 1.75 1.50 D TMV+p MT20 1.5 4.0 E BMVW1-t MT20 3.0 4.0 F BMVW-t MT20 3.0 4.0 G BMV1+p MT20 1.5 4.0</p>	<p>UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 291 176 / 0 52 / 0 0 / 0 0 / -120 62 / 0 0 / 0 G 440 343 / 0 52 / 0 0 / 0 0 / -163 82 / 0 0 / 0 HORIZONTAL REACTIONS G --- 0 / 0 0 / 0 0 / 0 98 / 0 0 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G</p>	
	<p>BRACING MAX. UNBRACED TOP CHORD LENGTH = 6.25FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25FT. OR RIGID CEILING DIRECTLY APPLIED. LOADING TOTAL LOAD CASES: (19) C H O R D S MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) LC1 MAX UNBRAC LENGTH FR-TO A-B 0 / 40 -114.0 -114.0 0.33 (2) 10.00 F-C 0 / 156 0.03 (17) B-C -178 / 45 -114.0 -114.0 0.35 (2) 6.25 C-E -341 / 85 0.04 (1) C-D -15 / 39 -114.0 -114.0 0.03 (3) 6.25 B-F 0 / 176 0.03 (1) E-D -72 / 33 0.0 0.0 0.03 (13) 7.81 G-B -599 / 174 0.0 0.0 0.06 (2) 7.81 G-F -117 / 2 -27.5 -27.5 0.09 (17) 6.25 F-E -45 / 166 -27.5 -27.5 0.10 (17) 6.25</p>	
	<p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8) CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (7.7) PSF AT (40-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.</p>	
<p>APEGA Permit No. P3837 August 25, 2015</p>		



TOTAL WEIGHT = 2 X 24 = 48 lb

<p>LUMBER</p> <p>N. L. G. A. RULES</p> <table border="1"> <tr><th>CHORDS</th><th>SIZE</th><th>LUMBER</th><th>DESCR.</th></tr> <tr><td>A - C</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>C - D</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>E - D</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>G - B</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>G - E</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> <tr><td>ALL WEBS</td><td>2x4 DRY</td><td>No.2</td><td>SPF</td></tr> </table> <p>DRY: SEASONED LUMBER.</p>	CHORDS	SIZE	LUMBER	DESCR.	A - C	2x4 DRY	No.2	SPF	C - D	2x4 DRY	No.2	SPF	E - D	2x4 DRY	No.2	SPF	G - B	2x4 DRY	No.2	SPF	G - E	2x4 DRY	No.2	SPF	ALL WEBS	2x4 DRY	No.2	SPF	<p>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</p> <p>BEARINGS</p> <table border="1"> <tr><th></th><th>FACTORED</th><th>MAXIMUM FACTORED</th><th>INPUT</th><th>REQRD</th></tr> <tr><td>JT</td><td>GROSS REACTION</td><td>GROSS REACTION</td><td>BRG</td><td>BRG</td></tr> <tr><td>E</td><td>VERT</td><td>HORZ</td><td>UPLIFT</td><td>IN-SX</td></tr> <tr><td>G</td><td>521</td><td>0</td><td>228</td><td>1-8</td></tr> </table> <p>PROVIDE ANCHORAGE AT BEARING JOINT E FOR 228 LBS FACTORED UPLIFT</p> <p>PROVIDE ANCHORAGE AT BEARING JOINT G FOR 204 LBS FACTORED UPLIFT</p> <p>PROVIDE FOR 108 LBS FACTORED HORIZONTAL REACTION AT JOINT G</p>		FACTORED	MAXIMUM FACTORED	INPUT	REQRD	JT	GROSS REACTION	GROSS REACTION	BRG	BRG	E	VERT	HORZ	UPLIFT	IN-SX	G	521	0	228	1-8	<p>DESIGN CRITERIA</p> <p>SPECIFIED LOADS:</p> <p>TOP CH. LL = 33.9 PSF DL = 5.0 PSF</p> <p>BOT CH. LL = 10.0 PSF DL = 7.0 PSF</p> <p>TOTAL LOAD = 55.8 PSF</p> <p>SPACING = 24.0 IN. C/C</p> <p>LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12</p> <p>THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010</p> <p>THIS DESIGN COMPLIES WITH: - PART 4 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011</p> <p>DESIGN ASSUMPTIONS - SLOPE REDUCTION FACTOR USED - OVERHANG NOT TO BE ALTERED OR CUT OFF.</p> <p>(80 % OF 39.7 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 33.9 P.S.F. SPECIFIED ROOF LIVE LOAD</p>																																																																																																												
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JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL																																																																																																																																																							
E	170	78 / -184	50 / 0	0 / 0	9 / -110	41 / 0	0 / 0																																																																																																																																																							
G	392	275 / -166	50 / 0	0 / 0	0 / -187	66 / 0	0 / 0																																																																																																																																																							
JT	---	0 / 0	0 / 0	0 / 0	77 / -4	0 / 0	0 / 0																																																																																																																																																							
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<p>HANGERS NOTES</p> <p>1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 63.2 lbs FACTORED DOWN AND 525.3 lbs FACTORED UP AT 1-11-4, AND 18.8 lbs FACTORED DOWN AND 183.9 lbs FACTORED UP AT 4-0-0 ON TOP CHORD, AND 43.0 lbs FACTORED UP AT 1-11-4, AND 35.6 lbs FACTORED UP AT 4-0-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.</p>	<p>LOADING</p> <p>TOTAL LOAD CASES: (36)</p> <table border="1"> <tr><th colspan="4">C H O R D S</th><th colspan="4">W E B S</th></tr> <tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>LC1</th><th>MAX UNBRAC</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX CSI (LC)</th></tr> <tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>LENGTH</td><td>FR-TO</td><td></td><td></td></tr> <tr><td>A-B</td><td>0 / 40</td><td>-114.0</td><td>-114.0</td><td>0.37 (2)</td><td>10.00</td><td>F-C</td><td>-20 / 227</td></tr> <tr><td>B-C</td><td>-123 / 368</td><td>-114.0</td><td>-114.0</td><td>0.42 (20)</td><td>6.25</td><td>C-E</td><td>-135 / 426</td></tr> <tr><td>C-H</td><td>-11 / 28</td><td>-114.0</td><td>-114.0</td><td>0.16 (1)</td><td>6.25</td><td>B-F</td><td>-421 / 135</td></tr> <tr><td>H-D</td><td>-11 / 28</td><td>-114.0</td><td>-114.0</td><td>0.16 (1)</td><td>6.25</td><td></td><td></td></tr> <tr><td>E-D</td><td>-133 / 85</td><td>0.0</td><td>0.0</td><td>0.03 (34)</td><td>7.81</td><td></td><td></td></tr> <tr><td>G-B</td><td>-499 / 215</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>7.81</td><td></td><td></td></tr> <tr><td>G-F</td><td>-87 / 1</td><td>-27.5</td><td>-27.5</td><td>0.06 (31)</td><td>6.25</td><td></td><td></td></tr> <tr><td>F-I</td><td>-390 / 120</td><td>-27.5</td><td>-27.5</td><td>0.08 (31)</td><td>6.25</td><td></td><td></td></tr> <tr><td>I-E</td><td>-390 / 120</td><td>-27.5</td><td>-27.5</td><td>0.08 (31)</td><td>6.25</td><td></td><td></td></tr> </table> <p>FACTORED CONCENTRATED LOADS (LBS)</p> <table border="1"> <tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX-</th><th>MAX+</th><th>FACE</th><th>DIR.</th><th>TYPE</th></tr> <tr><td>C</td><td>1-11-4</td><td>103</td><td>-63</td><td>525</td><td>BACK</td><td>VERT</td><td>TOTAL</td></tr> <tr><td>F</td><td>1-11-4</td><td>3</td><td>---</td><td>27</td><td>BACK</td><td>VERT</td><td>TOTAL</td></tr> <tr><td>F</td><td>2-0-0</td><td>25</td><td>---</td><td>36</td><td>BACK</td><td>VERT</td><td>TOTAL</td></tr> <tr><td>H</td><td>4-0-0</td><td>85</td><td>-19</td><td>184</td><td>BACK</td><td>VERT</td><td>TOTAL</td></tr> <tr><td>I</td><td>4-0-0</td><td>25</td><td>---</td><td>36</td><td>BACK</td><td>VERT</td><td>TOTAL</td></tr> </table> <p>TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)</p> <p>CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN</p> <p>WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.7 } PSF AT { 40-0-0 } FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE { MAIN WIND FORCE RESISTING SYSTEM }. INTERNAL WIND PRESSURE IS BASED ON DESIGN { CATEGORY 2 }. BUILDING MAY BE LOCATED ON { OPEN TERRAIN }, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST { 0-0 } FT-IN-SX AWAY FROM EAVE.</p>	C H O R D S				W E B S				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	FR-TO		FROM	TO	LENGTH	FR-TO			A-B	0 / 40	-114.0	-114.0	0.37 (2)	10.00	F-C	-20 / 227	B-C	-123 / 368	-114.0	-114.0	0.42 (20)	6.25	C-E	-135 / 426	C-H	-11 / 28	-114.0	-114.0	0.16 (1)	6.25	B-F	-421 / 135	H-D	-11 / 28	-114.0	-114.0	0.16 (1)	6.25			E-D	-133 / 85	0.0	0.0	0.03 (34)	7.81			G-B	-499 / 215	0.0	0.0	0.06 (1)	7.81			G-F	-87 / 1	-27.5	-27.5	0.06 (31)	6.25			F-I	-390 / 120	-27.5	-27.5	0.08 (31)	6.25			I-E	-390 / 120	-27.5	-27.5	0.08 (31)	6.25			JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	C	1-11-4	103	-63	525	BACK	VERT	TOTAL	F	1-11-4	3	---	27	BACK	VERT	TOTAL	F	2-0-0	25	---	36	BACK	VERT	TOTAL	H	4-0-0	85	-19	184	BACK	VERT	TOTAL	I	4-0-0	25	---	36	BACK	VERT	TOTAL	<p>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</p> <p>NAIL VALUES</p> <table border="1"> <tr><th>PLATE</th><th>GRIP(DRY) (PSI)</th><th>SHEAR (PLI)</th><th>SECTION (PLI)</th></tr> <tr><td>MT20</td><td>618</td><td>354</td><td>1667</td></tr> <tr><td></td><td>822</td><td>2284</td><td>1656</td></tr> </table> <p>PLATE PLACEMENT TOL. = 0.250 inches</p> <p>PLATE ROTATION TOL. = 5.0 Deg.</p> <p>JSI GRIP= 0.77 (C) (INPUT = 0.90) JSI METAL= 0.14 (E) (INPUT = 1.00)</p>	PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)	MT20	618	354	1667		822	2284	1656
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